To: BOARD OF SUPERVISORS

FROM: Planning and Building Services

MEETING DATE: October 8, 2024

DEPARTMENT CONTACT:	Shelby Miller
DEPARTMENT CONTACT:	Julia Krog

PHONE:707-234-6650PHONE:707-234-6650

ITEM TYPE: Consent Agenda

TIME ALLOCATED FOR ITEM: N/A

AGENDA TITLE:

Acceptance of Informational Report Regarding the Issuance of Emergency Coastal Development Permit EM_2024-0003 (Balderston) to Install Drilled Piers, a Grade Beam Foundation System, and a Perimeter Foundation Drain, Located at 12700 N. Highway 1, Mendocino; APN: 118-320-05

RECOMMENDED ACTION/MOTION:

Accept the Informational Report regarding the issuance of Emergency Coastal Development Permit EM_2024-0003 (Balderston) to install drilled piers, a grade beam foundation system, and a perimeter foundation drain, located at 12700 N. Highway 1, Mendocino; APN: 118-320-05

PREVIOUS BOARD/BOARD COMMITTEE ACTIONS:

In 1991 the Board adopted Division II of Title 20 of Mendocino County Code through Ordinance No. 3785. Included in Division II is Section 20.536.055 which allows Permits for Approval of Emergency Work. In cases of a verified emergency, temporary emergency authorization to proceed with remedial measures may be given by the Director or his or her designee until such time as a Coastal Development Permit application is filed. The Director shall report in writing to the Board of Supervisors at each meeting the emergency permits applied for or issued since the last report, with a description of the nature of the emergency permit is solely at the discretion of the Director. Notice of emergency permits shall be provided by phone or letter to the California Coastal Commission within three (3) days, following issuance of the emergency permit.

SUMMARY OF REQUEST:

The single-family residence was built in 1941. Seasonal seepage is the primary cause of the Emergency Permit request. The structural concrete foundation and stem walls are cracked and distressed. The single-family residence is unable to be secured without repairing the foundation by installing drilled piers, a grade beam foundation system, and a perimeter foundation drain.

The emergency permit was issued on September 23, 2024, and becomes null and void at the end of sixty (60) days. Prior to expiration of this Emergency Permit, the applicant shall submit a standard Coastal Development Permit application for the work authorized by the permit.

ALTERNATIVE ACTION/MOTION:

None, informational Item.

STRATEGIC PLAN PRIORITY DESIGNATION: A Safe and Healthy County

SUPERVISORIAL DISTRICT: DISTRICT 5

VOTE REQUIREMENT: Majority

SUPPLEMENTAL INFORMATION AVAILABLE ONLINE AT: https://www.mendocinocounty.gov/departments/planning-

FISCAL DETAILS:

SOURCE OF FUNDING: N/A CURRENT F/Y COST: N/A ANNUAL RECURRING COST: N/A BUDGETED IN CURRENT F/Y: N/A IF NO, PLEASE DESCRIBE: REVENUE AGREEMENT: N/A

AGREEMENT/RESOLUTION/ORDINANCE APPROVED BY COUNTY COUNSEL: N/A

CEO LIAISON: Steve Dunnicliff, Deputy CEO

CEO REVIEW: Choose an item. CEO COMMENTS:

FOR COB USE ONLY

Executed By: Deputy Clerk Date: Date Executed

Final Status: Item Status Executed Item Type: item Number:



COUNTY OF MENDOCINO DEPARTMENT OF PLANNING AND BUILDING SERVICES 860 NORTH BUSH STREET · UKIAH · CALIFORNIA · 95482

860 North Bush Street · Ukiah · California · 95482 120 West Fir Street · Fort Bragg · California · 95437 JULIA KROG, DIRECTOR PHONE: 707-234-6650 FAX: 707-463-5709 FB PHONE: 707-964-5379 FB FAX: 707-961-2427 pbs@mendocinocounty.gov/pbs

September 23, 2024

Melissa Kraemer, District Manager North Coast District California Coastal Commission 1385 Eighth Street, Suite 130 Arcata, CA 95521

Subject: Issuance of Emergency Permit (EM_2024-0003)

Dear Melissa Kraemer,

Please find enclosed a copy of Emergency Coastal Development Permit EM 2024-0003. The Permit is scheduled as an informational item during the Consent Agenda for the Board of Supervisors Hearing on October 8, 2024.

Should you have any questions, please feel free to contact this office.

Sincerely, .Thur Shelby Miller

Planner II

Encl: EM_2024-0003 Permit

cc: Aum Construction P.O Box 1033 Mendocino, CA 95460



COUNTY OF MENDOCINO DEPARTMENT OF PLANNING AND BUILDING SERVICES

860 North Bush Street · Ukiah · California · 95482 120 West Fir Street · Ft. Bragg · California · 95437 SULIA KROG, DIRECTOR Telephone 707-234-6650 FAX 707-463-5709 FB PHONE: 707-964-5379 FB FAX: 707-961-2427 pbs@mendocinocounty.gov/pbs

September 23, 2024

COASTAL DEVELOPMENT STANDARD PERMIT AUTHORIZATION FOR EMERGENCY WORK CASE FILE EM_2024-0003

OWNER/ APPLICANT:

DANIEL BALDERSTON P.O BOX 296 LOYALTON, CA 96118

AGENT:

AUM CONSTRUCTION C/O SUNNY CHANCELLOR P.O BOX 1033 MENDOCINO, CA 95460

SITE ADDRESS/ APN:

NATURE OF EMERGENCY:

CAUSE OF EMERGENCY:

The existing stem walls and structural concrete foundation of the single-family residence is cracking and distressed. The residence was built in 1941.

12700 N. HIGHWAY 1, MENDOCINO, CA (APN: 118-320-05)

The emergency is the result of seasonal seepage. A Geotechnical Investigation, prepared by Brunsing Associates, Inc. on January 10, 2024, recommends that a perimeter foundation drain be installed around the perimeter foundation elements. Prior to construction, Brunsing shall review the final grading and foundation plans, and geotechnical related specifications for conformance. During construction, Brunsing shall be retained to provide periodic observations, together with field and laboratory testing, during site preparation, placement, and compaction of fills, and foundation construction.

REMEDIAL ACTION: The action to be approved under this Emergency Permit is authorization of the installation of drilled piers, a grade beam foundation system, and a perimeter foundation drain prior to the approval and issuance of the Standard Coastal Development Permit (CDP_2024-0032) request currently being processed by the Mendocino County Department of Planning and Building for review and entitlement from the Coastal Permit Administrator.

See the attached Emergency Permit application, site plan, and the Geotechnical Investigation prepared on January 10, 2024.

CIRCUMSTANCES TO JUSTIFY EMERGENCY:

Failure to allow this emergency work may result in a risk to the property, residents, adjacent parcels and the environment.

COMPLIANCE:

Permits for Approval of Emergency Work are subject to MCC Sec. 20.536.060 *et seq*.

This emergency permit is effective immediately and shall become null and void at the end of sixty (60) days. Prior to expiration of this Emergency Permit, the applicant shall submit a standard Coastal Development Permit application for the work authorized by this permit.

Prior to construction, Brunsing shall review the final grading and foundation plans, and geotechnical related specifications for conformance.

During construction, Brunsing shall be retained to provide periodic observations, together with field and laboratory testing, during site preparation, placement, and compaction of fills, and foundation construction.

RECOMMENDED BY:

PLANNER II

September 23,2024 DATE 9/23/2024 DATE

APPROVED BY: STAL PERMIT ADMINISTRATOR

Case No(s) EM 2024- 0003 **COUNTY OF MENDOCINO** DEPT OF PLANNING & BUILDING SERVICES CDF No(s) Date Filed **120 WEST FIR STREET** FORT BRAGG, CA 95437 Fee \$ L. A Telephone: 707-964-5379 Receipt No. Received by mellano Office Use Onl EMERGENCY PERMIT APPLICATION FORM Name of Applicant Name of Owner(s) Name of Agent AUM ConstructionInc Daniel Balderston Daniel Balderston closung Chancellor Mailing Address Mailing Address Mailing Address POBox 296 Loyalton, CA 96118 Loyalton, CA 96118 Telephone Number POBOX 1033 POBox 296 Mendocino, CA 95460 Telephone Numbe 412-889-0082 707-937-3624 412-889-0082 Project Description: Repair and Fortity Foundation. Add Generat-EM_2024-0003 This application for Fence Blocking View of Ger SFR Foundation Repair/Stabilization Propane Tank. Add Rolling (only the townant on work Driving Directions The site is located on the E (N/S/E/W) side of Road GOO D (name road) approximately 100 $(f+ (feet/miles) \ (N/S/E/W) \text{ of its intersection with}$ Hwy (provide nearest major intersection). Assessor's Parcel Number(s) 118-32-005 Parcel Size Street Address of Project 12700 N Hury I, Mendocino, CA Square Feet .49 Acres Please note: Before submittal, please verify correct street address with the Planning Division in Ukiah.

2 .

EMERGENCY PERMIT APPLICATION QUESTIONNAIRE

The purpose of this questionnaire is to relate information concerning your application to the Planning & Building Services Department and other agencies who will be reviewing your project proposal. The more detail that is provided, the easier it will be to promptly process your application. Please answer all questions. For questions which do not pertain to your project, please indicate "Not Applicable" or "N/A".

1. NATURE OF THE EMERGENCY NARRATIVE (use additional pages if necessary).

a) Describe the nature, cause and location of the emergency.

Cracking and Fracturing of Existing Structural Concrede Foundation.

b) Describe the remedial protective or preventive work required to deal with the emergency.

Structural Retro Eit of Conc Foundation. Design of Reprotez work to be provided and stamped by Licenced Structural Engineer.

c) Describe the circumstances during the emergency that justify the course(s) of action taken, including the probable consequences of failing to take action.

It Existing concrede Coundation work not fixed, existing building subject to shifting and settling.

d) Describe any secondary improvements such as wells, septic systems, grading, vegetation removal, roads, etc. that are necessary to deal with the emergency.

n/9

2.

1	If yes, describe below and identify the use of each structure on the plot plan. Easin /Library z/Garage 3/Shop 4/wood shed
3.	Is any grading or road construction planned? Yes No
	Estimate the amount of grading in cubic yards $\underline{n/a}$ c.y. If greater than 50 cubic yards or if greater than 2 feet of cut or 1 foot of fill will result, please provide a grading plan.
	Describe the terrain to be traversed (e.g., steep, moderate slope, flat, etc.).
	and wetter and the start of the second strate the second strates
4.	Will vegetation be removed on areas other than the building sites and roads? Yes XNo If yes, explain:
5.	Project Height. Maximum height of structure(s): <u>24'</u> feet
	Describe all exterior materials and colors of all proposed structures that are visible beyond the boundaries of the
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5.	Describe all exterior materials and colors of all proposed structures that are visible beyond the boundaries of the
5.	Describe all exterior materials and colors of all proposed structures that are visible beyond the boundaries of the subject parcel. No change to project colors of materials Existing building has un painted horizontal wood siding Are there any water courses, anadromous fish streams, ponds, lakes, sand dunes, rookeries, marine mammal haul- out areas, wetlands, riparian areas, pygmy vegetation, rare or endangered plants, animals or habitat which support rare and endangered species located on the project site or within 100 feet of the project site?
5.	Describe all exterior materials and colors of all proposed structures that are visible beyond the boundaries of the subject parcel. No Change to project colors of materials Existing building has un painted horizontal wood siding Are there any water courses, anadromous fish streams, ponds, lakes, sand dunes, rookeries, marine mammal haul- out areas, wetlands, riparian areas, pygmy vegetation, rare or endangered plants, animals or habitat which support
5. 6. 7.	Describe all exterior materials and colors of all proposed structures that are visible beyond the boundaries of the subject parcel. No change to project colors of materials Existing building has un painted horizontal wood siding Are there any water courses, anadromous fish streams, ponds, lakes, sand dunes, rookeries, marine mammal haul- out areas, wetlands, riparian areas, pygmy vegetation, rare or endangered plants, animals or habitat which suppor rare and endangered species located on the project site or within 100 feet of the project site?

SUBMIT ONLY ONE COPY

INDEMNIFICATION AND HOLD HARMLESS

ORDINANCE NO. 3780, adopted by the Board of Supervisors on June 4, 1991, requires applicants for discretionary land use approvals, to sign the following Indemnification Agreement. Failure to sign this agreement will result in the application being considered incomplete and withheld from further processing.

INDEMNIFICATION AGREEMENT

As part of this application, applicant agrees to defend, indemnify, release and hold harmless the County of Mendocino, its agents, officers, attorneys, employees, boards and commissions, as more particularly set forth in Mendocino County Code Section 1.04.120, from any claim, action or proceeding brought against any of the foregoing individuals or entities, the purpose of which is to attack, set aside, void or annul the approval of this application or adoption of the environmental document which accompanies it. The indemnification shall include, but not be limited to, damages, costs, expenses, attorney fees or expert witness fees that may be asserted by any person or entity, including the applicant, arising out of or in connection with the approval of this application, whether or not there is concurrent, passive or active negligence on the part of the County, its agents, officers, attorneys, employees, boards and commissions.

Date: 12/2024

Daniel Balderph Applicant

CERTIFICATION AND SITE VIEW AUTHORIZATION

- 1. I hereby certify that I have read this completed application and that, to the best of my knowledge, the information in this application, and all attached appendices and exhibits, is complete and correct. I understand that the failure to provide any requested information or any misstatements submitted in support of the application shall be grounds for either refusing to accept this application, for denying the permit, for suspending or revoking a permit issued on the basis of such misrepresentations, or for seeking of such further relief as may seem proper to the County.
- 2. I hereby grant permission for County Planning and Building Services staff and hearing bodies to enter upon and site view the premises for which this application is made in order to obtain information necessary for the preparation of required reports and render its decision.

7 23/24 Owner/Authorized Agent

NOTE: IF SIGNED BY AGENT, OWNER MUST SIGN BELOW.

AUTHORIZATION OF AGENT

I hereby authorize Aum Construction Inc - Sunny Chancellor to act as my representative and to bind me in all matters concerning this application.

amel Bilder

7/2 / 2024

MAIL DIRECTION

To facilitate proper handling of this application, please indicate the names and mailing addresses of individuals to whom you wish correspondence and/or staff reports mailed <u>if different from those identified on Page One of the application form</u>.

Name	Name	Name		
Mailing Address	Mailing Address	Mailing Address		



COUNTY OF MENDOCINO DEPARTMENT OF PLANNING AND BUILDING SERVICES

860 North Bush Street · Ukiah · California · 95482 120 West Fir Street · Ft. Bragg · California · 95437

ACKNOWLEDGMENT OF DEPOSIT/HOURLY FEE

By signing below, the applicant acknowledges that the staff at Planning and Building Services has discussed the potential for collection of a deposit fee for the projects listed below (as adopted by the Board of Supervisors in Resolution No.'s 11-072, 16-150, 18-122 and 19-170):

- 1. Division of Land Project
- 2. General Plan Project
- 3. Coastal Project
- 4. Zoning Project
- 5. Administrative Project
- 6. Cannabis Project
- 7. Private Road Naming

Once an application has been submitted and the processing costs approach 80% of the application fee, additional staff processing time will be billed at \$90.00/hour. Staff will notify the applicant/owner that a deposit equal to 50% of the initial filing fee is required for further processing, and more than one deposit may be required depending on the complexity of the project and the staff time necessary to complete application processing.

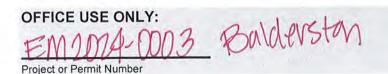
I acknowledge that I was advised of the deposit fee for continued processing after the initial application fee has been expended

rielk

Applicant Signature

2/2024

Date



RECORDING REQUESTED BY: Redwood Empire Title Company of Mendocino County 2023-06914 Recorded at the request of: REDWOOD EMPIRE TITLE 08/29/2023 02:37 PM Fee: \$22.00 Pgs: 1 of 4 OFFICIAL RECORDS Katrina Bartolomie - Clerk-Recorder Mendocino County, CA



Mail Tax Statements and When Recorded Mail Document To:

Daniel E. Balderston PO Box 296 Loyalton, CA 96118

Escrow No.: 20230853MN

SPACE ABOVE THIS LINE FOR RECORDER'S USE

GRANT DEED



The undersigned grantor(s) declare(s) Documentary Transfer Tax stamps in the sum of \$1,595.00

- computed on full value of property conveyed, or
- □ computed on full value less value of liens or encumbrances remaining at time of sale,
- Unincorporated area of Mendocino

FOR A VALUABLE CONSIDERATION, receipt of which is hereby acknowledged,

Robyn Prizmich and Tracey Hathaway, formerly known as Tracey Bernardini, and Brook J. Hathaway and Linda L. Hathaway, Trustees of The Hathaway Family Trust, dated July 30, 2019

hereby GRANT(S) to

Daniel E. Balderston, an unmarried man

the following described real property:

A certain lot or parcel of land, situated North of the Town of Mendocino and North of Jack Peters Gulch Bridge, and which point of beginning is the Southeast corner of the Christensen Ranch, formerly the Gray Ranch, and which point is the Southeast corner of what was known as the Lachlin Johnson Lot, and lying in the East and West center line of Section 19, Township 17 North, Range 17 West, Mount Diablo Meridian, and where the same is cut by the West line of the Old Coast Road; thence from said point of beginning North 6° West, 142.75 feet; thence North 15° 45' West, 75 feet; thence South 68° 34' West, 347 feet to the East line of California Highway No. 1; thence along said East line South 53° 35' East 147.33 feet to center line of Section 19; thence East on such line 239 feet, more or less, to the point of beginning.

EXCEPTING THEREFROM: That portion conveyed in the Deed executed by John E. Anderson and Alma F. Anderson, husband and wife, to State of California, dated July 19, 1962, recorded September 20, 1962 in Volume 607, Official Records, Page 73, Mendocino County Records.

APN: 118-320-05

This document is being executed in counterparts which together shall constitute one original.

Dated: August 23, 2023

Tracey Bernardini

Brook J. Hathaway and Linda L. Hathaway, Trustees of The Hathaway Family Trust, dated July 30, 2019

Brook J. Hathaway, Trustee

Linda L. Hathaway, Trustee

A notary public or other officer completing this certificate verifies only the identity of the individual who signed the document to which this certificate is attached, and not the truthfulness, accuracy, or validity of that document.

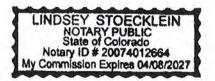
State of Galilonia Colurado County of <u>Elbert</u> On August 24, 2023 before me, <u>Lind Stan Strecklein</u>, Notary Public, personally appeared <u>Robyn Prizmich</u>

the person(s) whose name(s) is/are subscribed to the within instrument and acknowledged to me that he/she/they executed the same in his/her/their authorized capacity(ies), and that by his/her/their signature(s) on the instrument the person(s), or the entity upon behalf of which the person(s) acted, executed the instrument.

I certify under PENALTY OF PERJURY under the laws of the State of California that the foregoing paragraph is true and correct.

Witness my hand and official seal.

exp: 04/08/2027 (Seal) Signature /



.

Dated: August 23, 2023

Robyn Prizmich

fk a Tracey Bernardin

Brook J. Hathaway and Linda L. Hathaway, Trustees of The Hathaway Family Trust, dated July 30, 2019

Brook J. Hathaway, Trustee

Linda L. Hathaway, Trustee

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State of California County of Meh before me, KUP Ont Notary Public, Howze M aura personally appeared acou

who proved to me on the basis of satisfactory evidence to be the person(s) whose name(s) is/are subscribed to the within instrument and acknowledged to me that he/she/they executed the same in his/her/their authorized capacity(les), and that by his/her/their signature(s) on the instrument the person(s), or the entity upon behalf of which the person(s) acted, executed the instrument.

I certify under PENALTY OF PERJURY under the laws of the State of California that the foregoing paragraph is true and correct.

Witness my hand and official seal.

(Seal) Signature/ M. How

M. HOWZE Notary Public - California Mendocino County Commission # 2399170 Comm. Expires Apr 27, 2026 Dated: August 23, 2023

Robyn Prizmich

Tracey Bernardini

Brook J. Hathaway and Linda L. Hathaway, Trustees of The Hathaway Family Trust, dated July 30, 2019

Brook J. Hathaway, Trustee

hours inda L. Hathaway, Trustee

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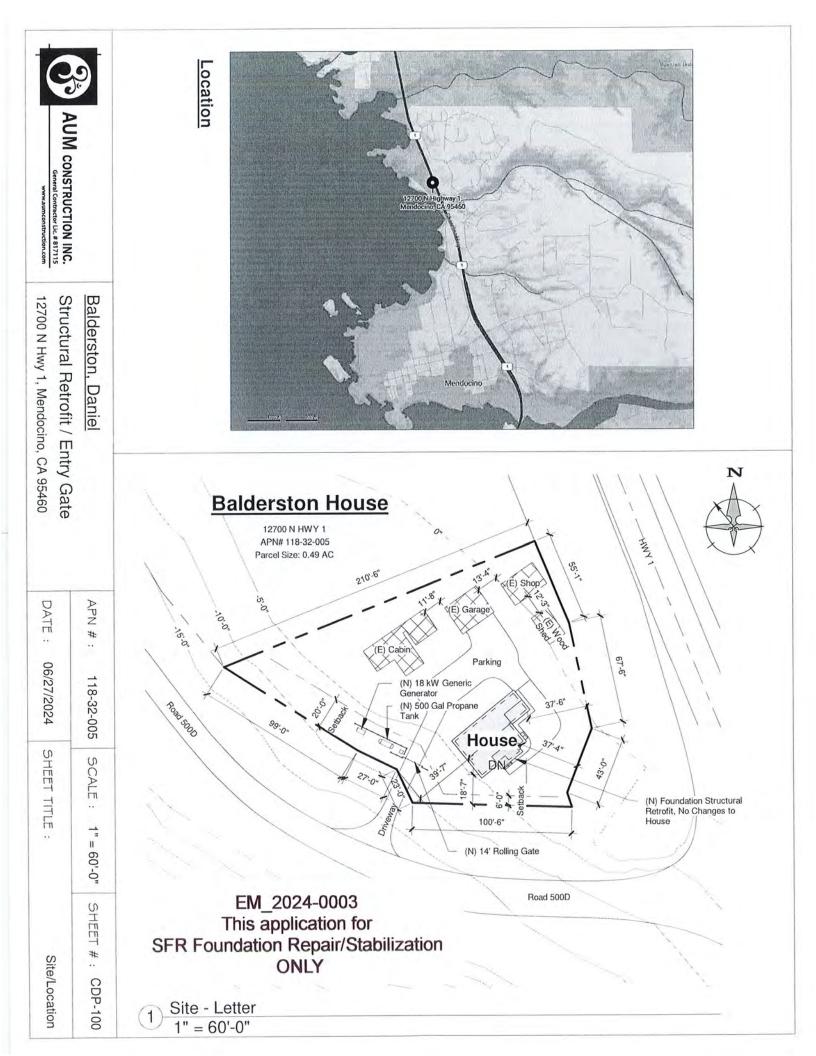
State of California County of <u>P</u><u>A</u><u>C</u><u>U</u> On <u>August</u> <u>23</u>, <u>2023</u> before me, <u>Gina Gonzalez</u>, Notary Public, personally appeared <u>Brook J</u>. Ha thaway and Linda L. Ha thaway who proved

Lind a L. Ha fhaway who proved to me on the basis of satisfactory evidence to be the person(s) whose name(s) is/are subscribed to the within instrument and acknowledged to me that he/she/they executed the same in his/her/their authorized capacity(ies), and that by his/her/their signature(s) on the instrument the person(s), or the entity upon behalf of which the person(s) acted, executed the instrument.

I certify under PENALTY OF PERJURY under the laws of the State of California that the foregoing paragraph is true and correct.

Witness my hand and official seal. Signature______(Seal)

GINA GONZALEZ COMM. # 2445380 OTARY PUBLIC - CALIFORNIA 0 COUNTY OF PLACER Y CONN. EXP. MAY 23, 2027





Mendocino County Planning and Building Services

860 North Bush Street Ukiah, CA 95482 (707) 234-6650

120 West Fir Street Fort Bragg, CA 95437 (707) 964-5379

Receipt: PRJ_063428

Paid By: AUM CONSTRUCTION INC P. O. BOX 1033

MENDOCINO

CA 95460

Date: 8/16/2024 Pay Method: CHECK 14849

Received By: SANDY ARELLANO

EM 2024-0003

Project Number: EM_2024-0003

Project Description: SFR FOUNDATION REPAIR AND FORTIFICATIC

Site Address: 12700 NO HWY 1

Account Number	Qty	Fee Amount
1100-2851-822609		\$1,916.00
		\$1,916.00
1100-2851-826188		\$200.00
		\$200.00
1222-2852-826260		\$147.00
		\$147.00
	1100-2851-822609	1100-2851-822609

Total Fees Paid:

\$2,263.00

MC_ProjectReceipt.rpt

Printed: 8/16/24



THIS MAP AND DATA ARE PROVIDED WITHOUT WARRANTY OF ANY KIND. DO NOT USE THIS MAP TO DETERMINE LEGAL PROPERTY BOUNDARIES



Brunsing Associates, Inc.

GEOTECHNICAL INVESTIGATION

BALDERSTON FOUNDATION IMPROVEMENTS 12700 NORTH HIGHWAY 1 MENDOCINO, CALIFORNIA

Project Number 13646.02

January 10, 2024

Engineers and Geologists

GEOTECHNICAL INVESTIGATION

BALDERSTON FOUNDATION IMPROVEMENTS 12700 NORTH HIGHWAY 1 MENDOCINO, CALIFORNIA

Project Number 13646.02

prepared for

Daniel Balderson

prepared by

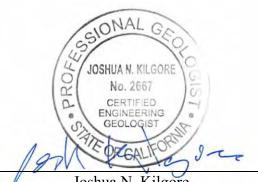
Brunsing Associates, Inc.

5468 Skylane Blvd. Suite 201 Santa Rosa, CA 95403 (707) 528-6108

January 10, 2024



Keith A. Colorado Geotechnical Engineer - 2894 kcolorado@brunsing.com



Joshua N. Kilgore Engineering Geologist - 2667 jkilgore@brunsing.com



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13646.02

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1.0 INTRODUCTION

This report presents the results of the geotechnical investigation that Brunsing Associates, Inc. (BAI) has performed for the proposed foundation improvements at the Balderston residence located at 12700 North Highway 1, Mendocino, California. The site is shown on the Vicinity Map, Plate 1.

The purpose of our investigation was to evaluate the site soil, bedrock, and geologic conditions to provide conclusions and recommendations for the improvement of the existing foundation, and associated construction considerations. Our approach to providing the geotechnical guidelines for the design of the project utilized our knowledge of the soil, bedrock and geologic conditions in the site vicinity, and experience with similar projects in the area. Field exploration for this investigation was directed toward confirming anticipated geologic conditions to provide the basis for our conclusions and recommendations.

As outlined in our Change Order No. 1, dated September 5, 2023, our scope of services for the geotechnical investigation included, subsurface exploration, and engineering and geologic analyses, to provide conclusions and recommendations for support of the existing residence.

2.0 INVESTIGATION AND LABORATORY TESTING

2.1 Research

As part of our investigation, we reviewed published geotechnical literature, including geologic, fault and seismic hazard maps for the site and vicinity. A list of selected published references reviewed for this investigation is presented in Appendix A.

2.2 Field Reconnaissance

BAI's senior engineering geologist performed an initial reconnaissance of the property and provided the results in a letter dated August 14, 2023. He observed access conditions and constraints and observed areas of the residence. He also observed the surrounding area and photographed the site.

2.3 Field Exploration

Our subsurface exploration on September 19, 2023, consisted of drilling, logging, and sampling three auger test borings, B-1 through B-3. The test borings were approximately 12 to 20.4 feet in depth. The borings were drilled and sampled with a track mounted drill rig using 4.5-inch solid stem flight augers.

Our senior engineering geologist logged the borings and obtained relatively undisturbed soil and bedrock samples using a 3.0-inch (CA) outside diameter, modified California split-barrel samplers, along with a 2-inch outside diameter Standard Penetration Test (SPT) split-barrel sampler. The inside of the sampler barrels contained liners for retaining the soil and bedrock samples. The samplers were driven by a 140-pound drop hammer falling 30 inches per blow. Blows required to



drive the CA sampler were converted to Standard Penetration Test (SPT) blow counts for correlation with empirical test data. The conversion factor of 0.64 was used for the CA sampler. SPT blow counts provide a relative measure of soil and bedrock consistency and strength and are utilized in our engineering analyses. Blow counts are presented on the boring logs alongside the sample locations.

The test boring locations are shown on the Site Map, Plate 2. The logs of the test borings showing the various soil and bedrock materials encountered and the depths at which samples were obtained are presented on Plates 3 through 5. The soils are classified in accordance with the Unified Soil Classification System outlined on Plate 6. The soil and bedrock descriptive properties are presented on Plates 7 and 8, respectively.

2.4 Laboratory Testing

Soil and bedrock samples obtained during our subsurface exploration were transported to our laboratory and examined to confirm field classifications. Laboratory tests were performed on selected samples to estimate their pertinent geotechnical engineering characteristics. Laboratory testing consisted of moisture content, dry density, grain size, expansion index and unconsolidated-undrained triaxial compression tests.

The laboratory test results are presented opposite the samples tested on the test pit logs. A key to test data is provided on Plate 7. Expansion index test results are presented on Plate 9 and triaxial compression test results are presented on Plate 10.

3.0 SITE CONDITIONS

The subject property is located due north of Jack Peters Gulch, on the north and northeast sides of Mendocino County Road 5000 (Former Coast Highway). The site is developed with two residential structures and 3 outbuildings, as shown on Plate 2. The two residences consist of a main house on an open slope and a guest house to the northwest at the edge of a forest. This report addresses the main house located within the southern extent of the property. The property is bounded by California Highway 1 to the east, Mendocino County Road 5000 (RD 5000) and an existing residence to the south, RD 5000 and an undeveloped property to the west and undeveloped wooded terrain to the north. Based on information provided by you, it is our understanding that the southernmost residence was originally located to the north within the CA Highway 1 alignment but was moved to its present location during the re-routing of Highway 1.

The subject property consists of an irregularly shaped parcel which is generally grass covered with mature trees along the perimeter of the property. The residences are located near the center of the property and are accessed by a gravel driveway which extends north and east from RD 5000. The driveway has several "repaired areas" that may have been a result of seasonal seepage. Three outbuildings consisting of wooden sheds are located northeast of the residences.

The property consists of gently west sloping terrain with approximate gradients of 10H:1V (horizontal to vertical) to 15H:1V. Based on Google earth imagery, the northernmost residence is



located approximately 150 feet to the east of the ocean bluff, the southernmost residence is located approximately 130 feet to the east of the bluff.

The coastal bluffs located southwest of the subject property appear to be nearly vertical, and approximately 70 vertical feet in height. The lower 50 to 60 feet of bluffs appeared to consist of resistant bedrock capped by approximately 10 feet or more of Pleistocene terrace deposits. Mature vegetation was observed along the bluff edge. The base of the bluff consists of a rocky beach and areas of resistant bedrock and boulder. During our initial site reconnaissance, we made visual observations of the bluff top. A review of Coastal Records Project imagery of these bluffs shows little changes between 1972 and 2019.

In general, the observed structures appear to have been constructed at grade, however some evidence of previous site grading was observed in the vicinity of the southernmost residence. Surficial observations of the residential foundations indicate that the houses are supported on a perimeter foundation. The exposed portions of the foundations of the northernmost structure appeared in good condition. Observations of the southernmost structure noted areas of distressed and cracked foundations and stem walls along the northwest and southwest perimeter of the structure. In addition, cracking of interior walls was observed within the eastern extent of the structure. The cracking of the foundations consisted of cracks with separation of up to ½ inch.

An excavation adjacent to the southwest corner of the foundation was advanced to determine the existing foundation depth in this area. The foundation was found to extend approximately 2 feet below grade and appeared to be supported on fill material placed at some time prior to the foundation construction.

No standing water was observed at the site during our surface and subsurface field explorations. Groundwater was not encountered in our test borings at the site. Temporarily perched groundwater may occur within one to two feet of the surface during (and just following) wet weather periods.

4.0 SITE GEOLOGY AND SOILS

The site is underlain by Pleistocene Epoch terrace deposits overlying Cretaceous-Tertiary Period coastal belt Franciscan Complex bedrock consisting of shale and sandstone. The bedrock was encountered at about eight to 15 feet within our borings. The bedrock with borings B-1 and B-3 is gray shale, and within boring B-3 is gray-brown sandstone. The bedrock is crushed to intensely fractured, friable to low hardness, and deeply to moderately weathered.

The property is situated on an elevated marine terrace that was formed during the Pleistocene Epoch, when glaciation caused sea level fluctuations, which created a series of steps, or terraces, cut into the coastal bedrock by wave erosion. Shallow marine sediments (Pleistocene terrace deposits) were deposited on the wave-cut, bedrock platforms while they were submerged beneath the ocean. Some of these marine deposits have been locally eroded as the terrace began to emerge from the ocean approximately 12,000 years ago. Present sea levels were achieved about 5,000 to 7,000 years ago.



Approximately eight to 15 feet of fill and terrace deposits were encountered in our test borings. The fill material was encountered to a depth of about 3.5 to 8.0 feet. The fill consists of brown to dark brown loose to medium dense clayey sand to silty sand and stiff, sandy clay. Under the fill is mottled gray-brown to orange-brown clayey sand to sandy clay and silty sand to a depth of about eight to 15 feet.

No evidence of landsliding was found at the property. No evidence of active faulting was observed at the site and none of the published references that we reviewed for this study show active faults on, or trending towards, the property.

Mendocino County is within a zone of potentially high seismic activity related to the San Andreas Fault system located offshore, approximately 5 miles southwest of the site. Future damaging earthquakes could occur on the San Andreas Fault during the lifetime of the existing structure. In general, the intensity of ground shaking at the site will depend on the distance to the causative earthquake epicenter, the magnitude of the shock, and the response characteristics of the underlying earth materials.

5.0 DISCUSSIONS AND CONCLUSIONS

5.1 General

Based upon the results of our investigation, it appears that the residence is supported on undocumented fill which was placed by unknown methods at some time prior to the construction of the residence. In addition, the underlying terrace deposits are variable in composition and are subject to downslope creep during periods of seasonal moisture variation. To adequately stabilize the residential foundation, we judge that foundation support should be obtained from the supporting soils and underlying bedrock which were encountered at depths between 8 and 10 feet below the surface. It is our opinion that foundation support can be most adequately achieved by supporting the structure on a drilled pier and grade beam foundation system. A structural engineer should evaluate the existing foundations for design of the connections from the new piers to the existing footings. We anticipate that the piers will be subject to creep forces withing the upper eight feet of embedment. Foundation support should be designed to resist these creep forces. The main geotechnical considerations affecting the design and construction of the project are undocumented fill, settlement, creep forces, and strong seismic shaking. These constraints are discussed in the following subsections.

5.2 Settlement

Assuming the foundation improvements are designed and constructed in accordance with our recommendations, we estimate that the post-construction settlement due to foundation loads will be less than $\frac{3}{4}$ inch. We estimate the post-construction differential settlement will be less than $\frac{1}{2}$ inch.



5.3 Undocumented Fill

Planned foundations improvements supported directly on the undocumented fills would undergo erratic and excessive settlement. This fill extended to between 3.5 and 8.0 feet below the existing surface and may be subject to collapse when loaded in a saturated condition.

5.4 Soil Creep

The upper portions of foundations improvements should be designed to resist soil creep forces related to downslope movement of plastic and fill soils. We anticipate these creep forces will act on the upper eight feet of the foundation elements.

5.5 Seismicity and Faulting

As is typical of the Mendocino County area, the site will be subject to strong ground shaking during future, nearby, large magnitude earthquakes. The intensity of ground shaking at the site will depend on the distance to the causative earthquake epicenter, the magnitude of the shock, and the response characteristics of the underlying earth materials. Generally, structures founded in supporting materials and designed in accordance with current building codes are well suited to resist the effects of ground shaking.

No evidence of faulting was observed by BAI or shown in the site vicinity on the published geologic maps that we reviewed for this investigation. Therefore, the potential for fault rupture at the site is considered low.

6.0 **RECOMMENDATIONS**

6.1 Foundation Underpinning / Drilled Pier and Grade Beam

Support for the residence can be provided by a system of drilled cast in place concrete piers interconnected by a grade beam. Drilled piers should be at least 12 inches in diameter and gain support from the underlying bedrock beyond the zone of anticipated creep forces. The piers should extend a minimum of 5 feet into bedrock which was encountered at depths between 8.0 to 15 feet below the existing surface. Minimum pier depths of 13 to 20 feet below existing grade should be anticipated. Pier depth and diameter should be determined by a structural engineer based on our recommendations.

Pier spacing should be no closer than 3 pier diameters, center to center. The drilled piers should be designed to gain support from skin friction. A skin friction value of 400 pounds per square foot (psf) of shaft area may be used in the supporting soils, for dead loads plus live loads. A skin friction value of 700 psf of shaft area may be used in the supporting bedrock, for dead loads plus live loads. For total downward loads due to wind or seismic forces, the pier capacity can be increased by one third. Uplift frictional capacity for piers should be limited to 2/3 of the allowable downward capacity. When final pier depths have been achieved, as determined by BAI, the bottoms of the pier holes should be cleaned of loose materials. BAI should observe the drilling and final clean out of the pier holes, prior to the placement of reinforcing steel and/or concrete.



Resistance to lateral loads can be obtained using passive earth pressure against the face of the foundations. An allowable passive pressure of 250 psf (rectangular distribution) can be used within the supporting soils. An allowable passive pressure of 450 psf (rectangular distribution) can be used within the supporting bedrock. In addition to the structure loads, the piers should be designed to resist creep forces equal to 75 psf (rectangular distribution) over a depth of eight feet below ground surface. Passive pressure should be neglected within the upper eight feet. Passive pressure can be projected over two pier diameters.

If groundwater is encountered during construction, the pier holes should be dewatered prior to placement of reinforcing steel and concrete. Alternatively, if more than six inches of groundwater has entered the pier hole, concrete can be tremied into place with an adequate head to displace water or slurry. Concrete should not be placed by freefall in such a manner as to hit the sidewalls of the excavation.

6.2 Seismic Design Criteria

The structures should be designed and/or constructed to resist the effects of strong ground shaking (in the order of Modified Mercalli Intensity IX) in accordance with current building codes. The California Building Code (CBC) 2022 edition indicates that the site classification for the property is Site Class C. CBC indicates that the following seismic design parameters are appropriate for the site:

Site Class	=	С
Mapped Spectral Response Acceleration at 0.2 sec	Ss =	1.901g
Mapped Spectral Response Acceleration at 1.0 sec	$S_1 =$	0.781g
Modified Spectral Response Acceleration at 0.2 sec	$S_{MS} =$	2.282g
Modified Spectral Response Acceleration at 1.0 sec	$S_{M1} =$	1.093g
Design Spectral Response Acceleration at 0.2 sec	$S_{DS} =$	1.521g
Design Spectral Response Acceleration at 1.0 sec	$S_{D1} =$	0.729g
Site Coefficient	F _a =	1.2
Site Coefficient	$F_v =$	1.4
Long Period Transition Period	T _L =	12
Seismic Design Category	=	Е

Table 2: Seismic Design Parameters

6.3 Site Drainage

Because surface and/or subsurface water is often the cause of foundation or interior moisture problems, care should be taken to intercept and divert concentrated surface flows and subsurface seepage away from the building foundations. Drainage across the lot should be by sheet-flow. Surface grades should maintain a recommended five percent gradient away from building foundations. To minimize the chance for moisture inundation of the near surface soils, we recommend that a perimeter foundation drain be installed around the perimeter foundation elements, as shown on Plate 11



7.0 ADDITIONAL SERVICES

Prior to construction, BAI should review the final grading and foundation plans, and geotechnical related specifications for conformance with our recommendations.

During construction, BAI should be retained to provide periodic observations, together with field and laboratory testing, during site preparation, placement, and compaction of fills, if required, and foundation construction. Foundation excavations should be reviewed by BAI while the excavation operations are being performed. Our reviews and tests would allow us to verify conformance of the work to project guidelines, determine that soil conditions are as anticipated, and to modify our recommendations, if necessary.

8.0 LIMITATIONS

This geotechnical investigation was performed in accordance with the usual and current standards of the profession, as they relate to this and similar localities. No other warranty, expressed or implied, is provided as to the conclusions and professional advice presented in this report. Our conclusions are based upon reasonable geological and engineering interpretation of available data.

The samples taken and tested, and the observations made, are representative of the site; however, soil and geologic conditions may vary significantly between borings and across the site. As in most projects, conditions revealed during construction excavation may be at variance with preliminary findings. If this occurs, the changed conditions must be evaluated by BAI, and revised recommendations be provided as required.

This report is issued with the understanding that it is the responsibility of the Owner, or his/her representative, to ensure that the information and recommendations contained herein are brought to the attention of all other design professionals for the project, and incorporated into the plans, and that the Contractor and Subcontractors implement such recommendations in the field. The safety of others is the responsibility of the Contractor. The Contractor should notify the owner and BAI if he/she considers any of the recommended actions presented herein to be unsafe or otherwise impractical.

Changes in the condition of a site can occur with the passage of time, whether they are due to natural events or to human activities on this, or adjacent sites. In addition, changes in applicable or appropriate codes and standards may occur, whether they result from legislation or the broadening of knowledge. Accordingly, this report may become invalidated wholly or partially by changes outside of our control. Therefore, this report is subject to review and revision as changed conditions are identified.

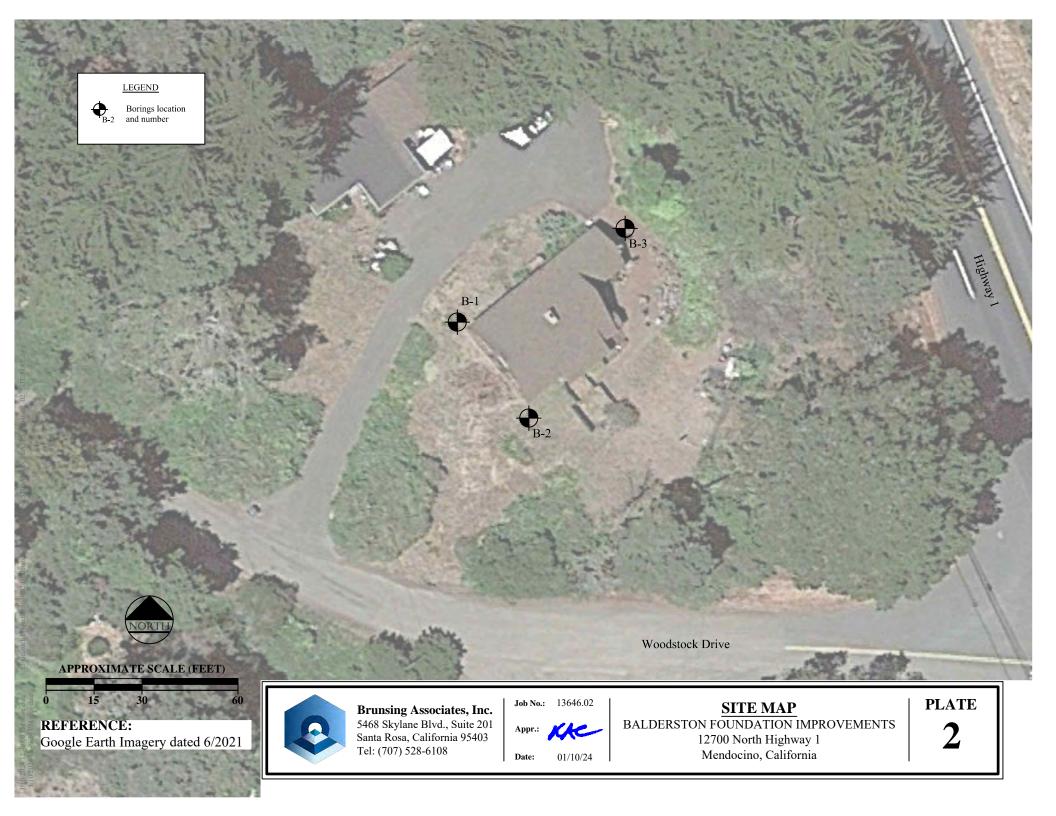
The recommendations contained in this report are based on certain specific project information regarding type of construction and current structure locations, which have been made available to us. If conceptual changes are undertaken during final project design, we should be allowed to review them in light of this report to determine if our recommendations are still applicable.



PLATES



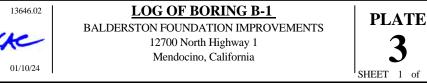




Laboratory Tests	Sampler Type* Moisture Content (%) Dry Density (pcf) Blows/foot	Log of Boring B-1 Equipment: McMillen X975W; 4.5-inch solid-stem flight aug Date: 9/19/23 Logged By: JNK Elevation: 94 feet *** Latitude: 39.321205 Longitude: -12	-
Tx 1978 (720)	CA 27.6 80 10**	DARK BROWN CLAYEY SAND (SC) loose to medium stiff, dry to moist, porous to 2.5 feet	
	CA 14 ** CA 13 **	4 - 5 - 2 6 - 0 7 MOTTLED GRAY-BROWN TO ORANGE CLAYEY SAND (SC	:)
Tx 2583 (1080) 48% Passing #200	CA 23.1 99 12 **	medium dense, moist	,
Tx 2698 (1440)	CA 20.9 107 17**	12 13 14 15 GRAY SHALE	
		crushed to intensely fractured, friable to low hardness, deeply moderately weathered	to
	CA 26/1" ** SPT 75/4"	20 - Notes: 1. No free water encountered 2. No caving 3. Practical drilling refusal at 20.4 feet	

Latitude/Longitude estimated from Google Earth. * See Soil Classification Chart & Key to Test Data ** Equivalent "Standard Penetration" Blow Counts. *** Elevations interpolated from USGS LiDAR - FEMA 2016. Brunsing Associates, Inc. 5468 Stylene Blyd, Suite 201

Brunsing Associates, Inc. 5468 Skylane Blvd., Suite 201 Santa Rosa, California 95403 Tel: (707) 528-6108 Job No.: 13646.02 Appr.: Date: 01/10/24



Scale: 1" = 4'

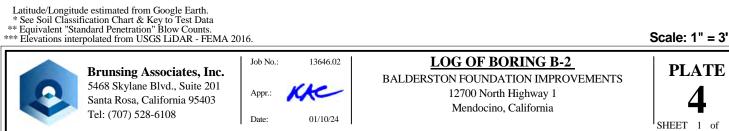
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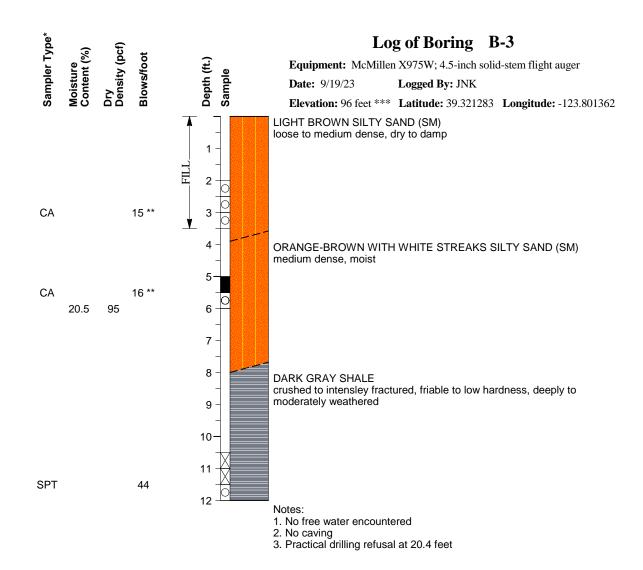
	Sampler Type*	Moisture Content (%)	Dry Density (pcf)	Blows/foot	Depth (ft.) Sample	Log of BoringB-2Equipment:McMillen X975W; 4.5-inch solid-stem flight augerDate:9/19/23Logged By:JNK
Laboratory Tests	Sai	°S	D D E O	Blo	Del	Elevation: 94 feet *** Latitude: 39.321131 Longitude: -123.801454
·						BROWN CLAYEY SAND (SC) loose to medium dense, dry to moist with bricks and debris
	CA	22.2	87	17 **	3 0	
						MOTTLED GRAY, ORANGE-BROWN SANDY CLAY (CL) stiff, moist
Tx 3591 (1008)	СА	27.1	93	15 **		
					7 -	
					▲ 8 - - 9 - -	LIGHT BROWN TO ORANGE-BROWN SILTY SAND (SM) medium dense to dense, moist
	CA	16.6	108	44 **		
					12 -	
	SPT			50/6"		GRAY-BROWN SANDSTONE crushed to intensely fractured, low hardness, deeply to moderately weathered
						Notes: 1. No free water encountered 2. No caving

- No caving
 Practical drilling refusal at 14.5 feet

13646.01 GINT.GPJ, 1/10/24

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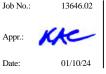
Latitude/Longitude estimated from Google Earth. * See Soil Classification Chart & Key to Test Data ** Equivalent "Standard Penetration" Blow Counts. *** Elevations interpolated from USGS LiDAR - FEMA 2016.



Scale: 1" = 3'



5468 Skylane Blvd., Suite 201 Santa Rosa, California 95403





			10	SYM	BOLS	TYPICAL	
		MAJOR DIVISION	15	GRAPHIC LETTER		DESCRIPTIONS	
		GRAVELS AND	CLEAN GRAVELS		GW	WELL-GRADED GRAVELS, GRAVEL-SAND MIXTURES, LITTLE OR NO FINES	
	COARSE- GRAINED	GRAVELLY SOILS	(Less than 5% fines)		GP	POORLY-GRADED GRAVELS, GRAVEL-SAND MIXTURES, LITTLE OR NO FINES	
cs)	SOILS	MORE THAN 50% OF COARSE	GRAVELS WITH FINES		GM	SILTY GRAVELS, GRAVEL-SAND-SILT MIXTURES	
SYSTEM (USCS)		FRACTION RETAINED ON NO. 4 SIEVE	(Greater than 12% fines)		GC	CLAYEY GRAVELS, GRAVEL-SAND-CLAY MIXTURES	
STEN		SAND AND	CLEAN SANDS		sw	WELL-GRADED SANDS, GRAVELLY SANDS, LITTLE OR NO FINES	
	MORE THAN 50% OF MATERIAL IS	SANDY SOILS	(Less than 5% fines)		SP	POORLY-GRADED SANDS, GRAVELLY SANDS, LITTLE OR NO FINES	
CLASSIFICATION	LARGER THAN NO. 200 SIEVE SIZE	50% OR MORE OF COARSE FRACTION PASSING	SANDS WITH FINES		SM	SILTY SANDS, SAND-SILT MIXTURES	
SIFIC		THROUGH NO. 4 SIEVE	(Greater than 12% fines)		sc	CLAYEY SANDS, SAND-CLAY MIXTURES	
CLAS	FINE- GRAINED SOILS	SILTS AND CLAYS			ML	INORGANIC SILTS AND VERY FINE SANDS, ROCK FLOUR, SILTY OR CLAYEY FINE SANDS OR CLAYEY SILTS WITH SLIGHT PLASTICITY	
SOIL (LIQUID LIMIT LESS THAN 50		CL	INORGANIC CLAYS OF LOW TO MEDIUM PLASTICITY, GRAVELLY CLAYS, SANDY CLAYS, SILTY CLAYS, LEAN CLAYS	
	00120				OL	ORGANIC SILTS AND ORGANIC SILTY CLAYS OF LOW PLASTICITY	
UNIFIED					МН	INORGANIC SILT, MICACEOUS OR DIATOMACEOUS FINE SAND OR SILTY SOILS	
	MORE THAN 50% OF MATERIAL IS SMALLER THAN NO. 200 SIEVE SIZE	SILTS AND CLAYS	LIQUID LIMIT GREATER THAN 50		СН	INORGANIC CLAYS OF HIGH PLASTICITY	
					ОН	ORGANIC CLAYS OF MEDIUM TO HIGH PLASTICITY, ORGANIC SILTS	
	HIGHLY ORGANIC SOILS				РТ	PEAT, HUMOUS, SWAMP SOILS WITH HIGH ORGANIC CONTENTS	
			KEY TO TES	ST DA	ATA		
LL	•	Consol - Consolida		Strength	n, psf 1	f Confining Pressure, psf	
PI		EI - Expansion Ind				4 (1440) - Unconsolidated Undrained Triaxial	
	Sample Retained	SA - Sieve Analys	IS			4 (1440) - Consolidated Undrained Triaxial	
	•	NUL RELAINED			DS 202 VS 520	0 (1440) - Consolidated Drained Direct Shear - Field Vane Shear	
		ered			VS 520 JC 150		
CA		Split Barrel Sampler 3.	0-inch O.D.		P 150		
~					. 130		

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CM - California Modified Split Barrel Sampler 2.5-inch O.D.

SPT - California Split Barrel Sampler 2.0-inch O.D.

SH - Shelby Tube

RC - Rock Coring

Recovery - Percent Core Recovered

C 01/10/24 Date:

SOIL CLASSIFICATION CHART & KEY TO TEST DATA 13646.02 BALDERSTON FOUNDATION IMPROVEMENTS

Sat

PLATE 12700 North Highway 1 Mendocino, California

- Sample saturated prior to test

 $\underline{\bigtriangledown}$ Initial Groundwater Level Reading

Second Groundwater Level Reading

6

Tel: (707) 528-6108

Appr.:

Job No.:

RQD - Rock Quality Designation (length of core pieces >= 4-inches / core length)

RELATIVE DENSITY OF COARSE-GRAINED SOILS

Relative Density

Standard Penetration Test Blow Count (blows per foot)

Very loose Loose Medium dense Dense Very dense

4 or less 5 to 10 11 to 30 31 to 50 More than 50

CONSISTENCY OF FINE-GRAINED SOILS

Consistency	istency Identification Procedure		
Very soft	Easily penetrated several inches with fist	Less than 250	
Soft	Easily penetrated several inches with thumb	250 to 500	
Medium stiff	Penetrated several inches by thumb with moderate effort	500 to 1000	
Stiff	Readily indented by thumb, but penetrated only with great effort	1000 to 2000	
Very stiff	Readily indented by thumb nail	2000 to 4000	
Hard	indented with difficulty by thumb nail	More than 4000	

NATURAL MOISTURE CONTENT

	Dry	No noticeable moisture content. Requires considerable moisture to obtain optimum moisture content* for compaction.
	Damp	Contains some moisture, but is on the dry side of optimum.
	Moist	Near optimum moisture content for compaction.
	Wet	Requires drying to obtain optimum moisture content for compaction.
S	Saturated	Near or below the water table, from capillarity, or from perched or ponded water. All void spaces filled with water.
	.	

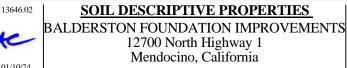
* Optimum moisture content as determined in accordance with ASTM Test Method D1557, latest edition.

Where laboratory test data are not available, the above field classifications provide a general indication of material properties; the classifications may require modification based upon laboratory tests.



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PLATE

Generalized Graphic Bedrock Symbols



Claystone



Siltstone





Andesite



Shale



Chert



Sandstone



Serpentine





Conglomerate



Greenstone



Schist

Stratification

Bedding of Sedimentary Rocks Massive

Very thick bedded Thick bedded Thin bedded Very thin bedded Laminated Thinly laminated

Thickness of Beds No apparent bedding Greater than 4 feet 2 feet to 4 feet 2 inches to 2 feet 0.5 inches to 2 inches 0.125 inches to 0.5 inches less than 0.125 inches

Fracturing

Fracturing Intensity Little Occasional Moderate Close Intense Crushed

Fracture Spacing Greater than 4 feet 1 foot to 4 feet 6 inches to 1 foot 1 inch to 6 inches 0.5 inches to 1 inch less than 0.5 inches

Strenath

Soft	Plastic or very low strength.
Friable	Crumbles by hand.
Low hardness	Crumbles under light hammer blows.
Moderate hardness	Crumbles under a few heavy hammer blows.
Hard	Breaks into large pieces under heavy, ringing hammer blows.
Very hard	Resists heavy, ringing hammer blows and will yield with difficulty only dust and small flying fragments.

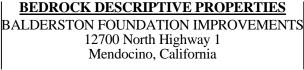
Weathering

- Moderate to complete mineral decomposition, extensive disintegration, deep and Deep thorough discoloration, many extensively coated fractures.
- Moderate Slight decomposition of minerals, little disintegration, moderate discoloration, moderately coated fractures.
- Little No megascopic decomposition of minerals, slight to no effect on cementation, slight and intermittent, or localized discoloration, few stains on fracture surfaces.
- Fresh Unaffected by weathering agents, no disintegration or discoloration, fractures usually less numerous than joints.

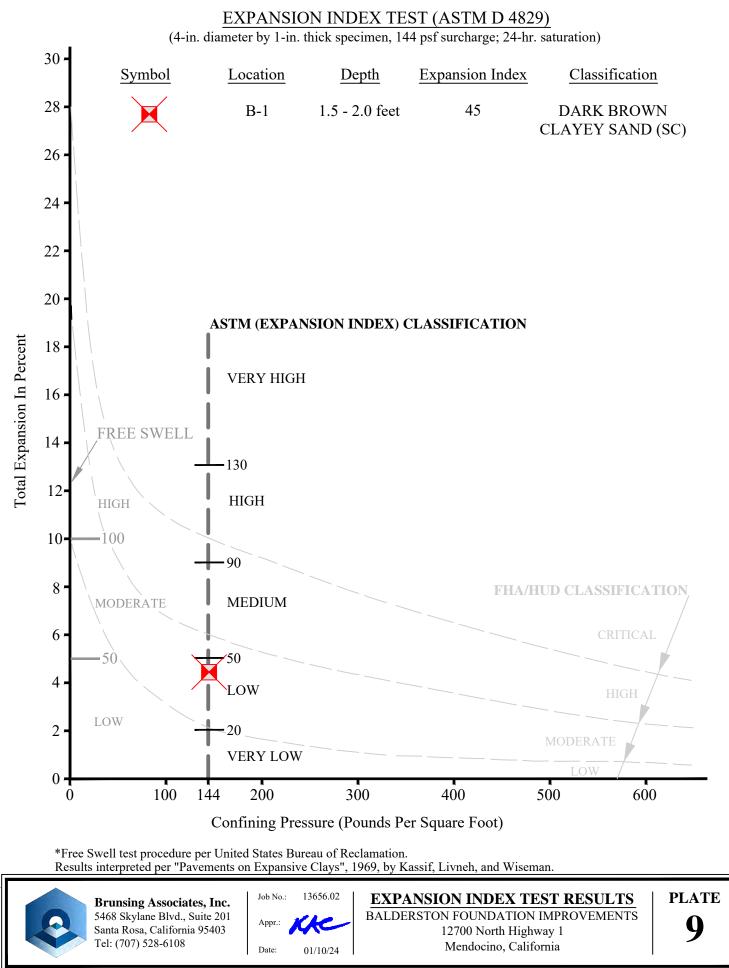


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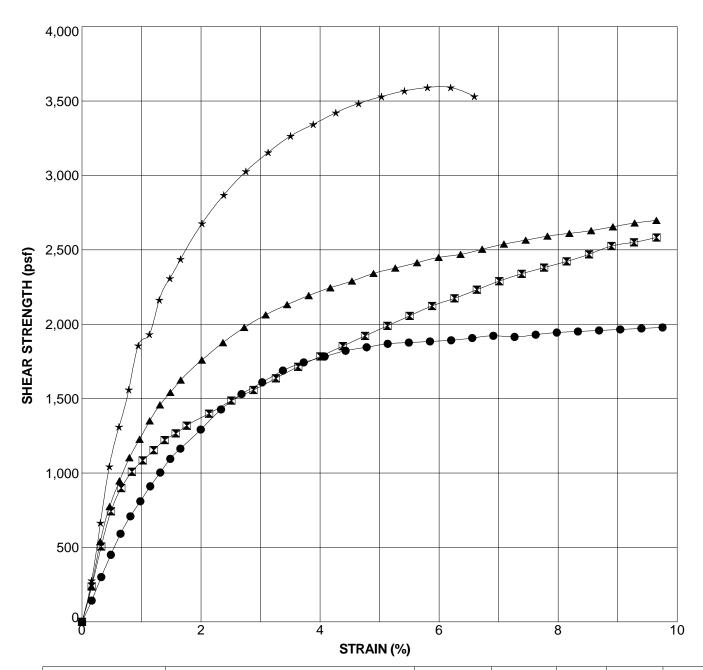








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Sample Source	Classification	Confining Pressure (psf)	Ultimate Strength (psf)	Strain (%)	Dry Density (pcf)	Moisture Content (%)
● B-1 at 2 ft	DARK BROWN CLAYEY SAND (SC)	720	1978	9.8	80	27.6
I B-1 at 9 ft	MOTTLED GRAY-BROWN TO ORANGE CLAYEY SAND (SC)	1080	2583	9.7	99	23.1
▲ B-1 at 13 ft	MOTTLED GRAY-BROWN TO ORANGE CLAYEY SAND (SC)	1440	2698	9.7	107	20.9
★ B-2 at 6 ft	MOTTLED GRAY, ORANGE-BROWN SANDY CLAY (CL)	1008	3591	6.2	93	27.1

0

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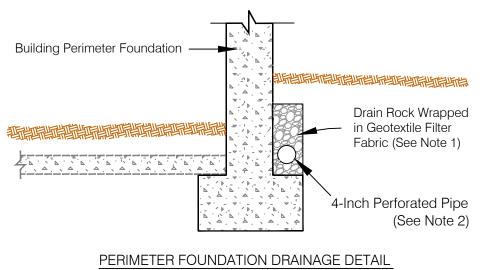


13646.02

Job No.:

UNCONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST RESULTS BALDERSTON FOUNDATION IMPROVEMENTS 12700 North Highway 1 Mendocino, California





(Not to Scale)

NOTES:

- 1. Drain rock should be clean, free-draining 3/4 inch crushed rock. Perimeter foundation drain rock should be wrapped in a non-woven geotextile filter fabric (Mirafi 140N or equivalent) or Class 2 permeable material, without filter fabric, per Caltrans standard specifications, latest edition.
- 2. Pipe should be SDR 35 or equivalent, placed with perforations down, sloped at least 1 percent to gravity outlet or sump with automatic pump.
- 3. A clean-out pipe with cap should be installed at the up-slope end of the pipe, pipe elbows should be 45 degrees or less (for "snake" access).



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Job No.: 13646.02 Appr.: Date: 01/10/24

PERIMETER FOUNDATION DRAINAGE DETAILS BALDERSTON FOUNDATION IMPROVEMENTS

PLATE 12700 North Highway 1 Mendocino, California

11

APPENDIX A

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