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The background of the cover is a photograph of a two-lane asphalt road winding through a dense forest of tall evergreen trees. The road curves to the right in the distance. A few cars are visible on the road. The overall scene is bright and somewhat hazy, suggesting a sunny day with some atmospheric haze or mist. The text is overlaid on this image.

Updated Supplemental Traffic Impact Analysis for the Harris Quarry

County of Mendocino

January 27, 2010

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Introduction and Setting

Introduction

This report provides an analysis of the impacts on the circulation system associated with the proposed project at Harris Quarry which is located in Mendocino County south of the City of Willits. This analysis is an update to a report that supplements the Draft Environmental Impact Report (DEIR) for the Harris Quarry Use Permit and reflects conditions with the continuation of existing aggregate hauling activities from the Quarry for the horizon years of 2010, 2014 (both with and without the US 101 Willits bypass construction project in progress), 2030 and 2040. For each horizon year project traffic impacts have been determined for peak production activity in July and October respectively. Impacts have been evaluated for the peak traffic hours on US 101 (Redwood Highway) for the time periods of 9:00 to 10:00 a.m., 11:00 a.m. to noon, 1:15 to 2:15 p.m. and 4:45 to 5:45 p.m. Harris Quarry currently operates on weekdays with no regular operations projected for week nights or weekends. On rare occasions there may be overnight or weekend operations, but this would only occur when there is an emergency need for aggregate. Measures have been identified to mitigate significant impacts of the proposed project. New information provided in this update includes analysis for the horizon years of 2030 and 2040 as well as conditions that would be expected to occur if the quarry production was limited to currently permitted levels, regardless of actual historical production levels, and without the asphalt concrete (AC) plant which is part of the proposed project. Additionally, an analysis of vehicle miles traveled was completed and included. The traffic study was completed in accordance with criteria established by the County and the State, and is consistent with standard traffic engineering techniques. Issues such as intersection level of service, sight distance and safety were evaluated.

Setting

Quarry Location and Access

Harris Quarry is located on the west side of US 101 approximately 500 feet south of Black Bart Drive, south of the City of Willits in Mendocino County. Access to the Quarry is currently provided via a single access drive on the west side of US 101. There are no intersection controls along US 101, with stop sign controls on the side streets. The project site and study intersections are shown on Figure 1.

US 101 in the project vicinity has two travel lanes in each direction. It is an expressway and has at-grade intersections with some local roads and major driveways. US 101 is classified as “rural – other principal arterial” in the Federal Highway Administration (FHWA) functional classification system and is shown in the California Department of Transportation (Caltrans) “California Roads System Maps.” The posted speed limit in the project area is 65 miles per hour (mph). US 101 has a moderate extended uphill grade south to north as it climbs out of the Ukiah Valley. It crests the mountain ridge separating the Russian River and Eel River watersheds just north of the intersection with Black Bart Drive. Northbound US 101 traffic is climbing an uphill grade through both the Harris Quarry and Black Bart Drive intersections, while southbound traffic is on a downhill grade through both intersections. No left or right turn deceleration or acceleration lanes currently exist on the US 101 approaches to the Harris Quarry entrance.

Black Bart Drive is a two-lane rural road extending to the west of US 101. It has a moderate east-to-west uphill grade, numerous horizontal curves and lacks shoulders in most locations. Black Bart Drive is stop-sign controlled on its single lane approach to US 101, and flares out at the intersection to permit a right turning



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Figure 1

Updated Supplemental Traffic Impact Analysis for the Harris Quarry

Study Area and Lane Configurations

County of Mendocino

motorist to bypass vehicles making left turns. Left and right turn deceleration lanes are provided on both US 101 approaches to the Black Bart Drive intersection, where all turn movements are also allowed; however, left and right turn acceleration lanes are not provided leaving Black Bart Drive. Land use along Black Bart Drive consists of single family residences. Although traffic from the Quarry is not expected to travel on Black Bart Drive, the street was included in the analysis due to its proximity to the Quarry entrance and the fact that accelerating or decelerating trucks will interact with vehicles entering or exiting Black Bart Drive.

Planned Improvements

There are currently no planned improvements to Black Bart Drive or the US 101/Black Bart Drive intersection by the Mendocino County Department of Transportation or Caltrans. The nearest planned and funded project is the Willits bypass which will start approximately three miles to the north of the Harris Quarry Driveway. This project has an estimated construction start date of January 2011 and is expected completion in 2015. This is relevant because if Harris Quarry is selected to provide the aggregate for the project, the distribution of truck trips entering or exiting the Quarry is expected to shift, with more trucks traveling north to the bypass project site. This project is not expected to alter the total number of trucks entering and exiting the Quarry.

Segment Speed

The prevailing speed of traffic on US 101 is an essential factor in determining the level of improvements needed at an intersection. The posted speed limit on US 101 is 65 miles per hour in the vicinity of the Harris Quarry access. Vehicle speeds were sampled on US 101 southerly of Black Bart Drive on March 11, 2008, during which time data were collected in both directions, northbound and southbound, as well as by inside, number one, and outside, number two, lanes. Table I summarizes statistics of the vehicle speeds measured, and speed survey spreadsheets are provided in Appendix A.

**Table I
Summary of Vehicle Speeds on US 101**

	Northbound		Southbound	
	Inside	Outside	Inside	Outside
Average Speed	66	61	65	60
85 th Percentile Speed	70	65	70	66
Pace Speed	60-69	57-66	60-69	58-67
Number Observed	28	67	35	64

Note: All speeds in miles per hour (mph)

As can be seen in Table I, the inside, or number one, lane had recorded vehicle speeds approximately five miles per hour faster than those observed in the outside, or number two, lane. This is expected as slower traffic is required to drive in the outer lane. The 85th percentile of observed speeds is often used to establish the speed limit; however, due to the type of facility, the California Vehicle Code (CVC) limits the speed limit to be not more than 65 miles per hour (CVC §22349). The speed sample was obtained at the

location of the Quarry driveway because in some situations the free flow speed of a roadway may exceed the speed limit defined by the CVC. For the purposes of this evaluation, a free flow speed of 65 miles per hour was deemed appropriate for evaluating the need for speed change lanes.

Collision History

The collision history for the study area was reviewed to determine any trends or patterns that may indicate a safety issue. Collision rates were calculated for the one-mile segment of US 101 extending one-half mile in either direction from Black Bart Drive based on the most recent records available from the California Highway Patrol and published in their Statewide Integrated Traffic Records System (SWITRS) reports (January 2004 through December 2008). For the five-year study period, the corridor experienced 16 collisions, translating to a calculated collision rate of 0.52 collisions per million vehicle miles (c/mvm). This is less than the statewide average of 1.65 c/mvm for similar facilities statewide, as indicated in *2007 Accident Data on California State Highways*, California Department of Transportation. One collision, or 6.3 percent of all reported collisions, resulted in a fatality, which is greater than the statewide average of 2.2 percent for similar facilities. However, the 31.3 percent of collisions that resulted in one or more injuries is less than the statewide average of 38.4 percent for similar facilities. Details of the collision data are provided in Appendix B.

The collision data was further examined to determine which collisions, if any, can be attributed to turning movements at the Harris Quarry Driveway. None of the collisions within the study segment were identified as involving large trucks or trucks with trailers. Additionally, none of the collisions during the five-year period were associated with a turning movement at the project driveway.

Analysis

Base Permit Traffic Volumes

Base turning movement counts at Black Bart Drive and the Harris Quarry access road were taken from the *Draft Environmental Impact Report for the Proposed Harris Quarry Use Permit and Reclamation Plan*, Leonard Charles and Associates (DEIR).

The primary factor related to the number of truck trips into and out of a quarry facility is the demand for aggregate and AC, which is highly influenced by the season and the level of construction activity. Demand will vary not only monthly, but also from day to day and hour to hour. To determine the seasonal truck trip generation from the site production estimates for each product were obtained from the quarry operator. Recently published environmental impact reports for the Canyon Rock and Blue Rock Quarries in Sonoma County were also reviewed. From the EIRs for the Canyon Rock and Blue Rock Quarries seasonal, weekly, peak daily and hourly production information was obtained. Table 2 summarizes the projected monthly production as a percent of the total annual production. As can be seen in Table 2, October has the highest level of production.

Table 2
Monthly Production

Month	Percentage for Year		Month	Percentage for Year	
	Aggregates	AC		Aggregates	AC
January	5.5%	2.0%	July	9.4%	12.0%
February	5.4%	2.0%	August	9.4%	17.0%
March	7.5%	2.0%	September	9.2%	17.0%
April	5.6%	3.0%	October	12.2%	17.0%
May	10.6%	3.0%	November	12.1%	15.0%
June	9.3%	6.0%	December	3.8%	4.0%

An important factor to be considered when determining potential traffic impacts of land uses with seasonal variations is the traffic volume on the adjacent street network. Caltrans maintains permanent volume count stations on various routes throughout the state and these figures are published monthly. The nearest count station location on US 101 is in Humboldt County, and the monthly variations in volumes are summarized in Table 3.

Table 3
Summary of Monthly Traffic Volumes at Count Station 109

Month	Average ADT	% of Average Annual ADT	Month	Average ADT	% of Average Annual ADT
January	2826	78.60%	July	4678	130.11%
February	2888	80.30%	August	4514	125.55%
March	3133	87.10%	September	4078	113.42%
April	3458	96.00%	October	3593	99.93%
May	3957	110.10%	November	3139	87.31%
June	4255	118.30%	December	2626	73.04%

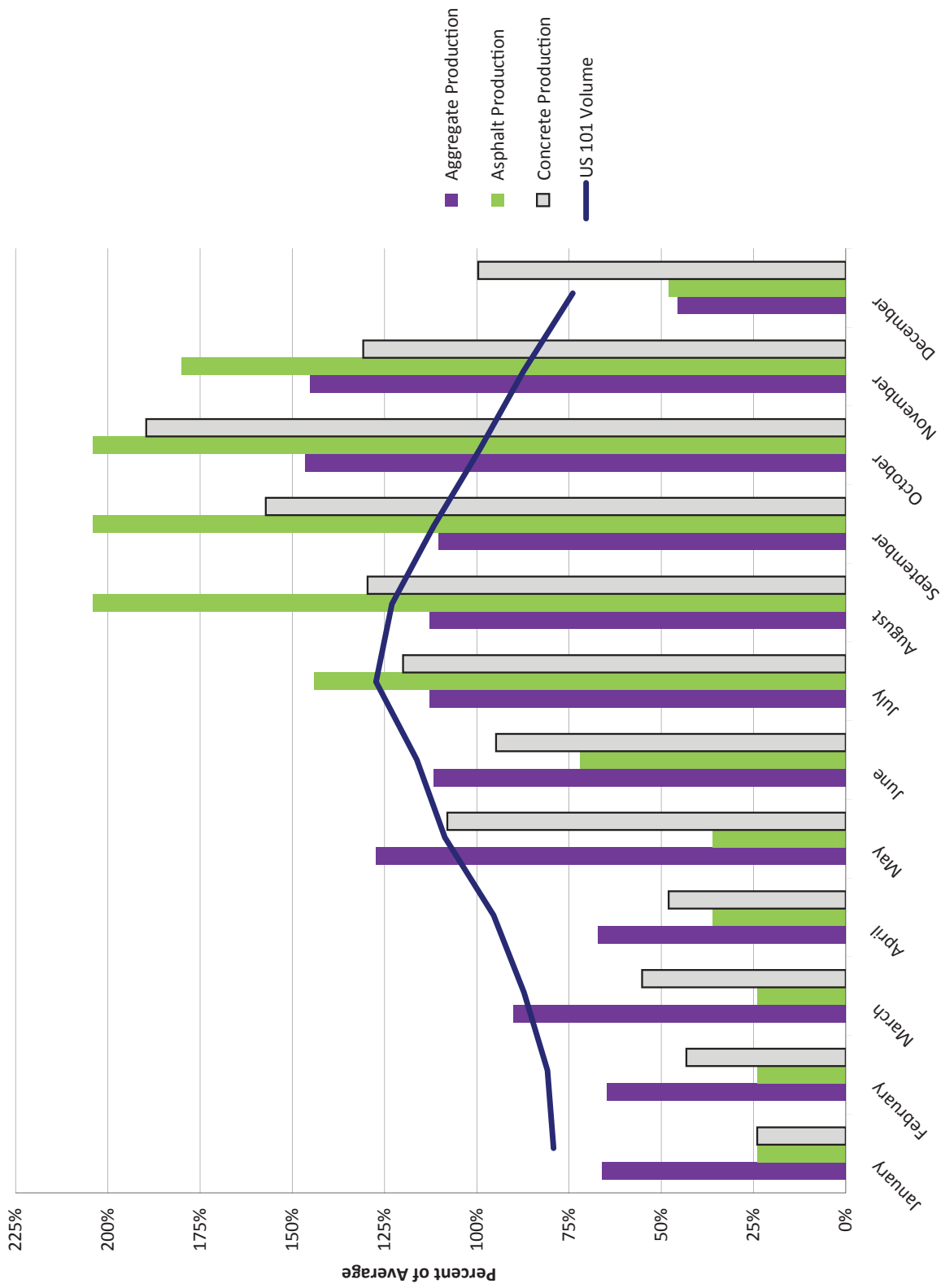
Note: ADT = Average Daily Traffic

As seen in Table 3, the month of July has the highest percentage of annual average traffic volumes. The relationship between the data presented in Tables 2 and 3 are shown graphically in Figure 2 and indicates the annual variation in production of the Quarry together with the variations in traffic volumes on US 101 as a percentage of average production and volume. Using these relationships, traffic volumes for July and October were developed based on traffic volume counts conducted near the Quarry entrance. Although these counts were obtained some distance north of the project area, the annual distribution of the trips is expected to be similar, making this data appropriate for the purpose of developing these ratios.

Base Assumptions

Several factors were considered when projecting truck trips to and from the Harris Quarry. A primary factor is the bulking that occurs when material is mined. For example, a solid cubic yard block of material when processed yields a larger volume of gravel because when the gravel is processed, it is not nearly as compact as a solid rock and contains many air voids, causing the same weight of material to occupy a larger volume. When the gravel is processed, it increases in volume by a factor of approximately 1.24 compared to solid rock. This factor was derived by dividing a typical unit weight of processed gravel by a typical unit weight of mined material.

The capacity of the truck will determine the number of trucks needed to haul a given quantity of material. Truck capacity was based upon a standard truck size of 20 cubic yards; however, due to varying truck size and/or varying demand for transporting gravel or AC it was assumed that not all loads would be 20 cubic yards, so an average of 16 cubic yards per truck was used for analysis purposes. Since aggregate transferred to Willits for concrete production is expected to be much more consistent, it was assumed that the entire truck capacity of 20 cubic yards would be utilized. The relationships between the data presented in Tables 2 and 3 are shown in Figure 2 and indicate the annual variation in production of the Quarry together with the variations in traffic volumes on US 101 as a percentage of average production and volume. Using the relationship between average production and peak Quarry production as shown on Page 7 of the DEIR it was determined that there is a 147 percent variance in production between average and peak conditions, or nearly 50 percent higher production during the peak month than on average. This ratio is consistent with expected typical traffic patterns experienced by various types of developments, therefore it is reasonable to conclude that during peak construction periods there will be times when the production of the Quarry



will be increased by approximately 50 percent to meet these demands. Additionally, it was assumed that 25 percent of weekly traffic occurs on a peak day and that there is an average of 4.15 weeks per month after deducting eleven non-working days a year to account for holidays, inclement weather or closures for other reasons. These factors were based upon data published in EIRs for the Canyon Rock and Blue Rock Quarries in Sonoma County. Summarized in Table 4 are the factors used to project truck trips.

**Table 4
Production Conversion Factors**

Unit	Unit Factor
Unit Weight of Mined Material	4,185 lbs/cy or 0.48 cy/ton
Unit Weight of Processed Gravel	3,375 lbs/cy or 0.59 cy/ton
Bulking Factor (Row 1/Row 2)	1.24
Average Truck Capacity Gravel-AC	16 cy/truck
Average Truck Capacity Concrete Aggregate	20 cy/truck
Peak Production Level	1.47 x Average

Notes: cy = cubic yards, lbs = pounds, ton = 2,000 lbs

Existing Permit Conditions

For the Quarry to continue operating at its current levels of production, a permit renewal is necessary. To evaluate current production levels a Base Permit Conditions scenario was prepared to assess the impacts and mitigation measures needed under existing permitted aggregate production levels. The Harris Quarry currently operates under a permit allowing extraction of 75,000 cubic yards of material annually, resulting in 93,000 cubic yards per year of material hauled from the site after taking into account changes in volume associated with crushing the rock. Generally, the anticipated trip generation for proposed projects is estimated using standard rates published by the Institute of Transportation Engineers (ITE) in *Trip Generation*, 8th Edition, 2008. However, the publication does not contain information for quarries or similar developments, so assumptions were developed for use in determining average daily and peak daily truck volumes. These assumptions were based on the projected Quarry production for the months of July, which experiences 130.11 percent of the average traffic volume on US 101 and October, during which the highway experiences average traffic flows. Within each of these months, peak production for the Quarry was assumed to be 147 percent of the average. Since the Quarry is not expected to operate on weekends and will be occasionally closed for holidays or due to inclement weather, operation was based on 21 operating days per month. Average truck capacities of 16 cubic yards for aggregate and AC were used for the analysis. Details of the assumptions applied and copies of the calculation are provided in Appendix C.

Based upon this production level and the monthly production factors presented in Table 2, hourly truck trips into and out of the Harris Quarry site were projected for the average and peak July and October periods. These truck trips were determined by calculating the total production in a day, and distributing that amount throughout the day in a logical manner based on normal construction activity and demand for aggregate. During peak production one trip from each resource, such as fuel and asphalt oil, as well as delivery services such as the Postal Service, UPS and FedEx can be expected on any given day and as many as two deliveries could arrive during a single hour. To ensure a conservative analysis, one inbound and one

outbound truck trip were added to each hour between 7:00 a.m. and 4:00 p.m. Truck trips on an hourly basis under Existing Permit Conditions are indicated in Appendix D and turning movements at the project access are shown in Appendix E. As shown in Table 5, it is projected that the Harris Quarry currently generates 84 truck trips on an average day in July and 114 truck trips during a peak day in July. In October, Harris generates an average of 104 truck trips per day, with 150 truck trips on a peak day. The base permit levels were used to analyze traffic impacts that would be expected if the quarry production was limited to the permit amount, rather than using historical actual quarry production.

**Table 5
Truck Trips for Permitted Production (75,000 cy Mined)**

Time	July				October			
	Average Day		Peak Day		Average Day		Peak Day	
	In	Out	In	Out	In	Out	In	Out
6:00 – 7:00 AM	0	0	1	0	1	0	1	0
7:00 – 8:00 AM	4	4	5	6	5	6	7	8
8:00 – 9:00 AM	5	5	7	7	6	6	9	9
9:00 – 10:00 AM	5	5	7	7	6	6	9	9
10:00 – 11:00 AM	5	5	7	7	6	6	9	9
11:00 AM – Noon	5	5	7	7	6	6	9	9
Noon – 1:00 PM	4	4	6	6	5	5	8	8
1:00 – 2:00 PM	4	4	5	5	5	5	7	7
2:00 – 3:00 PM	4	4	5	5	5	5	7	7
3:00 – 4:00 PM	4	4	5	5	5	5	7	7
4:00 – 5:00 PM	2	2	2	2	2	2	2	2
5:00 – 6:00 PM	0	0	0	0	0	0	0	0
Total Trips	42	42	57	57	52	52	75	75

Employees are generally expected to arrive at approximately 7:00 a.m. and leave at approximately 5:00 p.m. Under existing operations, the quarry employs eight people. Due to the relatively long distance from commercial areas, it is expected that few employees will leave the site for a lunch break; however, to provide a conservative analysis, it was assumed that up to half of the employees leave and return during the noon hour for a lunch break. This is consistent with standard employee trip generation estimates of three trips per employee per day. Since employees are expected to arrive outside of a peak analysis period, their inbound trips in the morning and two-way lunch break trips were not included in the analysis, but employees are expected to leave during the 4:45 p.m. to 5:45 p.m. analysis period, so a total of eight outbound personal vehicle trips were added to that analysis period.

Project Trip Generation

The Harris Quarry project would allow the permitted annual extraction to increase to 200,000 cubic yards and adds an AC batch plant capable of producing 150,000 tons annually. The project proposal also includes the importation of 10,000 cubic yards of concrete and AC annually for recycling. While the project proposal includes importation of 10,000 tons of material for recycling, the analysis was based on importation of 10,000 cubic yards. Since 10,000 tons of material for recycling is expected to yield approximately 5,000 to 6,000 cubic yards of material, the analysis was based on a considerably higher volume of recycling material, so provides conservative results of the project's truck trip generation and actual potential impacts. Additionally, while it is anticipated that the imported material will be brought in on trucks that will be used to export other material, therefore resulting in no additional trips, to provide a conservative analysis it was assumed that the recycle material imported will come in on trucks that will then leave the site empty. Once processed, the recycled material will be exported as part of the aggregate for sale. Additional assumptions specific to each product type and the two peak months are shown in Table 6.

Table 6
Trip Generation Assumptions with Proposed Project

Source	Annual Production	Percent of Annual in:	
		July	October
Material Mined	200,000 cy	n/a	n/a
Recycle Material Imported	10,000 cy	9.4%	12.2%
Aggregate Production For Sale	162,520 cy	9.4%	12.2%
Aggregate transferred to Willits for PCC	37,200 cy	10.0%	15.8%
AC Production	58,280 cy	12.0%	17.0%
Total Production	258,000 cy		

In addition, the Quarry will generate truck trips for deliveries as well as clients or other quarry visitors who will arrive by personal vehicle. As with the Base Permit conditions, one inbound and one outbound truck trip was added to each hour between 7:00 a.m. and 4:00 p.m. to account for delivery of goods or other services. This is expected to provide a conservative analysis as it would result in a greater number of delivery trucks than would be expected in a typical month. Using the assumptions indicated, trips on the average weekday and monthly peak day were derived for the months of July and October. An average of 214 truck trips and a peak of 310 daily truck trips are expected during the month of July. For October, 286 truck trips are expected on average, while 412 truck trips would be expected on a peak day.

Construction activities typically begin in the morning between 7:00 and 8:00 a.m., reaching peak activity near 9:00 a.m. This peak continues until about noon, then reduces slightly until 3:30 or 4:00 p.m. Limited quarry activity occurs after 4:00 p.m. and none after 5:00 p.m. Using the hourly trend data from the EIRs for the Canyon Rock and Blue Rock Quarries in Sonoma County, hourly truck trips were projected for the average and peak days in July and October. Table 7 summarizes the projected truck trips entering and leaving the Harris Quarry during these peak months. Project Conditions truck trips by hour are provided in Appendix D and turning movements at the project access are provided in Appendix E. On occasion, the quarry operates overnight or on weekends due to an urgent need for AC or aggregate, usually due to a natural

disaster, or to satisfy the need for AC or aggregate for overnight road work as requested by Caltrans or other governmental agency. Since overnight and weekend operations are rare and do not represent typical operations, they were not included in this analysis. Details of the assumptions applied and the calculation are located in Appendix C.

**Table 7
Projected Truck Trips for Project Production (200,000 cy Mined)**

Time	Aggregate				AC				Transfer to Willits				Other Trucks			
	Average Day		Peak Day		Average Day		Peak Day		Average Day		Peak Day		Average Day		Peak Day	
	In	Out	In	Out	In	Out	In	Out	In	Out	In	Out	In	Out	In	Out
July																
6:00 – 7:00 AM	1	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0
7:00 – 8:00 AM	6	6	8	9	4	3	4	3	2	1	2	1	1	1	1	1
8:00 – 9:00 AM	7	7	11	11	3	3	5	4	2	1	2	1	1	1	1	1
9:00 – 10:00 AM	7	7	11	11	3	3	5	5	1	1	2	2	1	1	1	1
10:00 – 11:00 AM	7	7	11	11	3	3	5	5	1	1	2	2	1	1	1	1
11:00 AM – Noon	7	7	11	11	3	3	5	5	1	1	2	2	1	1	1	1
Noon – 1:00 PM	6	6	10	10	3	3	4	4	1	1	2	2	1	1	1	1
1:00 – 2:00 PM	6	6	8	8	2	2	4	4	1	1	2	2	1	1	1	1
2:00 – 3:00 PM	6	6	8	8	2	2	3	4	1	1	2	2	1	1	1	1
3:00 – 4:00 PM	6	6	8	8	2	2	3	4	1	2	1	2	1	1	1	1
4:00 – 5:00 PM	2	3	3	3	1	2	0	1	0	1	0	1	0	0	0	0
5:00 – 6:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total July Trips	61	61	90	90	26	26	39	39	11	11	17	17	9	9	9	9
October																
6:00 – 7:00 AM	1	0	2	0	2	0	1	0	0	0	0	0	0	0	0	0
7:00 – 8:00 AM	7	8	11	11	4	4	5	5	2	1	3	3	1	1	1	1
8:00 – 9:00 AM	10	10	14	14	5	4	7	6	2	2	4	3	1	1	1	1
9:00 – 10:00 AM	10	10	14	14	4	4	7	7	2	2	3	3	1	1	1	1
10:00 – 11:00 AM	10	10	14	14	4	4	7	7	2	2	3	3	1	1	1	1
11:00 AM – Noon	10	10	14	14	4	4	7	7	2	2	3	3	1	1	1	1
Noon – 1:00 PM	8	8	12	12	4	4	6	6	2	2	3	3	1	1	1	1
1:00 – 2:00 PM	7	7	11	11	3	3	5	5	2	2	2	2	1	1	1	1
2:00 – 3:00 PM	7	7	11	11	3	4	5	5	2	2	2	2	1	1	1	1
3:00 – 4:00 PM	7	7	10	11	3	4	4	5	2	2	2	2	1	1	1	1
4:00 – 5:00 PM	2	2	3	4	1	2	1	2	0	1	1	2	0	0	0	0
5:00 – 6:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total October Trips	79	79	116	116	37	37	55	55	18	18	26	26	9	9	9	9

As with existing conditions, employees are generally expected to arrive at approximately 7:00 a.m. and leave at approximately 5:00 p.m. Under the proposed expansion, the quarry is expected to employ eleven people during typical operations. As was assumed for Base Permit conditions, though most employees are not expected to leave the site for a lunch break it was conservatively assumed that half of the employees will leave and return during the noon hour. Since employees are expected to arrive and take lunch breaks outside of an analysis period their inbound trips in the morning and round trips at lunch time were not included in the analysis. Employees are expected to leave during the 4:45 p.m. to 5:45 p.m. analysis period, so a total of eleven outbound personal vehicle trips was added during that interval. Any non-standard employee trips during the day can be accounted for in the conservative delivery truck projections.

Projected Traffic Volumes

Caltrans District I has projected that volumes on US 101 in the project vicinity will grow by 50 percent over the 20-year period from 2006 to 2025. Likewise, County of Mendocino Planning staff has directed that volumes on Black Bart Drive should be projected to grow at the same rate for analysis purposes. The 50 percent growth in traffic was projected to occur by 2025, while year 2010 and 2014 volumes were developed using a straight line growth projection between current and projected 2025 volumes.

Typically, travel demand forecasting is completed for about a 20-year horizon. In the case of US 101 within the study area, the most recent forecasting for both Caltrans and the County of Mendocino was completed in 2006, meaning that current projections extend to 2025. The 2030 and 2040 horizon years are therefore beyond current Caltrans and County of Mendocino forecasting and beyond the typical 20-year forecasting horizon. In order to project traffic volumes for 2030 and 2040, the same annual growth rate was applied that was used to project the 2010 and 2014 volumes. It should be noted that since this is beyond the typical forecasting horizon, the reliability of these projected traffic volumes is unknown and may vary based on changes in area demographics and travel patterns that cannot be accounted for at this time.

Vehicle Miles Traveled

Vehicle miles traveled (VMT) is the measure of the total miles traveled by residents, costumers, employees and delivery of goods from a source location, and is typically calculated on an annual basis. VMT also serves as a measure of the broader potential impacts of vehicle travel on an area wide circulation system and correspondingly relates to fuel consumption and vehicle emissions, including greenhouse gases.

The demand for aggregates is a function of population and will increase with population; this relationship is recognized in Section 8.3 of the *Mendocino County General Plan Update Background Report*. From day to day, month to month and year to year there will be variations in project trips depending upon locations of major construction projects, and over time the demand for aggregates will follow population patterns in the County. The demand for aggregates will be met by a combination of quarries and mines currently operating within Mendocino County and neighboring Humboldt, Lake and Sonoma Counties.

Using the Mendocino County General Plan Update Growth Projections for incorporated cities and adjacent environs, population centers or sub-areas were identified and the distance from the operating quarries to the center of each sub-area was determined. Standard gravitational model theory indicates that the portion of total project trips to a sub-area is proportional to the population of the sub-area and inversely proportional to the square of the total trip distance. Based on this theory quarries would be expected to provide more aggregate to population centers that are closer to the quarry and correspondingly fewer trips

to population centers of equal size that are further away. Likewise larger population sub-areas will have a greater demand potential than smaller sub-areas and will attract more trips. For analysis purposes, it was necessary to hold demand equal over all scenarios to provide a common point of comparison. Applying this theory, the number of trips and VMT from each quarry and AC plant to each population center fulfilling the demand for aggregate and AC within the County was calculated. In the case of VMT for AC distribution the addition of a third AC plant in the County significantly changes the dynamics of AC related VMT. The VMT gravity model was conducted for aggregate and AC production only and not for concrete production. Calculations are provided in Appendix F.

It is projected for Base Permit conditions that all supplying quarries will make a combined total of approximately 38,900 truck haul trips per year with a combined total of approximately 2.45 million miles traveled to provide aggregate within Mendocino County. With the increased production potential of the Harris Quarry to 200,000 cubic yards of material mined, assuming that the demand for aggregate remains constant, it is projected that the total of truck hauls made countywide by all supplying quarries will remain constant at approximately 38,900 truck haul trips per year. There is a slight difference of four truck trips per year between the two scenarios which is attributed to rounding in the calculations and accounts for about one-hundredth of one percent of all trips, so for the purpose of analysis these values are considered to be equal. The increased production of the Harris Quarry project will not reduce the number of haul trips but will result in approximately 2.42 million vehicle miles traveled annually from all quarries and will result in 30,490 fewer VMT by aggregate haul trucks. Although the number of aggregate haul trips is equivalent, the total VMT decreases because the Harris Quarry is closer to major population centers and geographical centers of the County, resulting in shorter trip lengths and correspondingly fewer vehicle miles traveled to transport the same amount of aggregate.

The AC plant portion of the project will also increase the number of trips and vehicle miles traveled from the project site, but would decrease the VMT countywide from all supplying AC plants. The annual trips made countywide by all supplying quarries is expected to remain constant at approximately 11,720 trips per year, with Harris Quarry accounting for 5,622 trips. As with the aggregate production, the model produced a slight difference in the total number of trips projected under the two scenarios, but they were considered to be equal for analysis purposes. Countywide a total of 801,200 VMT are expected under Base Permit conditions; this would decrease to 648,100 VMT under project conditions which is a decrease of approximately 153,000 VMT per year. It is expected that Harris Quarry would account for about 85,600 of the annual VMT associated with AC production. As noted for the quarry operation, an AC plant at the Harris Quarry site will increase the number of trips to and from the site but will reduce the total number of VMT in the County overall as the trip lengths are typically shorter than those made by out-of-County sources. The project has the potential to decrease the total VMT associated with hauling aggregate and AC within Mendocino County by approximately 183,500 VMT annually. These results are summarized in Table 8.

**Table 8
Vehicle Miles Traveled Summary**

Quarry Aggregate VMT	Base Permit		Project		Change In VMT
	Annual Trips	Annual VMT	Annual Trips	Annual VMT	
Harris	3,719	98,761	7,550	225,263	126,502
Davis Pit	3,721	322,636	3,579	318,899	-3,737
Keithly Ranch	4,959	415,506	4,747	401,631	-13,875
DNA River	1,240	145,695	1,209	144,970	-725
Cooks Humboldt	992	133,202	973	129,297	-3,905
Ford Gravel	9,920	287,671	8,096	236,262	-51,409
Ten Mile	2,480	75,480	2,018	58,816	-16,664
Pieta	2,481	109,033	2,058	89,771	-19,262
Layton Rock	2,480	69,650	2,088	47,997	-21,653
Cooks Valley	2,480	329,488	2,420	319,171	-10,317
Wisley Ranch	744	25,178	591	19,419	-5,759
Coal Mine	1,241	108,060	1,199	104,864	-3,196
Syar Healdsburg	2,449	326,041	2,412	319,551	-6,490
Total	38,936	2,446,401	38,940	2,415,911	-30,490
Plant AC VMT					
Harris	0	0	2,644	85,621	85,621
Granite	7,499	284,767	5,444	183,063	-101,704
Baxman	1,718	71,146	1,815	61,089	-10,057
Bodean/Syar (Santa Rosa)	2,501	445,239	1,816	318,347	-126,892
Total	11,718	801,152	11,719	648,120	-153,032
Project Total	50,654	3,247,553	50,659	3,064,031	-183,522

Note: VMT = Vehicle miles traveled

It is anticipated that VMT related to aggregate demand will increase in the future as population and demand for aggregate increases in Mendocino County. The relationships developed based upon 2000 population data will remain valid in the future. As the demand for aggregate and AC increases, while production levels within the Mendocino County remain constant, the shortfall will be filled by out-of-County sources. Trips made from out-of-County sources have greater travel distances and result in higher VMT overall. Conversely, the expansion of the Harris Quarry together with AC production will result in fewer VMT in

the future as a greater portion of the total demand will be met by quarries and AC plants within Mendocino County and less by out-of-County sources.

The increased aggregate production and a new AC plant associated with the Harris Quarry project will reduce the need to import aggregates and AC into the County as more of the demand will be met by local sources. However, the Harris Quarry project is not expected to eliminate the need to import aggregate and AC to meet current and future demand, and importation of aggregate and AC will remain a significant component in fulfilling the need for these products in Mendocino County.

Trip Distribution

The projected distribution of trips to and from the Harris Quarry under current aggregate production levels was tested and compared to driveway turning movements obtained in the summer of 2006. The VMT gravity model described earlier projects a trip distribution of 45 percent to/from the north and 55 percent to/from the south on US 101, which is consistent with measured driveway volumes.

The increased production potential of the Harris Quarry will cause the redistribution of trips from all quarries and will result in a different distribution of trips to and from the Harris Quarry along US 101. With the project and associated increase in production the distribution of trips is projected to shift to 35 percent of trips to/from the north and 65 percent to/from the south. Trips destined for the concrete plant in Willits were all assumed to be traveling to/from the north. Additionally, since employment does not necessarily match the gravity model, it was assumed that an equal number of employees will be coming from and return to the north and south.

If Harris Quarry is selected to provide aggregate and/or AC for the Willits Bypass construction, it is expected that a greater portion of quarry generated traffic will be destined to the north. For this condition it is estimated that 75 percent of trips will be to/from the north with the remaining 25 percent going to/from the south for base permit conditions and 60 percent to/from the north and 40 percent to/from the south for project conditions. These trip distribution assumptions are summarized in Table 9.

**Table 9
Trip Distribution**

Condition	Percent North	Percent South
Base	45	55
Base w/By-pass Construction	75	25
Project	35	65
Project w/ By-pass Construction	60	40
Aggregate to Concrete Plant in Willits	100	0

Study Scenarios

For consistency with the DEIR, the following weekday peak traffic hours were evaluated.

- 9:00 – 10:00 a.m.
- 11:00 a.m. – 12:00 Noon
- 1:15 – 2:15 p.m.
- 4:45 – 5:45 p.m.

To evaluate the highest impact situations, traffic generated by the Quarry was evaluated for July, when traffic on US 101 is at its highest, and October, when Quarry production peaks.

Conditions were evaluated for the following ten scenarios.

2010 Base Permit: This scenario is an analysis of operating conditions under projected 2010 traffic volumes together with operation of the Harris Quarry at current permitted levels.

2010 Project: Conditions with the Harris Quarry project added to projected 2010 traffic volumes are evaluated in this scenario.

2014 Base Permit with Willits Bypass Construction: This scenario indicates operating conditions under projected 2014 traffic volumes together with the operation of the Harris Quarry at current permitted levels, assuming that Harris Quarry is selected to supply material for the Willits Bypass project.

2014 Project with Willits Bypass Construction: Operating conditions under projected 2014 traffic volumes together with the Harris Quarry project, assuming that Harris Quarry is selected to supply material for the Willits Bypass project, are indicated by this scenario.

2014 Base Permit without Willits Bypass Construction: This scenario indicates operating conditions under projected 2014 traffic volumes together with the operation of the Harris Quarry at current permitted levels, assuming that Harris Quarry is not selected to supply material for the Willits Bypass project.

2014 Project with Willits Bypass Construction: Operating conditions under projected 2014 traffic volumes together with the Harris Quarry project assuming that Harris Quarry is not selected to supply material for the Willits Bypass project are indicated by this scenario.

2030 Base Permit: This scenario is an evaluation of operating conditions under projected 2030 traffic volumes with operation of the Harris Quarry at current permitted levels.

2030 Project: This scenario reflects operating conditions under projected 2030 traffic volumes together with the Harris Quarry project.

2040 Base Permit: This scenario is an evaluation of operating conditions under projected 2040 traffic volumes with operation of the Harris Quarry at current permitted levels.

2040 Project: The final scenario is an evaluation of operating conditions under projected 2040 traffic volumes together with the proposed Harris Quarry project.

Passenger Car Equivalent

Level of service calculations are based upon predominantly passenger vehicles, while the Quarry trip generation is primarily large trucks. Truck trips were factored to a passenger car equivalent (PCE) at a ratio of one truck being equivalent to four passenger vehicles. The purpose of the PCE is to account for the fact that a truck is considerably larger, slower and less maneuverable than a standard passenger car. The passenger car equivalent volumes were used in the evaluation of potential project impacts. PCE ratios were applied only to trucks entering or exiting the Quarry and not to preexisting truck traffic on US 101 that does not interact with the Quarry.

Speed Change Lane Warrants

The need for turn acceleration and/or deceleration speed change lanes on US 101 was evaluated based on recommendations of Larson and Mannering for the Washington State Transportation Center (TRAC), as contained in their report, "Method for Prioritizing Intersection Improvements" (1997). The authors performed analysis and review of several differing methodologies and found that the 1991 "Modified Harmelink" model developed by Kikuchi and Chakroborty yielded the most reliable and appropriate recommendations. These calculations are based upon passenger vehicles, which make up the vast majority of vehicles on the roadway; for trucks turning at the project driveway the above-described passenger car equivalents were used. The need for speed change acceleration lanes was determined using the same warrant methodology.

Level of Service Methodologies

Level of Service (LOS) is used to rank traffic operation on various types of facilities based on traffic volumes and roadway capacity using a series of letter designations ranging from A to F. Generally, Level of Service A represents free flow conditions and Level of Service F represents forced flow or breakdown conditions. A unit of measure that indicates a level of delay generally accompanies the LOS designation.

Intersection

The intersection of US 101/Black Bart Drive was evaluated using the two-way stop-controlled intersection capacity method from the *Highway Capacity Manual* (HCM), Transportation Research Board, 2000. Since it is not subject to the influence of intersection control, the through movements on the main street, US 101, are assumed to operate at free flow and a Level of Service A. The level of service for each minor turning movement is based on the estimated average delay. Average delay for the intersection overall is also determined. The ranges of delay associated with the various levels of service are indicated in Table 10.

Table 10
Two-Way Stop-Controlled Intersection Level of Service Criteria

LOS A	Delay of 0 to 10 seconds. Gaps in traffic are readily available for drivers exiting the minor street.
LOS B	Delay of 10 to 15 seconds. Gaps in traffic are somewhat less readily available than with LOS A, but no queuing occurs on the minor street.
LOS C	Delay of 15 to 25 seconds. Acceptable gaps in traffic are less frequent, and drivers may approach while another vehicle is already waiting to exit the side street.
LOS D	Delay of 25 to 35 seconds. There are fewer acceptable gaps in traffic, and drivers may enter a queue of one or two vehicles on the side street.
LOS E	Delay of 35 to 50 seconds. Few acceptable gaps in traffic are available, and longer queues may form on the side street.
LOS F	Delay of more than 50 seconds. Drivers may wait for long periods before there is an acceptable gap in traffic for exiting the side streets, creating long queues.

Reference: *Highway Capacity Manual*, Transportation Research Board, 2000

Grade Separated Access

For evaluation of the conversion of the intersection of the Quarry Access with US 101 to a grade-separated interchange, the grade-separated ramp capacity method from the HCM was used. The level of service for each ramp merge and diverge area is based upon the through volume on the main line as well as the volume of traffic entering or exiting the main line. The capacity of the ramp is expressed as a density of passenger cars per mile per lane. The ranges of density associated with the various levels of service are indicated in Table 11.

Table 11
Ramp Level of Service Criteria

	Density (passenger cars/mile/lane)
LOS A	≤ 10
LOS B	> 10-20
LOS C	> 20-28
LOS D	> 28-35
LOS E	> 35
LOS F	Demand exceeds capacity

Reference: *Highway Capacity Manual*, Transportation Research Board, 2000

Minimum Acceptable Standards

In the *Guide for the Preparation of Traffic Impact Studies*, Caltrans indicates that they endeavor to maintain operation at the transition from LOS C to LOS D; however, where operation is already below LOS C, the existing measure of effectiveness should be maintained. For intersections, this means that the existing

control delay should be maintained. Under this criteria, any increase in delay would therefore result in a significant impact. Since this approach results in impacts that are deemed significant for even a small change in operating conditions, staff at Caltrans was consulted. The issue of applying this standard was discussed with Mr. Marc Birnbaum, the Statewide Advisor for Local Development and Traffic Impact Studies. Mr. Birnbaum indicated that for intersections, the standard is to be applied to the overall average intersection delay, not that associated with any single movement or approach. These criteria do not apply for private road intersections, such as the entrance to the Harris Quarry, but were applied to the intersection of US 101/Black Bart Drive.

In its *General Plan*, Mendocino County establishes Policy DE-121 that states that, “New discretionary development will be required to demonstrate that basic infrastructure is available and has adequate capacity to serve the project without degrading existing level of service standards.” The General Plan does not, however, establish thresholds for what level of degradation is considered to constitute a significant impact. For this reason, the above described Caltrans level of significance was applied to the study intersection. To provide a conservative analysis, in addition to the Caltrans standard for overall intersection operation, LOS D was considered to be the worst acceptable LOS for any single movement. This is consistent with standards established by many other jurisdictions throughout California. As with Caltrans standards, there are no applicable standards for private road intersections such as the Harris Quarry access road connection to US 101.

While Caltrans standards apply to the study intersection of US 101/Black Bart Drive, both the overall average intersection delay and the delay on the worst approach were considered to provide a conservative analysis. Additionally, the approach with the highest delay was used as a metric to determine the effectiveness of proposed turn-lane mitigating measures.

Base Permit Operations

Since renewal of the Quarry’s permit to continue current operations is one of the options possible under this environmental review, the need for improvements under Base Permit conditions was evaluated. The evaluation takes into account the traffic volumes on the major roadway, US 101, as well as turning movements into and out of the minor street or access roadway and the speed of travel on the major roadway.

Speed Change Lanes

The need for speed change lanes was evaluated based on projected traffic volumes on both US 101 and the project access for peak day conditions. A northbound left turn lane and southbound right turn taper are warranted for all analysis years, with the exception of 2014 with the Willits Bypass construction; for this scenario the northbound left turn lane is not warranted. This exception is due to the fact that the Willits Bypass construction is expected to reduce quarry traffic to the north to the point that not enough vehicles will make the northbound left turn movement to warrant the lane; however, since the lane is warranted without the bypass construction and is warranted in 2010, the installation of the northbound left turn lane is recommended for continued operation at currently permitted levels.

A southbound right turn lane is not warranted under any project scenario; however, since a right turn taper is warranted, and due to the proximity to Black Bart Drive, it is recommended that a full right-turn

deceleration lane be installed. This will also operate as an acceleration lane for vehicles turning right out of Black Bart Drive.

A northbound left-turn acceleration lane is warranted immediately for continued Quarry operation. This lane would provide refuge for vehicles turning left out of the Quarry Access driveway onto Northbound US 101. This acceleration lane would provide space for turning vehicles to reach or at least approach free-flow speeds on US 101, therefore reducing the impact on through traffic and increasing the frequency of acceptable gaps for turning. This acceleration lane should be at least 1,410 feet in length, which would extend beyond Black Bart Drive and have a secondary benefit of providing refuge for vehicles turning out of Black Bart Drive. Since this lane would overlap with the northbound left-turn lane at Black Bart Drive, there is a possibility of a vehicle needing to use the lane to decelerate while another driver needs to use the lane to accelerate; however, it was determined that this is likely to be a rare occurrence. Therefore, no additional action is recommended.

Additionally, a southbound right-turn acceleration taper is warranted by 2030. This taper would give trucks an opportunity to accelerate before entering traffic.

Turn lane warrant results are summarized in Table 12 for each year; if a turn lane is warranted for one or more analysis time periods, it is considered to be warranted for that analysis year. Turn lane warrants were calculated for all analysis time periods for both July and October peak months and details are provided in Appendix G.

**Table 12
Speed Change Lane Warrants for Base Permit Conditions**

Year	Northbound Left Turn		Southbound Right Turn	
	Deceleration Lane	Acceleration Lane	Deceleration Taper	Acceleration Taper
2010	Warranted	Warranted	Warranted	Not Warranted
2014 w/o Willits Bypass	Warranted	Warranted	Warranted	Not Warranted
2014 w/Willits Bypass	Not Warranted	Warranted	Warranted	Not Warranted
2030	Warranted	Warranted	Warranted	Warranted
2040	Warranted	Warranted	Warranted	Warranted

Base Permit Intersection Level of Service

The Base Permit Conditions scenario indicates operation under projected traffic volumes at US 101/Black Bart Drive for the four study time periods during the July and October peak periods.

Under Base Permit conditions, the intersection of US 101/Black Bart Drive is expected to operate acceptably during all peak hours. The level of service results representing the movement with the highest delay for the July and October peak production periods are summarized in Table 12 for US 101/Harris Quarry Access and Table 13 for US 101/Black Bart Drive. Although not reported in Tables 13 and 14, the overall delay at both intersections was studied and was determined to be LOS A at all times with no more

than 3.2 seconds of average delay. Detailed LOS calculations for average and peak production levels are provided in Appendix H.

Table 13
US 101/Harris Quarry Access Base Permit Level of Service Calculations – Peak Production

Condition	Production Period	9-10 AM		11 AM-noon		1:15-2:15 PM		4:45-5:45 PM	
		Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS
2010 Base Permit	July	15.6	C	21.7	C	23.5	C	20.7	C
<i>Mitigated (turn lanes)</i>	July	13.0	B	15.3	C	15.7	C	14.7	B
	October	13.9	B	17.6	C	17.2	C	15.5	C
<i>Mitigated (turn lanes)</i>	October	12.2	B	13.8	B	13.5	B	12.7	B
2014 Base Permit	July	16.6	C	24.11	C	26.5	D	23.0	C
<i>Mitigated (turn lanes)</i>	July	13.5	B	16.1	C	16.6	C	15.4	C
	October	14.5	B	19.0	C	19.0	C	18.6	C
<i>Mitigated (turn lanes)</i>	October	12.5	B	14.4	B	14.0	B	13.2	B
2014 Base Permit w/Bypass	July	19.4	C	30.5	D	31.6	D	23.0	C
<i>Mitigated (turn lanes)</i>	July	14.9	B	18.3	C	18.3	C	15.4	C
	October	15.4	C	20.9	C	22.4	C	16.6	C
<i>Mitigated (turn lanes)</i>	October	13.1	B	15.2	C	15.7	C	13.2	B
2030 Base Permit	July	22.0	C	39.6	E	45.4	E	36.8	E
<i>Mitigated (turn lanes)</i>	July	15.6	C	19.9	C	20.7	C	18.8	C
	October	17.7	C	26.7	D	26.2	D	22.3	C
<i>Mitigated (turn lanes)</i>	October	14.0	B	17.0	C	16.5	C	15.2	C
2040 Base Permit	July	26.9	D	57.7	F	67.9	F	51.7	F
<i>Mitigated (turn lanes)</i>	July	17.2	C	23.1	C	24.1	C	21.5	C
	October	20.4	C	34.3	D	33.7	D	27.6	D
<i>Mitigated (turn lanes)</i>	October	15.1	C	19.0	C	18.3	C	16.7	C

Notes: Delay is in average number of seconds per vehicle; LOS = Level of Service; **Bold** = deficient operation

Table 14
US 101/Black Bart Drive Base Permit Level of Service Calculations – Peak Production

Condition	Production Period	9-10 AM		11 AM-noon		1:15-2:15 PM		4:45-5:45 PM	
		Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS
2010 Base Permit	July	15.8	C	25.5	D	20.0	C	19.9	C
<i>Mitigated (turn lanes)</i>	July	13.1	B	17.0	C	14.4	B	14.3	B
	October	13.3	B	18.9	C	20.4	C	15.0	B
<i>Mitigated (turn lanes)</i>	October	11.9	B	14.4	B	14.8	B	12.5	B
2014 Base Permit	July	16.8	C	31.2	D	22.6	C	22.5	C
<i>Mitigated (turn lanes)</i>	July	13.6	B	18.3	C	15.3	C	15.1	C
	October	13.9	B	20.9	C	22.8	C	16.2	C
<i>Mitigated (turn lanes)</i>	October	12.2	B	15.2	C	15.7	C	13.0	B
2014 Base Permit w/Bypass	July	17.0	C	31.8	D	22.7	C	22.5	C
<i>Mitigated (turn lanes)</i>	July	13.7	B	18.4	C	15.3	C	15.11	C
	October	14.0	B	21.2	C	23.2	C	16.2	C
<i>Mitigated (turn lanes)</i>	October	12.3	B	15.3	C	13.0	B	13.6	B
2030 Base Permit	July	23.1	C	68.6	F	40.9	E	40.4	E
<i>Mitigated (turn lanes)</i>	July	16.0	C	24.6	C	19.3	C	19.0	C
	October	17.0	C	32.7	D	38.3	E	22.6	C
<i>Mitigated (turn lanes)</i>	October	13.7	B	18.6	C	19.5	C	15.1	C
2040 Base Permit	July	29.8	D	**	F	69.2	F	68.6	F
<i>Mitigated (turn lanes)</i>	July	18.0	C	30.6	D	22.8	C	22.4	C
	October	19.9	C	47.5	E	59.9	F	29.6	D
<i>Mitigated (turn lanes)</i>	October	14.9	B	21.5	C	22.9	C	16.9	C

Notes: Delay is in average number of seconds per vehicle; LOS = Level of Service; **Bold** = deficient operation
 ** = greater than 120 seconds of average delay

Base Permit Conditions

Analysis Years

Project impacts were analyzed for the horizon years of 2010, 2014, 2030 and 2040. Year 2014 conditions were evaluated both with and without the Willits bypass under construction. While this construction would alter distribution of project trips, it would not be expected to change background traffic volumes on US 101 in the vicinity of Black Bart Drive and the Harris Quarry.

Base Permit Intersection Operation

Intersection Level of Service was determined for both US 101/Black Bart Drive and US 101/Quarry Access Driveway based upon the existing lane configuration. The intersection of US 101/Harris Quarry Access is provided for informational purposes. Since LOS criteria do not apply to private driveways, only the intersection of US 101/Black Bart Drive is discussed in further detail.

Year 2010

The intersection of US 101/Black Bart Drive is expected to operate at LOS A overall and the stop-controlled eastbound approach is expected to operate at LOS D or better during all time periods.

Year 2014 (No Willits Bypass)

Under the projected 2014 Base Permit conditions the intersection of US 101/Black Bart Drive is expected to operate acceptably at LOS A overall with the eastbound approach operating at LOS D or better during all time periods analyzed.

Year 2014 (During Willits Bypass Construction)

With the change in distribution that would be expected if Harris Quarry is selected to provide material for the construction of the Willits Bypass, US 101/ Black Bart Drive is expected to operate at LOS A overall with all individual movements operating at LOS D or better during all peak hours analyzed.

Year 2030

Under the projected year 2030 volumes with production at permitted levels, US 101/Black Bart Drive is expected to operate overall acceptably at LOS A; however as indicated in Table 9, the eastbound approach is expected to operate deficiently at LOS E during the 11:00-noon time period and LOS F during the 1:15-2:15 p.m. and 4:45-5:45 p.m. time periods in July, though acceptable operating conditions of LOS D or better are projected for the 9:00-10:00 a.m. peak hour during July and during all four time periods analyzed in October.

Year 2040

As shown in Table 9, under the projected year 2040 volumes and with production at levels consistent with the existing permit, the eastbound movement at US 101/Black Bart Drive is expected to operate deficiently at LOS F during the 11:00-noon, 1:00-2:00 p.m. and 4:45-5:45 p.m. time periods in July as well as LOS E in 11:00-noon and LOS F in 1:15-2:15 p.m. time periods in October. For the 9:00-10:00 a.m. time period in July and for the 9:00-10:00 a.m. and 4:45-5:45 p.m. time periods in October, the eastbound movement is projected to operate acceptably at LOS D or better. Overall, the intersection is expected to operate acceptably at LOS A during all time periods.

Base Permit Mitigated Intersection Operations

Year 2010

Based upon intersection LOS, no mitigating measures are necessary; however, with the installation of the speed change and turn lanes that are currently warranted, the LOS does slightly improve.

Year 2014 (Without Willits Bypass Construction)

Based upon intersection LOS, no mitigating measures are necessary; however, with the installation of all warranted speed change and turn lanes, the LOS does slightly improve.

Year 2014 (With Willits Bypass Construction)

Based upon intersection LOS, no mitigating measures are necessary. As with other scenarios, the installation of all warranted speed change and turn lanes improves the LOS slightly.

Year 2030

With the existing lane configuration, the intersection of US 101/Black Bart Drive would be expected to operate deficiently in 2030; however, with the addition of the above discussed speed change and turn lanes, all movements at US 101/Black Bart Drive are expected to operate acceptably at LOS D or better during all time periods.

Year 2040

Without any improvements the intersection of US 101/Black Bart Drive is expected would continue to operate deficiently in 2040. However, acceptable operation of LOS D or better would be expected during all time periods with the addition of the above discussed improvements.

Driveway Operation

Although there are no LOS standards for a private driveway, if there is excessive delay for drivers turning to or from the driveway they may be willing to turn in an adequate gap in traffic, therefore creating a potential safety issue. For this reason, operation of the Quarry Access Driveway was studied and it was determined that with the installation of all recommended turn lanes, the driveway would operate acceptably at all times with, on average, no more than 24.1 seconds of delay for exiting traffic. This greatly reduces the possibility of drivers attempting to make turns without an adequate gap in traffic.

Project Conditions

Speed Change Lanes

As with the Base Permit conditions, the need for speed change lanes was evaluated based on projected traffic volumes on both US 101 and the project turning movements for peak day conditions. The northbound left turn lane and southbound right turn taper are warranted for all analysis years. Although a southbound right-turn lane is not warranted, it is recommended that one be installed due to the

driveway's proximity to Black Bart Drive. Additionally a northbound left-turn acceleration lane is warranted that would provide refuge for vehicles turning left out of the Quarry Access driveway onto Northbound US 101. As with the Base Permit condition, this acceleration lane should be at least 1,410 feet in length, which would extend beyond Black Bart Drive and have a secondary benefit of providing refuge for vehicles turning out of Black Bart Drive. A southbound right-turn acceleration taper is already warranted and a full acceleration lane is warranted by 2040. Turn lane warrant results are summarized in Table 15 for each year; if a turn lane is warranted for one or more analysis time periods, it is considered to be warranted for that analysis year. Turn lane warrants were calculated for all analysis time periods for both July and October peak months and details are provided in Appendix G.

**Table 15
Speed Change Lane Warrants for Project Conditions**

Year	Northbound Left Turn		Southbound Right Turn	
	Deceleration Lane	Acceleration Lane	Deceleration Taper	Acceleration Taper
2010	Warranted	Warranted	Warranted	Warranted
2014 w/o Willits Bypass	Warranted	Warranted	Warranted	Warranted
2014 w/Willits Bypass	Warranted	Warranted	Warranted	Warranted
2030	Warranted	Warranted	Warranted	Warranted
2040	Warranted	Warranted	Warranted	Warranted (full acceleration lane)

Project Intersection Level of Service

The Project Conditions scenario indicates operation under projected traffic volumes at US 101/Black Bart Drive for the four study time periods during the July and October peak periods. Tables 16 and 17 summarize Levels of Service for US Harris Quarry Access and US 101/Black Bart Drive, respectively. Although not reported in Tables 16 and 17, the overall average delay at both intersections was studied and determined to be in the LOS A range at all times at 3.3 seconds or less. Detailed LOS calculations for average and peak production levels are provided in Appendix H.

Table 16
US 101/Harris Quarry Access Project Level of Service Calculations – Peak Production

Condition	Production Period	9-10 AM		11 AM-noon		1:15-2:15 PM		4:45-5:45 PM	
		Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS
2010 Project	July	16.8	C	24.5	C	26.2	C	21.0	C
<i>Mitigated (turn lanes)</i>	July	13.6	B	16.1	C	16.4	C	14.8	B
	October	15.0	C	19.7	C	18.6	C	15.7	C
<i>Mitigated (turn lanes)</i>	October	12.8	B	14.7	B	14.1	B	12.8	B
2014 Project	July	18.0	C	28.0	D	30.3	D	23.4	C
<i>Mitigated (turn lanes)</i>	July	14.1	B	17.0	C	17.4	C	15.5	C
	October	15.8	C	21.6	C	20.3	C	16.8	C
<i>Mitigated (turn lanes)</i>	October	13.2	B	15.3	B	14.7	B	13.2	B
2014 Project w/Bypass	July	21.1	C	36.9	E	39.3	E	23.4	C
<i>Mitigated (turn lanes)</i>	July	15.4	C	19.4	C	19.6	C	15.5	C
	October	17.7	C	26.0	D	24.8	C	16.8	C
<i>Mitigated (turn lanes)</i>	October	14.2	B	16.9	C	16.3	C	13.2	B
2030 Project	July	25.0	D	55.4	F	63.0	F	38.0	E
<i>Mitigated (turn lanes)</i>	July	16.5	C	21.7	C	22.3	C	19.1	C
	October	20.0	C	33.8	D	30.9	D	22.7	C
<i>Mitigated (turn lanes)</i>	October	14.9	B	18.4	C	17.5	C	15.3	C
2040 Project	July	32.3	D	100.6	F	118.0	F	54.2	F
<i>Mitigated (turn lanes)</i>	July	18.4	C	25.8	D	26.5	D	21.8	C
	October	23.8	C	49.1	E	43.5	E	28.2	D
<i>Mitigated (turn lanes)</i>	October	16.2	C	20.9	C	19.7	C	16.9	C

Notes: Delay is in average number of seconds per vehicle; LOS = Level of Service; **Bold** = deficient operation

Table 17
US 101/Black Bart Drive Project Level of Service Calculations – Peak Production

Condition	Production Period	9-10 AM		11 AM-noon		1:15-2:15 PM		4:45-5:45 PM	
		Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS
2010 Project	July	16.2	C	27.6	D	20.5	C	19.7	C
<i>Mitigated (turn lanes)</i>	July	13.3	B	17.3	C	14.6	B	14.3	B
	October	13.7	B	19.6	C	21.0	C	15.0	B
<i>Mitigated (turn lanes)</i>	October	12.1	B	14.7	B	15.1	C	12.5	B
2014 Project	July	17.2	C	32.4	D	23.2	C	21.8	C
<i>Mitigated (turn lanes)</i>	July	13.9	B	18.6	C	15.4	C	15.1	C
	October	14.2	B	21.7	C	20.6	C	16.2	C
<i>Mitigated (turn lanes)</i>	October	12.4	B	15.4	C	15.9	C	13.0	B
2014 Project w/Bypass	July	17.7	C	33.6	D	23.7	C	22.5	C
<i>Mitigated (turn lanes)</i>	July	14.0	B	18.9	C	15.6	C	15.1	14.0
	October	14.6	B	22.5	C	24.3	C	16.2	C
<i>Mitigated (turn lanes)</i>	October	12.6	B	15.7	C	16.2	C	13.0	B
2030 Project	July	23.8	C	72.6	F	42.9	E	40.5	E
<i>Mitigated (turn lanes)</i>	July	16.3	C	25.1	D	19.6	C	19.6	C
	October	17.6	C	34.4	D	40.0	E	22.6	C
<i>Mitigated (turn lanes)</i>	October	13.9	B	19.0	C	19.9	C	15.2	C
2040 Project	July	30.9	D	**	F	73.0	F	68.7	F
<i>Mitigated (turn lanes)</i>	July	18.3	C	31.4	D	23.2	C	22.4	C
	October	20.6	C	50.6	E	63.4	F	29.6	D
<i>Mitigated (turn lanes)</i>	October	15.1	C	22.1	C	23.3	C	16.9	C

Notes: Delay is in average number of seconds per vehicle; LOS = Level of Service; **Bold** = deficient operation

** = greater than 120 seconds of average delay

Project Conditions

Turning movements at the Quarry were calculated using the project trip generation values and trips to and from the north added as through movements at Black Bart Drive. These volumes were used to evaluate need for turn lanes as well as intersection operation. Results for the driveway are provided for informational purposes as LOS criteria do not apply to private driveways. Only the intersection of US 101/ Black Bart Drive is therefore discussed in further detail.

Project Intersection Operation

Year 2010

With the addition of project generated traffic and no changes to the existing configuration, the intersection of US 101/Black Bart Drive is expected to operate overall acceptably at LOS A with the eastbound approach operating acceptably at LOS D or better during all time periods studied.

Year 2014 (No Willits Bypass)

As shown in Table 9, with project-generated traffic added to 2014 volumes acceptable operation of LOS C or better on all individual US 101/Black Bart Drive approaches and LOS A overall operation is expected.

Year 2014 (During Willits Bypass Construction)

Conditions with the Bypass construction are nearly identical to those without it. US 101/Black Bart Drive would operate acceptably during all periods evaluated.

Year 2030

In the year 2030 the intersection of US 101/Black Bart Drive is expected to operate overall at LOS A; however, the eastbound left turn movement would be expected to operate unacceptably at LOS E or F during the 11 a.m.-12 noon, 1:15 to 2:15 p.m. and 4:45 to 5:45 p.m. time periods in July under the existing configuration.

Year 2040

Under projected 2040 volumes, US 101/Black Bart Drive is expected to operate at LOS A overall; however, the eastbound left turn movement is expected to operate unacceptably at LOS E or F during the 11 a.m.-12 noon, 1:15 to 2:15 p.m. and 4:45 to 5:45 p.m. time periods in July and the 11 a.m.-12 noon and 1:15 to 2:15 p.m. time periods in October.

Project Mitigated Conditions

Year 2010

Based upon intersection LOS, no mitigating measures are necessary; however, with the installation of all warranted speed change and turn lanes, the LOS does improve slightly.

Year 2014 (Without Willits Bypass Construction)

No mitigating measures are necessary to address operation; however, installation of speed change and turn lanes would be expected to reduce delay slightly.

Year 2014 (During Willits Bypass Construction)

Again, no mitigation is warranted based upon intersection LOS, but operation would improve slightly with

installation of turn and speed change lanes.

Year 2030

With the addition of the previously described improvements, all movements at US 101/Black Bart Drive would be expected to operate acceptably during all time periods.

Year 2040

Continued acceptable operation is expected at US 101/Black Bart Drive in 2040 with the improvements noted above.

Driveway Operation

As with the Base Permit conditions, operation of the quarry driveway was studied to determine average delays experienced by drivers exiting the site. With the installation of all recommended speed change and turn lanes, the driveway would operate acceptably at all times with an average delay of 23.3 seconds or less for exiting traffic.

Turn Lane Lengths

The minimum length of the left-turn lane has previously been determined by the California Department of Transportation (Caltrans) as communicated in a letter dated September 18, 2006, from Mr. Jesse Robertson. Based upon application of standards in the *Highway Design Manual*, 6th Edition, Caltrans staff have indicated that it is acceptable to assume that turning vehicles will slow to 43.5 mph in the through lane with the remaining deceleration occurring in the turn lane, resulting in a deceleration length of 371 feet. Additionally, Caltrans is requiring vehicle storage of 98 feet for a total turn lane length of 469 feet. A copy of this letter is provided in Appendix I.

The lengths of the left-turn and right-turn acceleration lanes were determined using Exhibit 10-70 from American Association of State Highway and Transportation Officials' (AASHTO) *A Policy on the Geometric Design of Highways and Streets*. The length needed to achieve a merging speed of 50 mph on a highway with a 65-mph design speed is 1,410 feet. To assess the impacts on the acceleration length of trucks the National Cooperative Highway Research Program (NCHRP) Report 505, *Review of Truck Characteristics in Roadway Design*, was consulted. Since the northbound acceleration lane would be below two-percent grade over the length of the acceleration lane, the acceleration distance of 1,410 feet remains appropriate for truck acceleration. However, since the southbound acceleration lane is at a two-percent down grade, Tables 64 and 65, "Maximum Weight-to-Power Ratios for Minimum Acceleration Length," in the NCHRP publication were consulted. For the down-grade, the weight-to-power ratio is decreased from 110 pounds per horsepower at a zero percent grade to 85 pounds per horsepower at two-percent grade. Applying this ratio of 85 to 110, the length of the southbound acceleration lane is decreased from 1,410 feet to 1,090 feet. A standard 300-foot transition taper will be needed at the terminus of the acceleration lane. The turn lane lengths are summarized in Table 18.

**Table 18
Turn and Acceleration Lane Lengths (feet)**

	Northbound Left-Turn Lane	Northbound Acceleration Lane	Southbound Acceleration Lane
Taper	120	300	300
Storage	98	N/A	N/A
Deceleration/Acceleration	251	1,410	1,090
Total	469	1,710	1,390

While a southbound right turn lane is not warranted, due to the proximity to Black Bart Drive and since a right turn deceleration taper is warranted, it is recommended that a short right turn lane of approximately 200 feet be installed for the Harris Quarry Entrance. The length of this turn lane is supported by Caltrans staff as communicated in a letter dated August 8, 2006, from Mr. Jesse Robertson; a copy of this letter is provided in Appendix I. To avoid potential conflicts with other movements, the southbound right turn lane for the Quarry Entrance should not extend the entire distance to Black Bart Drive so that southbound through movements destined for the Quarry do not enter the right-turn lane north of Black Bart Drive, causing confusion for drivers exiting Black Bart Drive.

Circulation Improvements

The following circulation improvements are recommended for both Base Permit and Project conditions:

Left Turns into Project Driveway From Northbound US 101

A left turn deceleration lane is needed on the northbound US 101 approach to the Quarry entrance. This would significantly reduce safety concerns associated with trucks slowing and turning left into the Quarry from the high speed left lane on the expressway, and would be a significant benefit.

Right Turns into Project Driveway from Southbound US 101

A right turn deceleration lane is needed on the Southbound US 101 approach to the Quarry entrance. This would allow vehicles turning right into the Quarry to slow outside of the through travel lane where speeds are often in excess of 60 miles per hour. Due to its proximity to the Quarry entrance, this right turn lane could also be used as a right turn acceleration lane by vehicles exiting Black Bart Drive. This would be a significant benefit.

Right Turns from Project Driveway to Southbound US 101

A southbound right turn acceleration taper was determined to be warranted for all but the 2040 conditions to allow loaded trucks leaving the quarry to enter the highway. Under the 2040 conditions an acceleration lane will be needed to allow trucks turning right from the quarry to get fully or partially up to prevailing travel speeds on US 101 before merging.

Left Turns from Project Driveway to Northbound US 101

A left turn acceleration lane is recommended for northbound US 101 north of the Quarry entrance. This lane would significantly improve safety for trucks turning left onto US 101 from the Quarry as they would no longer need to gain speed in the left travel lane of the expressway. This would be a significant benefit.

Alternative Quarry Access

As an alternative to the current quarry access driveway, installation of a partial grade-separated interchange was studied. This partial grade-separation would affect movements to and from northbound US 101 and eliminate the need for trucks to cross oncoming traffic while making left-turns into or out of the site. These movements would instead occur via on- and off-ramps on the east side of the existing expressway. Since drivers making the inbound and outbound right-turns do not have to cross oncoming traffic, these movements would still occur at an at-grade driveway. All right turn lanes and tapers warranted for the Base Permit or Project conditions would still be warranted with the partial grade-separated driveway.

Analysis of operating conditions for both ramps for all study time periods indicates that a grade separated access would be expected to operate acceptably at LOS B or better. Although a partial grade-separated driveway would operate at a better level of service than an at-grade driveway, this improvement is neither warranted nor recommended because acceptable operation can be achieved at the existing at-grade driveway through installation of turn and speed change lanes. Further, there is no documented historical safety issue based upon collision data that would indicate the need for grade separation. Detailed LOS calculations are provided in Appendix J.

Alternative Modes of Transportation

In addition to transportation by personal vehicle, transportation by pedestrians, bicyclists and transit users was considered. Since the majority of trips associated with Harris Quarry are truck haul trips, alternative modes of transportation are not feasible; however, alternative transportation was considered for employees or other Quarry users.

Pedestrian

Currently, no designated pedestrian facilities are provided on US 101 in the vicinity of the project site nor are there pedestrian facilities provided on Black Bart Drive. This is consistent with the rural and low density nature of surrounding land uses and the lack of pedestrian-oriented destinations. The proposed project is not expected to generate a need for pedestrian facilities and no additional facilities are recommended.

Bicycle

While no separated bicycle facilities are currently provided on US 101, bicyclists may utilize the existing shoulder. No separated bicycle facilities are provided along Black Bart Drive. As with pedestrian uses, this is consistent with the rural nature of the surrounding land uses. The proposed project is not expected to generate a need for bicycle facilities, so no additional facilities are recommended; however, when widening of US 101 is completed to add recommended turn lanes, shoulders should be built to applicable Caltrans standards so that they may continue to be utilized by bicyclists.

Transit

Mendocino Transit Authority (MTA) provides service to destinations within Mendocino County as well as some connections to adjacent counties. While MTA does operate fixed-route bus service along the US 101 corridor, there are no scheduled transit stops in the vicinity of the project. The proposed project is not expected to generate a need for additional transit services, so no modifications are recommended.

Driveway Safety

The effect of adverse weather conditions, especially fog, was considered as part of this analysis. While it is impossible to predict future safety conditions, historical data can indicate trends. As noted previously, during the five-year period reviewed there were no reported collisions at the project driveway and there were no collisions within one-half mile north or south of the project driveway involving large trucks with trailers. Since no historical safety issues are present, no mitigating measures are recommended.

Since the driveway is located along a high-speed expressway with highly dispersed intersections, advance warning signs should be placed on US 101 to the north and south of the project driveway. It is recommended “Slow Trucks,” California MUTCD sign designation W51(CA), be installed together with flashing beacons programmed to be on during normal quarry operating hours.

Impacts and Mitigation Measures

Base Permit Conditions

Year 2010

Impact T-1: Trucks traveling northbound and making a left turn into the Quarry must do so from the inside, or fast, lane on US 101, effectively blocking northbound through traffic in this lane while waiting for an acceptable gap in southbound traffic to make the left turn. This creates a safety and operations risk. This is considered a Class II Impact – significant, but mitigable.

Mitigation T-1: Install a 469-foot left turn lane for northbound traffic at the Quarry entrance.

Impact T-2: Trucks turning left out of the Quarry site must accelerate to reach highway speeds while climbing, resulting in trucks that are moving slower than the surrounding traffic. This creates a safety and operations risk. This is considered a Class II Impact – significant, but mitigable.

Mitigation T-2: Install a northbound acceleration lane. A length of 1,710 feet is needed for the acceleration lane to serve project volumes, which exceeds the available distance between the Harris Quarry access and Black Bart Drive. A special lane designation to permit left-turns onto Black Bart Drive and acceleration for both left-turns from Black Bart Drive and acceleration from the Harris Quarry access to northbound US 101 will need to be developed. The extension of the left turn acceleration lane through the intersection at Black Bart Drive can then be used for acceleration by drivers turning left from Black Bart Drive. The overlap of the left turn acceleration lane for trucks leaving the Quarry with the left turn deceleration lane for vehicles entering Black Bart Drive creates a possibility of conflict between a truck accelerating behind a vehicle decelerating. Using the left turn warrant method it was determined that this conflict will not occur in over 97 percent of turning movements; therefore, no further consideration is necessary.

Impact T-3: Trucks making a right turn into the Quarry from southbound US 101 must slow down in the outside travel lane on US 101. This creates a safety and operations risk. This is considered a Class II Impact – significant, but mitigable.

Mitigation T-3: Install a short right turn lane of approximately 200 feet in length for southbound traffic at the Quarry entrance. This lane should not extend to Black Bart Drive to avoid having drivers destined for the Quarry entering the right-turn lane north of Black Bart Drive.

Year 2014 (without Willits Bypass)

See Mitigation Measures T-1, T-2 and T-3; no new mitigation measures would be warranted.

Year 2014 (with Willits Bypass)

See Mitigation Measures T-1, T-2 and T-3; no new mitigation measures would be warranted.

Year 2030

See Mitigation Measures T-1, T-2 and T-3 in addition to the following new mitigation measure.

Impact T-4: Trucks turning right to US 101 southbound when exiting the Quarry must enter the outside travel lane after waiting for an acceptable gap in southbound traffic. Under projected volumes, gaps in traffic may not always be sufficient to allow for full acceleration in the travel lane. This is considered a Class II Impact – significant, but mitigable.

Mitigation T-4: Install an acceleration taper for trucks making the right turn onto US 101 southbound at the Quarry entrance to allow for some acceleration prior to entering the travel lane.

Year 2040

See Mitigation Measures T-1, T-2, T-3 and T-4; no new mitigation measures would be warranted.

Project Conditions

Year 2010

Impacts T-1 through T-4 would be expected by horizon year 2010, resulting in the need for Mitigation Measures T-1, T-2, T-3 and T-4.

Year 2025

See Mitigation Measures T-1, T-2, T-3 and T-4; no new mitigation measures would be warranted.

Year 2040

See Mitigation Measures T-1, T-2, T-3 and T-4 in addition to the following new mitigation measure.

Impact T-5: Under projected 2040 traffic volumes the right-turn acceleration taper indicated as Mitigation Measure T-4 would no longer be adequate and a full acceleration lane would be warranted. This is considered a Class II Impact – significant, but mitigable.

Mitigation T-5: Install a 1,390-foot-long speed change acceleration lane for trucks turning right onto US 101 southbound at the Quarry entrance to allow for full acceleration prior to entering the travel lane.

Conclusions and Recommendations

Conclusions

- During peak operating conditions, the project is expected to generate 310 truck trips in July and 412 truck trips in October. This is an increase of 196 and 262 truck trips from current operating conditions for July and October, respectively.
- During average operating conditions the project is expected to generate 215 truck trips in July and 286 truck trips in October. This is an increase of 131 and 182 truck trips from current operating conditions for July and October, respectively.
- The project has the potential to reduce vehicle miles traveled (VMT) associated with hauling aggregate and AC within the County by approximately 183,500 VMT annually.
- Northbound left turn deceleration and acceleration lanes are warranted in 2010 under Base Permit and Project conditions.
- A southbound right turn deceleration taper is warranted in 2010 under Base Permit and Project conditions; however, due to the proximity to Black Bart Drive, installation of a full deceleration lane that extends to Black Bart Drive would be beneficial.
- A southbound right turn acceleration taper is warranted in 2030 for continued operation of the quarry at its currently permitted levels.
- A southbound right turn acceleration taper is warranted in 2010 for Project Conditions. By the year 2040, a full right turn acceleration lane is warranted.
- Without the implementation of proposed mitigating measures, the intersection of US 101 and Black Bart Drive is expected to operate unacceptably in 2030 and 2040 with or without expansion of Quarry operations.
- With the implementation of recommended mitigating measures, the intersection of US 101 and Black Bart Drive is expected to continue to operate acceptably through the horizon year of 2040.
- The proposed project is not expected to generate a need for additional pedestrian, bicycle or transit facilities.
- Under the Project scenario and with the implementation of all recommended mitigating measures, the intersection of US 101 and Black Bart Drive is expected to operate at the same or better levels of service than without the project and associated mitigating measures for all scenarios and during all study periods, resulting in less average delay. This appears to be consistent with the Mendocino County General Plan policy DE-121.
- A partial grade-separated driveway would eliminate left-turning movements across oncoming traffic and would operate acceptably during all study time periods. However, based upon projected operations

of the at-grade intersection with the implementation of all recommended mitigating measures, a partial grade-separation is not warranted.

Recommendations

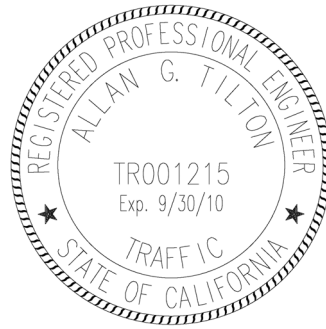
- Install a 469-foot long left-turn lane on US 101 North; this improvement is required regardless of whether the Quarry continues operation at the currently permitted levels or expands its operation.
- Install a 1,710-foot long acceleration lane on US 101 North; this improvement is required regardless of whether the Quarry continues operation at the currently permitted levels or expands its operation.
- Install a right turn lane of approximately 200 feet in length on US 101 South at the Harris Quarry Entrance; this improvement is required regardless of whether the Quarry continues operation at the currently permitted levels or expands its operation. This lane should be configured in a manner to prevent drivers destined for the Quarry from entering the lane north of Black Bart Drive as this may cause confusion for drivers on Black Bart Drive.
- Install a right turn acceleration taper on US 101 South; this improvement is needed immediately for the project, but not until 2030 if operation continues at the currently permitted levels. This lane will need to be converted to a full acceleration lane 1,390 feet in length by 2040 to accommodate the Project.
- Since the Quarry Access Driveway is expected to operate acceptably during all study periods with the implementation of the recommended mitigating measures, and since there are no documented historical safety issues associated with trucks turning at the project driveway, the installation of the partial grade-separation is not recommended.
- Installation of “Slow Trucks” warning signs with flashing beacons is recommended for both the northbound and southbound approaches to the Quarry Access Driveway.

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Study Participants and References

Study Participants

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Project Engineer: Allan G. Tilton, PE
Assistant Engineer: Tony Henderson, EIT
Graphics/Technician: Deborah J. Mizell
Editing/Formatting: Angela McCoy



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Communications

Letter regarding the required minimum length of the left-turn lane, Mr. Jesse Robertson of California Department of Transportation, August 8, 2006
Letter regarding the required minimum length of the left-turn lane, Mr. Jesse Robertson of California Department of Transportation, September 28, 2006

MEX047

Appendix A

Speed Surveys

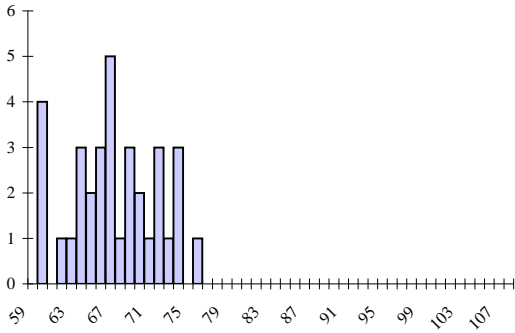
**W-Trans
Engineering and Traffic Survey**

VEHICLE SPEED DATA

Location: US 101 South of Black Bark
 Posted Limit: 65
 Direction: North #1 Recorder: CS
 Date: Mar 11-08 Day: Tuesday
 Begin Time: 5:50 PM End Time: 6:23 PM
 Weather: Clear Land Use: Rural

Summary Statistics

Total Observed 34
 Speed Range 60 - 76
 50th percentile speed 67
 85th percentile speed 72
 10 mph pace speed 63 - 72
 % in pace speed 71
 Skew 1.876



Speed	Number	Cumulative Percent	Speed	Number	Cumulative Percent
59	0	0	84	0	100
60	4	12	85	0	100
61	0	12	86	0	100
62	1	15	87	0	100
63	1	18	88	0	100
64	3	26	89	0	100
65	2	32	90	0	100
66	3	41	91	0	100
67	5	56	92	0	100
68	1	59	93	0	100
69	3	68	94	0	100
70	2	74	95	0	100
71	1	76	96	0	100
72	3	85	97	0	100
73	1	88	98	0	100
74	3	97	99	0	100
75	0	97	100	0	100
76	1	100	101	0	100
77	0	100	102	0	100
78	0	100	103	0	100
79	0	100	104	0	100
80	0	100	105	0	100
81	0	100	106	0	100
82	0	100	107	0	100
83	0	100	108	0	100
TOTAL	34				

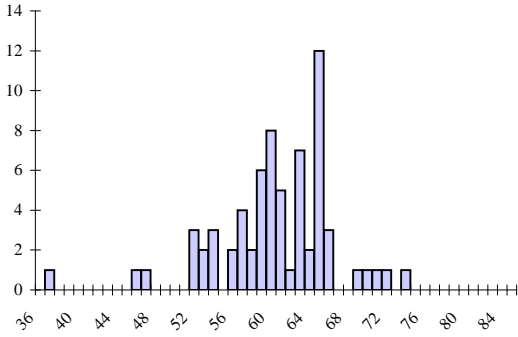
**W-Trans
Engineering and Traffic Survey**

VEHICLE SPEED DATA

Location: US 101 South of Black Bark
 Posted Limit: 65
 Direction: North #2 Recorder: CS
 Date: Mar 11-08 Day: Tuesday
 Begin Time: 5:50 PM End Time: 6:23 PM
 Weather: Clear Land Use: Rural

Summary Statistics

Total Observed 68
 Speed Range 37 - 74
 50th percentile speed 61
 85th percentile speed 65
 10 mph pace speed 57 - 66
 % in pace speed 74
 Skew 2.591



Speed	Number	Cumulative Percent	Speed	Number	Cumulative Percent
36	0	0	61	5	56
37	1	1	62	1	57
38	0	1	63	7	68
39	0	1	64	2	71
40	0	1	65	12	88
41	0	1	66	3	93
42	0	1	67	0	93
43	0	1	68	0	93
44	0	1	69	1	94
45	0	1	70	1	96
46	1	3	71	1	97
47	1	4	72	1	99
48	0	4	73	0	99
49	0	4	74	1	100
50	0	4	75	0	100
51	0	4	76	0	100
52	3	9	77	0	100
53	2	12	78	0	100
54	3	16	79	0	100
55	0	16	80	0	100
56	2	19	81	0	100
57	4	25	82	0	100
58	2	28	83	0	100
59	6	37	84	0	100
60	8	49	85	0	100
TOTAL	68				

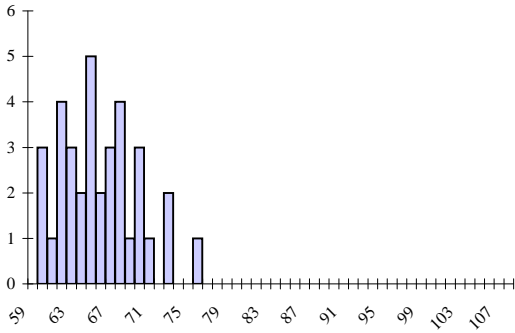
**W-Trans
Engineering and Traffic Survey**

VEHICLE SPEED DATA

Location: US 101 South of Black Bark
 Posted Limit: 65
 Direction: South #1 Recorder: CS
 Date: Mar 11-08 Day: Tuesday
 Begin Time: 6:25 PM End Time: 7:02 PM
 Weather: Clear Land Use: Rural

Summary Statistics

Total Observed 35
 Speed Range 60 - 76
 50th percentile speed 65
 85th percentile speed 70
 10 mph pace speed 60 - 69
 % in pace speed 80
 Skew 1.825



Speed	Number	Cumulative Percent	Speed	Number	Cumulative Percent
59	0	0	84	0	100
60	3	9	85	0	100
61	1	11	86	0	100
62	4	23	87	0	100
63	3	31	88	0	100
64	2	37	89	0	100
65	5	51	90	0	100
66	2	57	91	0	100
67	3	66	92	0	100
68	4	77	93	0	100
69	1	80	94	0	100
70	3	89	95	0	100
71	1	91	96	0	100
72	0	91	97	0	100
73	2	97	98	0	100
74	0	97	99	0	100
75	0	97	100	0	100
76	1	100	101	0	100
77	0	100	102	0	100
78	0	100	103	0	100
79	0	100	104	0	100
80	0	100	105	0	100
81	0	100	106	0	100
82	0	100	107	0	100
83	0	100	108	0	100
TOTAL	35				

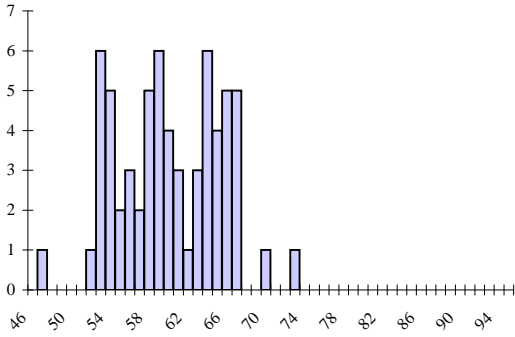
**W-Trans
Engineering and Traffic Survey**

VEHICLE SPEED DATA

Location: US 101 South of Black Bark
 Posted Limit: 65
 Direction: South #2 Recorder: CS
 Date: Mar 11-08 Day: Tuesday
 Begin Time: 6:25 PM End Time: 7:02 PM
 Weather: Clear Land Use: Rural

Summary Statistics

Total Observed 64
 Speed Range 47 - 73
 50th percentile speed 60
 85th percentile speed 66
 10 mph pace speed 58 - 67
 % in pace speed 66
 Skew 1.341



Speed	Number	Cumulative Percent	Speed	Number	Cumulative Percent
46	0	0	71	0	98
47	1	2	72	0	98
48	0	2	73	1	100
49	0	2	74	0	100
50	0	2	75	0	100
51	0	2	76	0	100
52	1	3	77	0	100
53	6	13	78	0	100
54	5	20	79	0	100
55	2	23	80	0	100
56	3	28	81	0	100
57	2	31	82	0	100
58	5	39	83	0	100
59	6	48	84	0	100
60	4	55	85	0	100
61	3	59	86	0	100
62	1	61	87	0	100
63	3	66	88	0	100
64	6	75	89	0	100
65	4	81	90	0	100
66	5	89	91	0	100
67	5	97	92	0	100
68	0	97	93	0	100
69	0	97	94	0	100
70	1	98	95	0	100
TOTAL	64				

Appendix B

Collision Data

SEGMENT COLLISION CALCULATIONS

County of Mendocino

Location: US 101 from one-half mile south of Black Bart Drive
to one-half mile north of Black Bart Drive

AADT: 17,000

Number of Collisions: 16
Number of Injuries: 5
Number of Fatalities: 1
Start Date: January 1, 2004
End Date: December 31, 2008
Number of Years: 5

Highway Type: UNDIVIDED 4 LANES
Area: Rural

Segment Length: 1.0 miles
Direction: NORTH/SOUTH

$$\frac{\text{NUMBER OF COLLISIONS} \times 1 \text{ MILLION}}{\text{AADT} \times 365 \text{ DAYS PER YEAR} \times \text{SEGMENT LENGTH} \times \text{NUMBER OF YEARS}}$$

$$\frac{16 \times 1,000,000}{17,000 \times 365 \times 1 \times 5}$$

	Collision Rate	Fatality Rate	Injury Rate
Study Segment	0.52 c/mvm	6.3%	31.3%
Statewide Average*	1.65 c/mvm	2.2%	38.4%

ADT = average daily traffic volume (adjusted for seasonal & weekday changes)

c/mvm = collisions per million vehicle miles

* 2007 Collision Data on California State Highways, Caltrans

Harris Quarry US 101 at or near Black Bart Drive

Collision Report Summary

8/31/2009

Date Range Reported: 1/1/04 - 12/31/08

Total Number of Collisions: 16

Report#	Date	Time	Location	Dist.	Dir.	Type of Collision	Motor Veh. Involved With	Dir. of Travel 1	Movement Prec. Coll. 1	Dir. of Travel 2	Movement Prec. Coll. 2	PCF	Inj.	Kil. Ver.
1235134	1/11/04	13:45	Rt 101 & Black Bart Dr	866'	South	Hit Object	Fixed Object	South	Ran Off Road			Improper Turning	0	1
1698994	10/24/04	22:19	Rt 101 & Black Bart Dr	317'	South	Rear-End	Other Motor Vehicle	South	Entering Traffic	South	Proceeding Straight	Auto RW Violation	2	0
1704587	10/30/04	15:35	Rt 101 & Black Bart Dr	389'	South	Hit Object	Animal	North	Proceeding Straight			Other Than Driver or Ped	0	0
1787815	12/20/04	09:45	Rt 101 & Black Bart Dr	1637'	North	Hit Object	Fixed Object	North	Ran Off Road			Wrong Side of Road	1	0
2005792	4/21/05	09:55	Rt 101 & Black Bart Dr	2112'	South	Hit Object	Fixed Object	North	Ran Off Road			Improper Turning	1	0
2248593	9/18/05	20:00	Rt 101 & Black Bart Dr	110'	South	Not Stated	Animal	North	Proceeding Straight			Other Than Driver or Ped	0	0
2379298	12/12/05	17:05	Rt 101 & Black Bart Dr	380'	South	Hit Object	Fixed Object	North	Ran Off Road			Other Than Driver or Ped	0	0
2870021	11/1/06	06:25	Rt 101 & Black Bart Dr	1214'	South	Hit Object	Animal	South	Proceeding Straight			Other Than Driver or Ped	0	0
2962979	12/21/06	08:48	Rt 101 & Black Bart Dr	1320'	North	Head-On	Not Stated	South	Proceeding Straight			Unsafe Speed	0	0
2963038	12/22/06	06:10	Rt 101 & Black Bart Dr	2640'	South	Head-On	Not Stated	South	Proceeding Straight			Unsafe Speed	2	0
2957604	12/22/06	06:15	Rt 101 & Black Bart Dr	2640'	South	Sideswipe	Other Motor Vehicle	South	Proceeding Straight	South	Stopped in Road	Other Than Driver or Ped	0	0
3307284	8/1/07	08:40	Rt 101 & Black Bart Dr	1373'	South	Hit Object	Fixed Object	North	Ran Off Road			Unsafe Speed	0	0
3398898	10/29/07	09:43	Rt 101 & Black Bart Dr	1584'	North	Hit Object	Fixed Object	North	Other Unsafe Turning			Improper Turning	1	0

Report#	Date	Time	Location	Dist.	Dir.	Type of Collision	Motor Veh. Involved With	Dir. of Travel 1	Movement Prec. Coll. 1	Dir. of Travel 2	Movement Prec. Coll. 2	PCF	Inj.	Kil. Ver.
3575132	1/8/08	16:00	Rt 101 & Black Bart Dr	686'	South	Hit Object	Fixed Object	North	Proceeding Straight			Unsafe Speed	0	0
3710134	4/11/08	20:50	Rt 101 & Black Bart Dr	500'	North	Hit Object	Animal	South	Proceeding Straight			Other Than Driver or Ped	0	0
3750801	5/25/08	07:47	Rt 101 & Black Bart Dr	1584'	North	Hit Object	Fixed Object	South	Ran Off Road			Unsafe Speed	0	0

Appendix C

Truck Trip Calculations

Harris Quarry – Truck Trip Calculations

Calculation Assumptions

Production Peak Levels (percent of annual total production)

Aggregate Use	July	October
Aggregate for Sale (including Recycle Import)	9.4%	12.2%
Aggregate to Willits for PCC	10.0%	15.8%
Aggregate for On-Site Asphalt Production	12.0%	17.0%

- Average of 4.15 weeks per month which results in eleven non-working days to account for holidays, inclement weather or closures for other reasons.
- Peak day trips represent 25 percent of total weekly trips
- Average of nine additional delivery trucks per day (one per hour)
- Peak productions represents 146.7 percent of average monthly production
- Each truck generates two trip ends (one inbound and one outbound)

Production Levels

Bulking Factor

$$\frac{\text{Unit Weight of Mined Material}}{\text{Unit Weight of Processed Material}} = \text{Bulking Factor}$$

$$\frac{4,185 \frac{\text{pounds}}{\text{cubic yard}}}{3,375 \frac{\text{pounds}}{\text{cubic yard}}} = 1.24$$

Base Permit – Annual Production

$$(\text{Annual Production}) \times (\text{Bulking Factor}) = \text{Adjusted Production}$$

$$(75,000 \text{ cubic yards}) \times (1.24) = 93,000 \frac{\text{cubic yards}}{\text{year}}$$

All Base Permit aggregate is sold offsite

Project – Annual Production

$$(\text{Annual Production}) \times (\text{Bulking Factor}) + \text{Recycled Material} = \text{Adjusted Production}$$

$$(200,000 \text{ cubic yards}) \times (1.24) + (\text{Recycled Material}) = 248,000 \frac{\text{cubic yards}}{\text{year}}$$

Breakdown of use of aggregate

Aggregate Use	Production Level
Aggregate for Sale ¹	162,520 cy/year
Aggregate to Willits for PCC	37,200 cy/year
Aggregate for On-Site Asphalt Production	58.280 cy/year
Total Aggregate Produced	258,000 cy/year
Imported Recycle Material ²	10,000 cy/year
Total Aggregate Transported	268,000 cy/year

Notes:

1. Includes sale of processed recycle material.
2. Although the Quarry expects that Imported Recycle Material will be brought in on trucks that will subsequently be refilled and used to export material, to provide a conservative analysis it was assumed that trucks that supply recycle material will then leave the site empty. Imported Recycle Material is assumed to generate traffic with the same peak period patterns as aggregate for sale.

Base Permit – July Average Production

July represents 9.4% of annual production

$$\begin{aligned} \text{Monthly Trucks} &= \frac{(\text{Adjusted Production}) \times (\text{Monthly Factor})}{(\text{Truck Capacity})} \\ &= \frac{(93,000 \text{ cubic yards/year}) \times (0.094 \frac{\text{cubic yards/month}}{\text{cubic yards/year}})}{(16 \text{ cubic yards/truck})} \\ &= 546.4 \text{ trucks/month} \end{aligned}$$

Average of 4.15 weeks per month

$$\begin{aligned} \text{Weekly Trucks} &= \frac{\text{Monthly Trucks}}{\text{Weeks per Month}} \\ \frac{546.4 \text{ trucks/month}}{4.15 \text{ weeks/month}} &= 131.7 \frac{\text{trucks}}{\text{week}} \approx 132 \frac{\text{trucks}}{\text{week}} \end{aligned}$$

Peak day represents 25% of the weekly trips

$$\begin{aligned} \text{Daily Truck Trips} &= (\text{Weekly Truck Trips}) \times (\text{Daily Factor}) \\ 132 \frac{\text{trucks}}{\text{week}} \times 0.25 &= 33 \text{ trucks/day} \end{aligned}$$

Additional 9 delivery truck trips per day

$$\begin{aligned} \text{Total Truck Trips} &= (\text{Daily Truck Trips}) + (\text{Delivery Truck Trips}) \\ 33 \frac{\text{Trucks}}{\text{Day}} + 9 \frac{\text{Delivery Truck Trips}}{\text{Day}} &= 42 \frac{\text{Trucks}}{\text{Day}} \end{aligned}$$

Base Permit – July Peak Production

July represents 9.4% of annual production

$$\begin{aligned} \text{Monthly Trucks} &= \frac{(\text{Adjusted Production}) \times (\text{Monthly Factor})}{(\text{Truck Capacity})} \\ &= \frac{(93,000 \text{ cubic yards/year}) \times (0.094 \frac{\text{cubic yards/month}}{\text{cubic yards/year}})}{(16 \text{ cubic yards/truck})} \\ &= 546.4 \text{ trucks/month} \end{aligned}$$

Average of 4.15 weeks per month

$$\begin{aligned} \text{Weekly Trucks} &= \frac{\text{Monthly Trucks}}{\text{Weeks per Month}} \\ \frac{546.4 \text{ trucks/month}}{4.15 \text{ weeks/month}} &= 131.7 \frac{\text{trucks}}{\text{week}} \approx 132 \frac{\text{trucks}}{\text{week}} \end{aligned}$$

Peak Production is 146.7% of average monthly production

$$\begin{aligned} \text{Peak Weekly Trips} &= (\text{Average Weekly Trips}) \times (\text{Peak Factor}) \\ \left(132 \frac{\text{Average Trucks}}{\text{week}} \right) \times 1.467 &= 193.64 \frac{\text{Peak Trucks}}{\text{Week}} \end{aligned}$$

Peak day represents 25% of the weekly trips

$$\begin{aligned} \text{Daily Truck Trips} &= (\text{Weekly Truck Trips}) \times (\text{Daily Factor}) \\ 193.64 \frac{\text{trucks}}{\text{week}} \times 0.25 &= 48.4 \approx 48 \text{ peak trucks/day} \end{aligned}$$

Additional 9 delivery trucks per day

$$\begin{aligned} \text{Total Truck Trips} &= (\text{Daily Truck Trips}) + (\text{Delivery Truck Trips}) \\ 48 \frac{\text{Trucks}}{\text{Day}} + 9 \frac{\text{Delivery Truck Trips}}{\text{Day}} &= 57 \frac{\text{Trucks}}{\text{Day}} \end{aligned}$$

Base Permit – October Average Production

October represents 12.2% of annual production

$$\begin{aligned} \text{Monthly Trucks} &= \frac{(\text{Adjusted Production}) \times (\text{Monthly Factor})}{(\text{Truck Capacity})} \\ &= \frac{(93,000 \text{ cubic yards/year}) \times (0.122 \frac{\text{cubic yards/month}}{\text{cubic yards/year}})}{(16 \text{ cubic yards/truck})} \\ &= 709.1 \text{ trucks/month} \end{aligned}$$

Average of 4.15 weeks per month

$$\begin{aligned} \text{Weekly Trucks} &= \frac{\text{Monthly Trucks}}{\text{Weeks per Month}} \\ &= \frac{709.1 \text{ trucks/month}}{4.15 \text{ weeks/month}} = 170.87 \frac{\text{trucks}}{\text{week}} \approx 171 \frac{\text{trucks}}{\text{week}} \end{aligned}$$

Peak day represents 25% of the weekly trips

$$\begin{aligned} \text{Daily Truck Trips} &= (\text{Weekly Truck Trips}) \times (\text{Daily Factor}) \\ &= 171 \frac{\text{trucks}}{\text{week}} \times 0.25 = 42.75 \approx 43 \text{ trucks/day} \end{aligned}$$

Additional 9 delivery truck trips per day

$$\begin{aligned} \text{Total Truck Trips} &= (\text{Daily Truck Trips}) + (\text{Delivery Truck Trips}) \\ &= 43 \frac{\text{Trucks}}{\text{Day}} + 9 \frac{\text{Delivery Truck Trips}}{\text{Day}} = 52 \frac{\text{Trucks}}{\text{Day}} \end{aligned}$$

Base Permit – October Peak Production

October represents 12.2% of annual production

$$\begin{aligned} \text{Monthly Trucks} &= \frac{(\text{Adjusted Production}) \times (\text{Monthly Factor})}{(\text{Truck Capacity})} \\ &= \frac{(93,000 \text{ cubic yards/year}) \times (0.122 \frac{\text{cubic yards/month}}{\text{cubic yards/year}})}{(16 \text{ cubic yards/truck})} \\ &= 709.1 \text{ trucks/month} \end{aligned}$$

Average of 4.15 weeks per month

$$\begin{aligned} \text{Weekly Trucks} &= \frac{\text{Monthly Trucks}}{\text{Weeks per Month}} \\ &= \frac{709.1 \text{ trucks/month}}{4.15 \text{ weeks/month}} = 170.8 \frac{\text{trucks}}{\text{week}} \approx 171 \frac{\text{trucks}}{\text{week}} \end{aligned}$$

Peak Production is 146.7% of average monthly production

$$\begin{aligned} \text{Peak Weekly Trips} &= (\text{Average Weekly Trips}) \times (\text{Peak Factor}) \\ &= \left(171 \frac{\text{Average Trucks}}{\text{week}} \right) \times 1.467 = 250.9 \frac{\text{Peak Trucks}}{\text{Week}} \end{aligned}$$

Peak day represents 25% of the weekly trips

$$\begin{aligned} \text{Daily Truck Trips} &= (\text{Weekly Truck Trips}) \times (\text{Daily Factor}) \\ &= 250.9 \frac{\text{trucks}}{\text{week}} \times 0.25 = 62.7 \approx 63 \text{ peak trucks/day} \end{aligned}$$

Additional 9 delivery trucks per day

$$\begin{aligned} \text{Total Truck Trips} &= (\text{Daily Truck Trips}) + (\text{Delivery Truck Trips}) \\ &= 63 \frac{\text{Trucks}}{\text{Day}} + 9 \frac{\text{Delivery Truck Trips}}{\text{Day}} = 72 \frac{\text{Trucks}}{\text{Day}} \end{aligned}$$

**Project – July Average Production
Aggregate for Sale and Recycle Import**

July represents 9.4% of annual production

$$\begin{aligned} \text{Monthly Truck} &= \frac{(\text{Adjusted Production} + \text{Recycle Import}) \times (\text{Monthly Factor})}{(\text{Truck Capacity})} \\ &= \frac{(162,520 + 10,000 \text{ cy/year}) \times (0.094 \frac{\text{cubic yards/month}}{\text{cubic yards/year}})}{(16 \text{ cubic yards/truck})} \\ &= 1013.6 \text{ trucks/month} \end{aligned}$$

Average of 4.15 weeks per month

$$\begin{aligned} \text{Weekly Trucks} &= \frac{\text{Monthly Trucks}}{\text{Weeks per Month}} \\ \frac{1013.6 \text{ trucks/month}}{4.15 \text{ weeks/month}} &= 244.2 \frac{\text{trucks}}{\text{week}} \cong 244 \frac{\text{trucks}}{\text{week}} \end{aligned}$$

Peak day represents 25% of the weekly trips

$$\begin{aligned} \text{Daily Truck Trips} &= (\text{Weekly Truck Trips}) \times (\text{Daily Factor}) \\ 244 \frac{\text{trucks}}{\text{week}} \times 0.25 &= 61 \text{ trucks/day} \end{aligned}$$

Aggregate to Willits for PCC

July represents 10% of annual production

$$\begin{aligned} \text{Monthly Trucks} &= \frac{(\text{Adjusted Production}) \times (\text{Monthly Factor})}{(\text{Truck Capacity})} \\ &= \frac{(37,200 \text{ cubic yards/year}) \times (0.10 \frac{\text{cubic yards/month}}{\text{cubic yards/year}})}{(20 \text{ cubic yards/truck})} \\ &= 186 \text{ trucks/month} \end{aligned}$$

An average of 4.15 weeks per month

$$\begin{aligned} \text{Weekly Trucks} &= \frac{\text{Monthly Trucks}}{\text{Weeks per Month}} \\ \frac{186 \text{ trucks/month}}{4.15 \text{ weeks/month}} &= 44.8 \frac{\text{trucks}}{\text{week}} \cong 45 \frac{\text{trucks}}{\text{week}} \end{aligned}$$

Peak day represents 25% of the weekly trips

$$\begin{aligned} \text{Daily Truck Trips} &= (\text{Weekly Truck Trips}) \times (\text{Daily Factor}) \\ 45 \frac{\text{trucks}}{\text{week}} \times 0.25 &= 11.25 \cong 11 \text{ trucks/day} \end{aligned}$$

**Project – July Average Production (continued)
Aggregate for On-Site Asphalt Production**

July represents 12% of annual production

$$\text{Monthly Trucks} = \frac{(\text{Adjusted Production}) \times (\text{Monthly Factor})}{(\text{Truck Capacity})}$$

$$\frac{\left(58,280 \text{ cubic } \frac{\text{yards}}{\text{year}}\right) \times \left(0.12 \frac{\text{cubic } \frac{\text{yards}}{\text{month}}}{\text{cubic } \frac{\text{yards}}{\text{year}}}\right)}{\left(16 \text{ cubic } \frac{\text{yards}}{\text{truck}}\right)} = 437.1 \frac{\text{trucks}}{\text{month}}$$

Average of 4.15 weeks per month

$$\text{Weekly Trucks} = \frac{\text{Monthly Trucks}}{\text{Weeks per Month}}$$

$$\frac{437.1 \text{ trucks/month}}{4.15 \text{ weeks/month}} = 105.3 \frac{\text{trucks}}{\text{week}} \approx 105 \frac{\text{trucks}}{\text{week}}$$

Peak day represents 25% of the weekly trips

$$\text{Daily Truck Trips} = (\text{Weekly Truck Trips}) \times (\text{Daily Factor})$$

$$105 \frac{\text{trucks}}{\text{week}} \times 0.25 = 26.25 \approx 26 \text{ trucks/day}$$

Total Truck Trips

Additional 9 delivery truck trips per day

$$\text{Total Truck Trips} = (\text{Gravel Sale \& Recycle Import}) + (\text{PCC Production}) + (\text{On Site Asphalt Production}) + (\text{Deliveries})$$

$$61 \frac{\text{trucks}}{\text{day}} + 11 \frac{\text{trucks}}{\text{day}} + 26 \frac{\text{trucks}}{\text{day}} + 9 \frac{\text{trucks}}{\text{day}} = 107 \frac{\text{trucks}}{\text{day}}$$

**Project – July Peak Production
Aggregate for Sale and Recycle Import**

July represents 9.4% of annual production

$$\text{Monthly Trucks} = \frac{(\text{Adjusted Production}) \times (\text{Monthly Factor})}{(\text{Truck Capacity})}$$

$$\frac{\left(162,520 \frac{\text{cy}}{\text{year}} + 10,000 \frac{\text{cy}}{\text{year}}\right) \times (0.094 \frac{\text{cy/month}}{\text{cy/year}})}{(16 \text{ cy/truck})} = 1013.6 \text{ trucks/month}$$

Average of 4.15 weeks per month

$$\text{Weekly Trucks} = \frac{\text{Monthly Trucks}}{\text{Weeks per Month}}$$

$$\frac{1013.6 \text{ trucks/month}}{4.15 \text{ weeks/month}} = 244.2 \frac{\text{trucks}}{\text{week}} \cong 244 \frac{\text{trucks}}{\text{week}}$$

Peak Production is 146.7% of average monthly production

$$\text{Peak Weekly Trips} = (\text{Average Weekly Trips}) \times (\text{Peak Factor})$$

$$\left(244 \frac{\text{Average Trucks}}{\text{week}}\right) \times 1.467 = 357.9 \frac{\text{Peak Trucks}}{\text{Week}}$$

Peak day represents 25% of the weekly trips

$$\text{Daily Truck Trips} = (\text{Weekly Truck Trips}) \times (\text{Daily Factor})$$

$$357.9 \frac{\text{trucks}}{\text{week}} \times 0.25 = 89.5 \cong 90 \text{ trucks/day}$$

Aggregate to Willits for PCC

July represents 10% of annual production

$$\text{Monthly Trucks} = \frac{(\text{Adjusted Production}) \times (\text{Monthly Factor})}{(\text{Truck Capacity})}$$

$$\frac{\left(37,200 \frac{\text{cy}}{\text{year}}\right) \times (0.10 \frac{\text{cy/month}}{\text{cy/year}})}{(20 \text{ cy/truck})} = 186 \text{ trucks/month}$$

Average of 4.15 weeks per month

$$\text{Weekly Trucks} = \frac{\text{Monthly Trucks}}{\text{Weeks per Month}}$$

$$\frac{186 \text{ trucks/month}}{4.15 \text{ weeks/month}} = 44.8 \frac{\text{trucks}}{\text{week}} \cong 45 \frac{\text{trucks}}{\text{week}}$$

Peak Production is 146.7% of average monthly production

$$\text{Peak Weekly Trips} = (\text{Average Weekly Trips}) \times (\text{Peak Factor})$$

$$\left(44.8 \frac{\text{Average Trucks}}{\text{week}}\right) \times 1.467 = 66 \frac{\text{Peak Trucks}}{\text{Week}}$$

Peak day represents 25% of the weekly trips

$$\text{Daily Truck Trips} = (\text{Weekly Truck Trips}) \times (\text{Daily Factor})$$

$$66 \frac{\text{trucks}}{\text{week}} \times 0.25 = 16.5 \cong 17 \text{ trucks/day}$$

**Project – July Peak Production (continued)
Aggregate for On-Site Asphalt Production**

July represents 12% of annual production

$$\text{Monthly Trucks} = \frac{(\text{Adjusted Production}) \times (\text{Monthly Factor})}{(\text{Truck Capacity})}$$

$$\frac{(58,280 \text{ cy/year}) \times (0.12 \frac{\text{cy/month}}{\text{cy/year}})}{(16 \text{ cy/truck})} = 437.1 \text{ trucks/month}$$

Average of 4.15 weeks per month

$$\text{Weekly Trucks} = \frac{\text{Monthly Trucks}}{\text{Weeks per Month}}$$

$$\frac{437.1 \text{ trucks/month}}{4.15 \text{ weeks/month}} = 105.3 \frac{\text{trucks}}{\text{week}} \approx 105 \frac{\text{trucks}}{\text{week}}$$

Peak Production is 146.7% of average monthly production

$$\text{Peak Weekly Trips} = (\text{Average Weekly Trips}) \times (\text{Peak Factor})$$

$$\left(105 \frac{\text{Average Trucks}}{\text{week}} \right) \times 1.467 = 154 \frac{\text{Peak Trucks}}{\text{Week}}$$

Peak day represents 25% of the weekly trips

$$\text{Daily Truck Trips} = (\text{Weekly Truck Trips}) \times (\text{Daily Factor})$$

$$154 \frac{\text{trucks}}{\text{week}} \times 0.25 = 38.5 \approx 39 \text{ trucks/day}$$

Total Truck Trips

Additional 9 delivery truck trips per day

$$\text{Total Truck Trips} = (\text{Gravel Sale \& Recycle Import}) + (\text{PCC Production})$$

$$+ (\text{On Site Asphalt Production}) + (\text{Deliveries})$$

$$90 \frac{\text{trucks}}{\text{day}} + 17 \frac{\text{trucks}}{\text{day}} + 39 \frac{\text{trucks}}{\text{day}} + 9 \frac{\text{trucks}}{\text{day}} = 155 \frac{\text{trucks}}{\text{day}}$$

**Project – October Average Production
Aggregate for Sale and Recycle Import**

Assume October represents 12.2% of annual production

$$\text{Monthly Trucks} = \frac{(\text{Adjusted Production}) \times (\text{Monthly Factor})}{(\text{Truck Capacity})}$$

$$\frac{\left(162,520 \frac{\text{cy}}{\text{year}} + 10,000 \frac{\text{cy}}{\text{year}}\right) \times \left(0.122 \frac{\text{cy/month}}{\text{cy/year}}\right)}{(16 \text{ cy/truck})} = 1315.5 \text{ trucks/month}$$

Assume an average of 4.15 weeks per month

$$\text{Weekly Trucks} = \frac{\text{Monthly Trucks}}{\text{Weeks per Month}}$$

$$\frac{1315.5 \text{ trucks/month}}{4.15 \text{ weeks/month}} = 317 \frac{\text{trucks}}{\text{week}}$$

Peak day represents 25% of the weekly trips

$$\text{Daily Truck Trips} = (\text{Weekly Truck Trips}) \times (\text{Daily Factor})$$

$$317 \frac{\text{trucks}}{\text{week}} \times 0.25 = 79.3 \cong 79 \text{ trucks/day}$$

Aggregate to Willits for PCC

October represents 15.8% of annual production

$$\text{Monthly Trucks} = \frac{(\text{Adjusted Production}) \times (\text{Monthly Factor})}{(\text{Truck Capacity})}$$

$$\frac{(37,200 \text{ cy/year}) \times \left(0.158 \frac{\text{cy/month}}{\text{cy/year}}\right)}{(20 \text{ cy/truck})} = 293.9 \text{ trucks/month}$$

Average of 4.15 weeks per month

$$\text{Weekly Trucks} = \frac{\text{Monthly Trucks}}{\text{Weeks per Month}}$$

$$\frac{293.9 \text{ trucks/month}}{4.15 \text{ weeks/month}} = 70.8 \frac{\text{trucks}}{\text{week}} \cong 71 \frac{\text{trucks}}{\text{week}}$$

Peak day represents 25% of the weekly trips

$$\text{Daily Truck Trips} = (\text{Weekly Truck Trips}) \times (\text{Daily Factor})$$

$$71 \frac{\text{trucks}}{\text{week}} \times 0.25 = 17.8 \cong 18 \text{ trucks/day}$$

**Project – October Average Production (continued)
Aggregate for On-Site Asphalt Production**

October represents 17% of annual production

$$\text{Monthly Trucks} = \frac{(\text{Adjusted Production}) \times (\text{Monthly Factor})}{(\text{Truck Capacity})}$$

$$\frac{(58,280 \text{ cy/year}) \times (0.17 \frac{\text{cy/month}}{\text{cy/year}})}{(16 \text{ cy/truck})} = 619.2 \text{ trucks/month}$$

Average of 4.15 weeks per month

$$\text{Weekly Trucks} = \frac{\text{Monthly Trucks}}{\text{Weeks per Month}}$$

$$\frac{619.2 \text{ trucks/month}}{4.15 \text{ weeks/month}} = 149.2 \frac{\text{trucks}}{\text{week}} \approx 149 \frac{\text{trucks}}{\text{week}}$$

Peak day represents 25% of the weekly trips

$$\text{Daily Truck Trips} = (\text{Weekly Truck Trips}) \times (\text{Daily Factor})$$

$$149 \frac{\text{trucks}}{\text{week}} \times 0.25 = 37.3 \approx 37 \text{ trucks/day}$$

Total Truck Trips

Additional 9 delivery truck trips per day

$$\text{Total Truck Trips} = (\text{Gravel Sale \& Recycle Import}) + (\text{PCC Production})$$

$$+ (\text{On Site Asphalt Production}) + (\text{Deliveries})$$

$$79 \frac{\text{trucks}}{\text{day}} + 18 \frac{\text{trucks}}{\text{day}} + 37 \frac{\text{trucks}}{\text{day}} + 9 \frac{\text{trucks}}{\text{day}} = 143 \frac{\text{trucks}}{\text{day}}$$

**Project – October Peak Production
Gravel for Sale**

October represents 12.2% of annual production

$$\text{Monthly Trucks} = \frac{(\text{Adjusted Production}) \times (\text{Monthly Factor})}{(\text{Truck Capacity})}$$

$$\frac{\left(162,520 \frac{\text{cy}}{\text{year}} + 10,000 \frac{\text{cy}}{\text{year}}\right) \times \left(0.122 \frac{\text{cy/month}}{\text{cy/year}}\right)}{(16 \text{ cy/truck})}$$

$$= 1315.5 \text{ trucks/month}$$

Average of 4.15 weeks per month

$$\text{Weekly Trucks} = \frac{\text{Monthly Trucks}}{\text{Weeks per Month}}$$

$$\frac{1315.5 \text{ trucks/month}}{4.15 \text{ weeks/month}} = 317 \frac{\text{trucks}}{\text{week}}$$

Peak Production is 146.7% of average monthly production

$$\text{Peak Weekly Trips} = (\text{Average Weekly Trips}) \times (\text{Peak Factor})$$

$$\left(317 \frac{\text{Average Trucks}}{\text{week}}\right) \times 1.467 = 465 \frac{\text{Peak Trucks}}{\text{Week}}$$

Peak day represents 25% of the weekly trips

$$\text{Daily Truck Trips} = (\text{Weekly Truck Trips}) \times (\text{Daily Factor})$$

$$465 \frac{\text{trucks}}{\text{week}} \times 0.25 = 116.3 \approx 116 \text{ trucks/day}$$

Aggregate to Willits for PCC

July represents 15.8% of annual production

$$\text{Monthly Trucks} = \frac{(\text{Adjusted Production}) \times (\text{Monthly Factor})}{(\text{Truck Capacity})}$$

$$\frac{(37,200 \text{ cubic yards/year}) \times \left(0.158 \frac{\text{cubic yards/month}}{\text{cubic yards/year}}\right)}{(20 \text{ cubic yards/truck})}$$

$$= 293.9 \text{ trucks/month}$$

Average of 4.15 weeks per month

$$\text{Weekly Trucks} = \frac{\text{Monthly Trucks}}{\text{Weeks per Month}}$$

$$\frac{293.9 \text{ trucks/month}}{4.15 \text{ weeks/month}} = 70.8 \frac{\text{trucks}}{\text{week}} \approx 71 \frac{\text{trucks}}{\text{week}}$$

Peak Production is 146.7% of average monthly production

$$\text{Peak Weekly Trips} = (\text{Average Weekly Trips}) \times (\text{Peak Factor})$$

$$\left(71 \frac{\text{Average Trucks}}{\text{week}}\right) \times 1.467 = 104.2 \frac{\text{Peak Trucks}}{\text{Week}}$$

Peak day represents 25% of the weekly trips

$$\text{Daily Truck Trips} = (\text{Weekly Truck Trips}) \times (\text{Daily Factor})$$

$$104.2 \frac{\text{trucks}}{\text{week}} \times 0.25 = 26.1 \approx 26 \text{ trucks/day}$$

**Project – October Peak Production (continued)
Aggregate for On-Site Asphalt Production**

October represents 17% of annual production

$$\text{Monthly Trucks} = \frac{(\text{Adjusted Production}) \times (\text{Monthly Factor})}{(\text{Truck Capacity})}$$

$$\frac{(58,280 \text{ cy/year}) \times (0.17 \frac{\text{cy/month}}{\text{cy/year}})}{(16 \text{ cy/truck})} = 619.2 \text{ trucks/month}$$

Average of 4.15 weeks per month

$$\text{Weekly Trucks} = \frac{\text{Monthly Trucks}}{\text{Weeks per Month}}$$

$$\frac{619.3 \text{ trucks/month}}{4.15 \text{ weeks/month}} = 149.2 \frac{\text{trucks}}{\text{week}} \approx 149 \frac{\text{trucks}}{\text{week}}$$

Peak Production is 146.7% of average monthly production

$$\text{Peak Weekly Trips} = (\text{Average Weekly Trucks}) \times (\text{Peak Factor})$$

$$\left(149 \frac{\text{Average Trucks}}{\text{week}} \right) \times 1.467 = 218.6 \frac{\text{Peak Trucks}}{\text{Week}}$$

Peak day represents 25% of the weekly trips

$$\text{Daily Truck Trips} = (\text{Weekly Truck Trips}) \times (\text{Daily Factor})$$

$$218.6 \frac{\text{trucks}}{\text{week}} \times 0.25 = 54.7 \approx 55 \text{ trucks/day}$$

Total Truck Trips

Additional 9 delivery truck trips per day

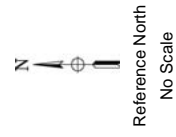
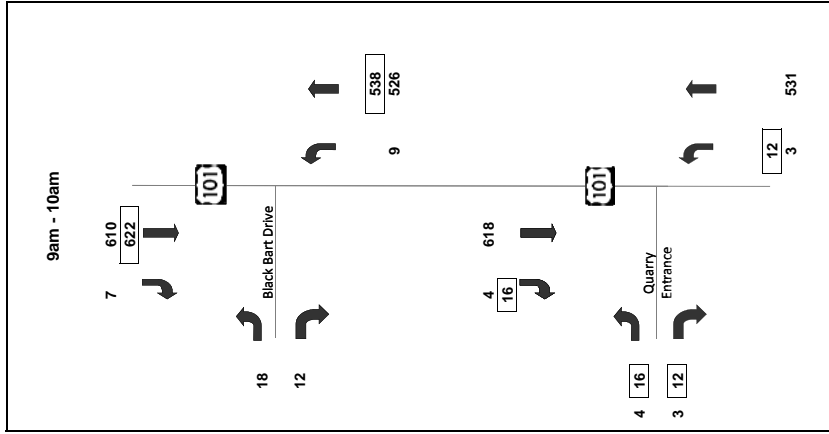
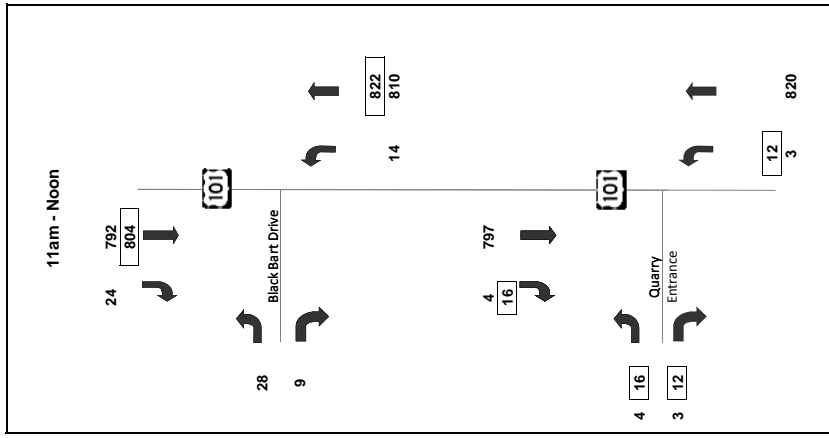
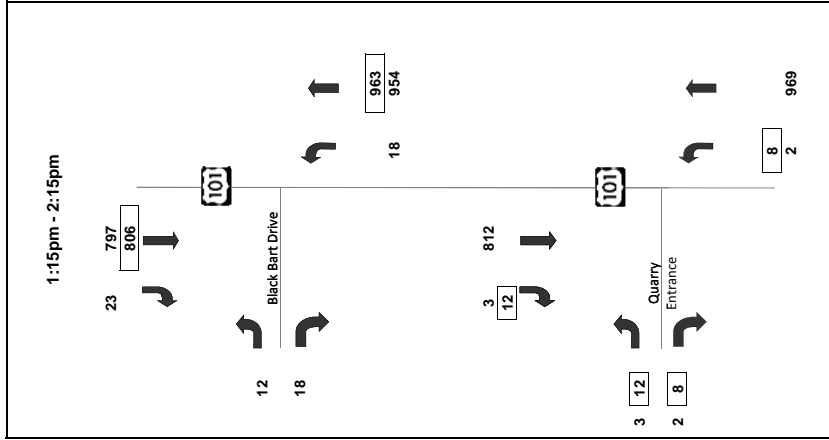
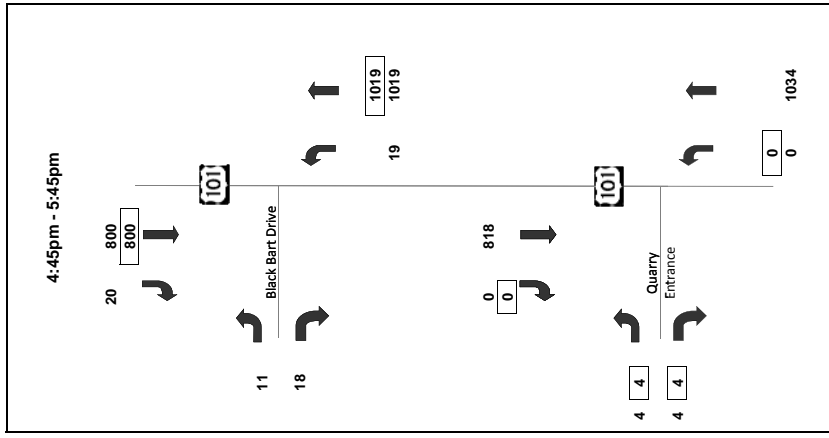
$$\text{Total Truck Trips} = (\text{Off Site Sale}) + (\text{PCC Production})$$

$$+ (\text{On Site Asphalt Production}) + (\text{Deliveries})$$

$$116 \frac{\text{trucks}}{\text{day}} + 26 \frac{\text{trucks}}{\text{day}} + 55 \frac{\text{trucks}}{\text{day}} + 9 \frac{\text{trucks}}{\text{day}} = 206 \frac{\text{trucks}}{\text{day}}$$

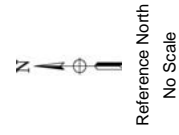
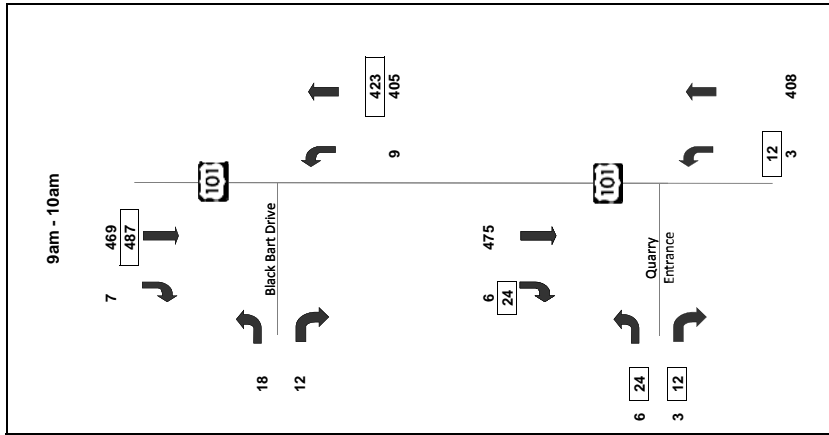
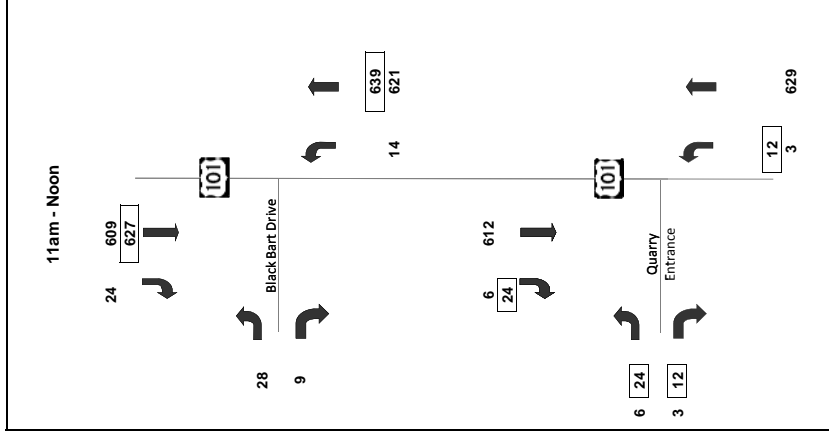
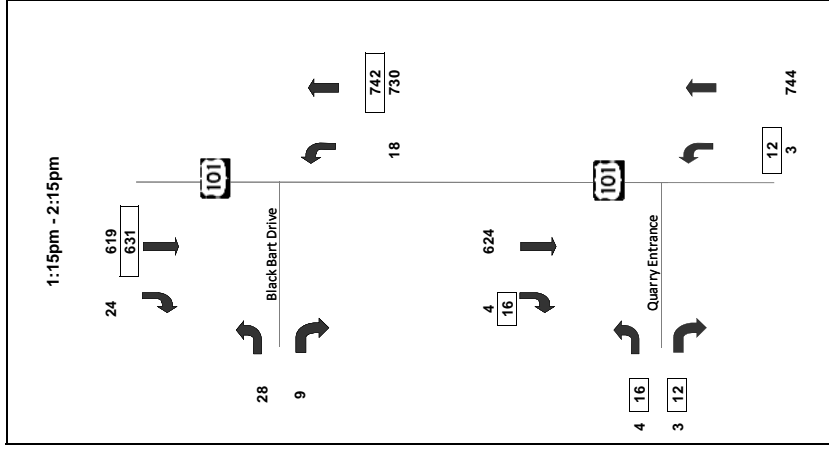
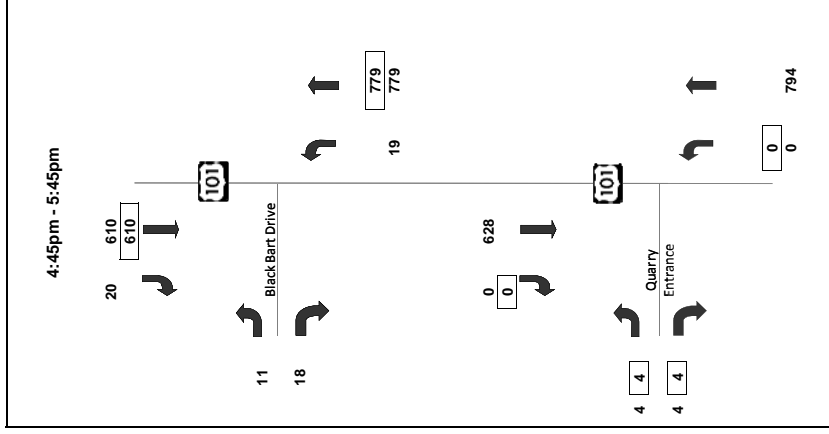
Appendix D

July and October Truck Volumes by Hour



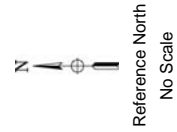
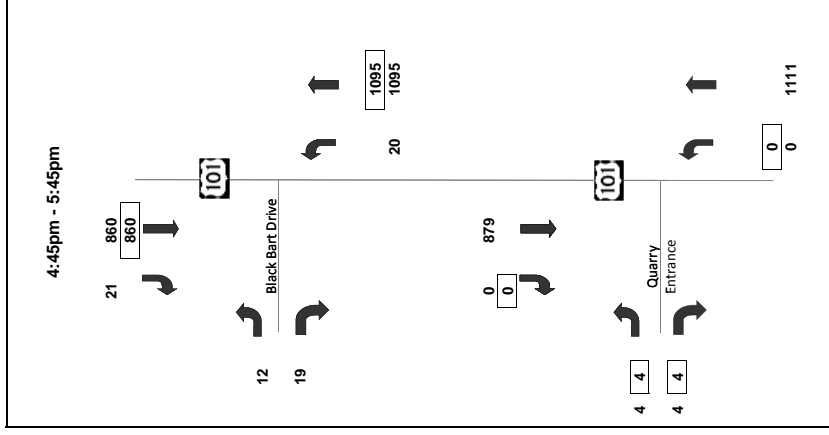
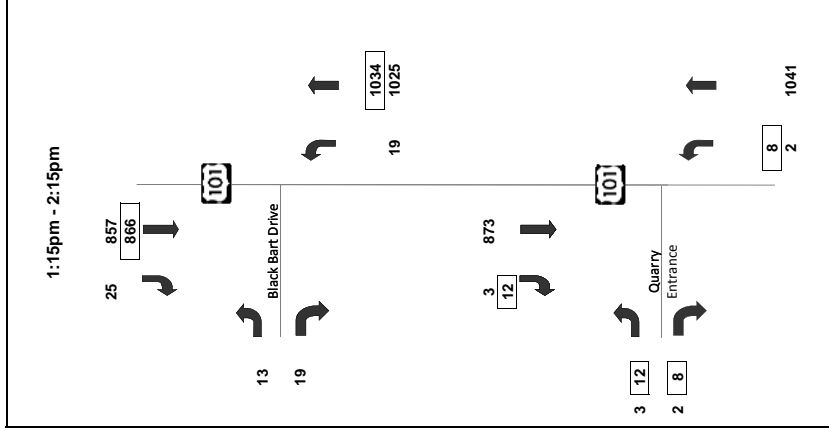
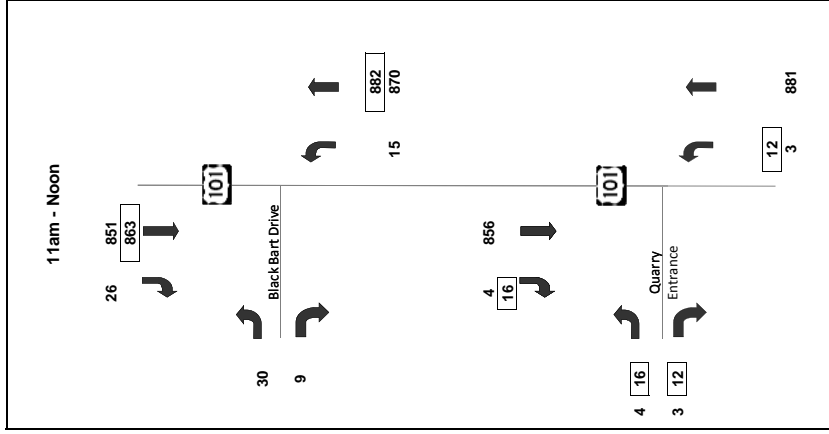
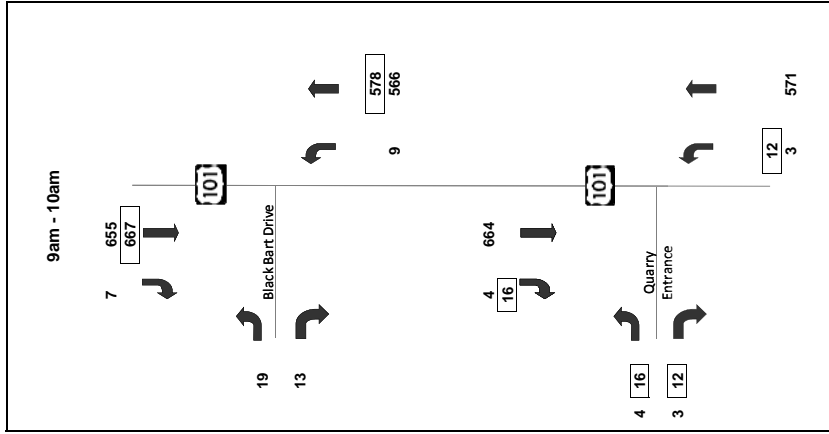
HARRIS QUARRY - TURN MOVEMENTS
2010
BASE CONDITIONS (75,000 CUBIC YARDS)
PRODUCTION:
JULY PEAK

Note:
 # = Trucks + Passenger Cars
 # = Passenger Car Equivalent
 Where:
 1 Truck = 4 Passenger Car Equivalents



HARRIS QUARRY - TURN MOVEMENTS
2010
BASE CONDITIONS (75,000 CUBIC YARDS)
PRODUCTION:
OCTOBER PEAK

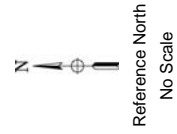
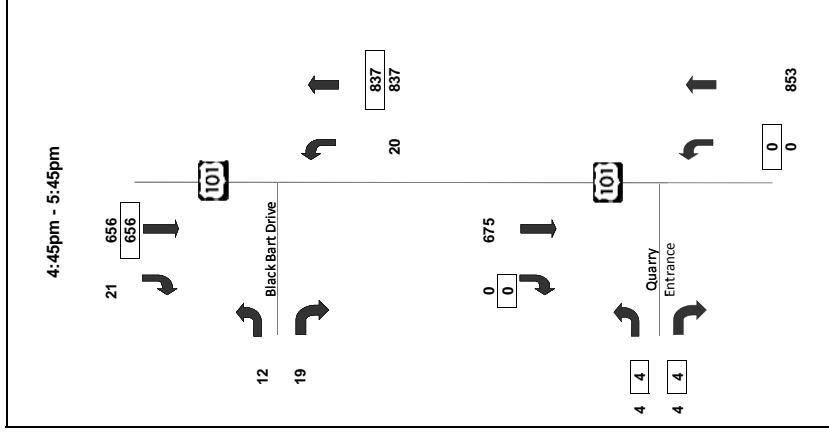
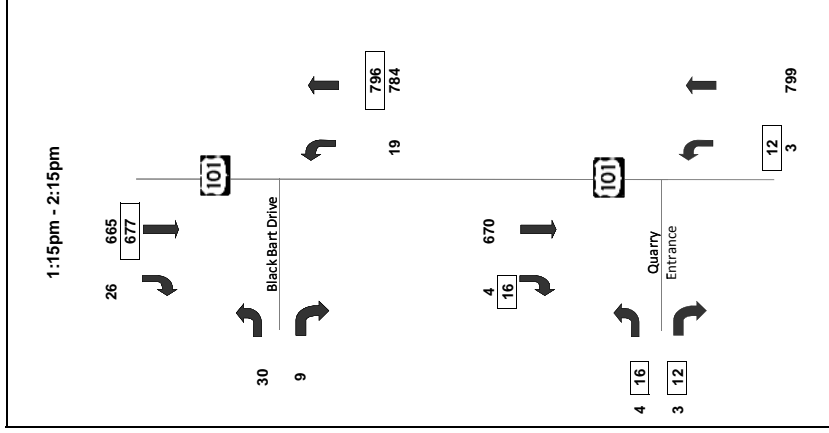
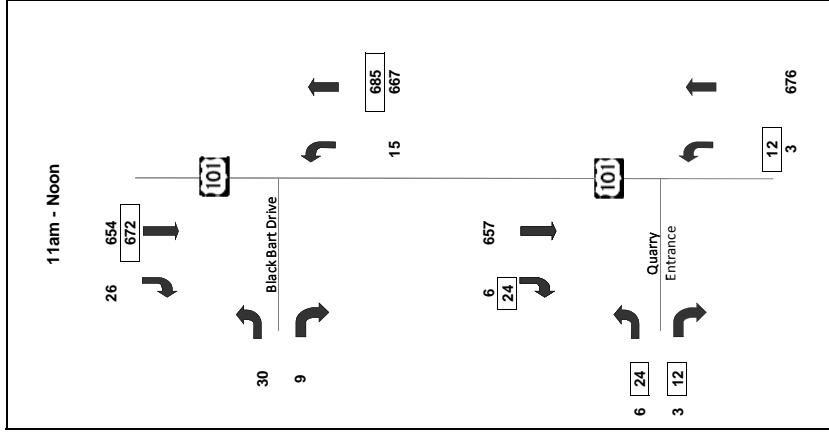
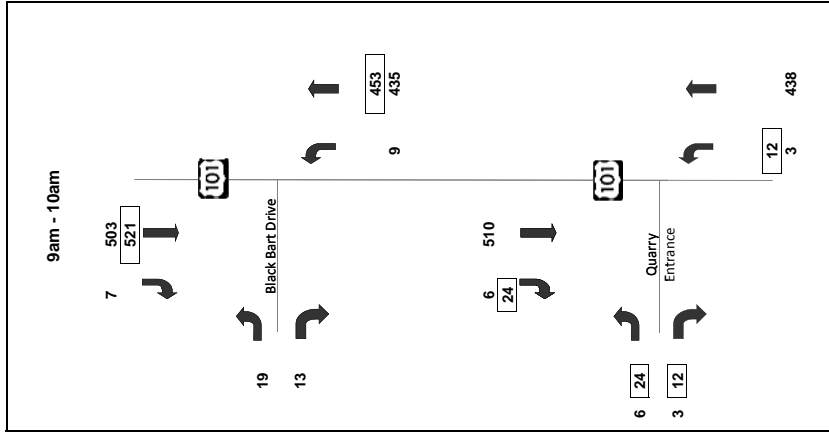
Note:
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Where:
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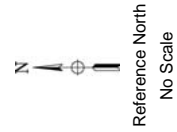
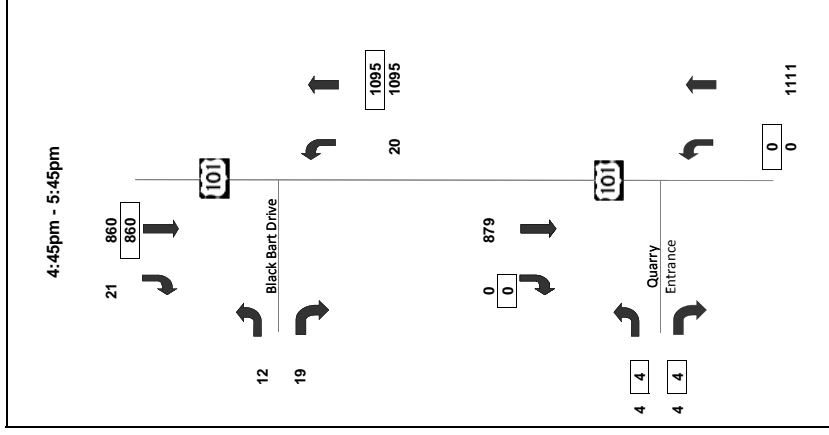
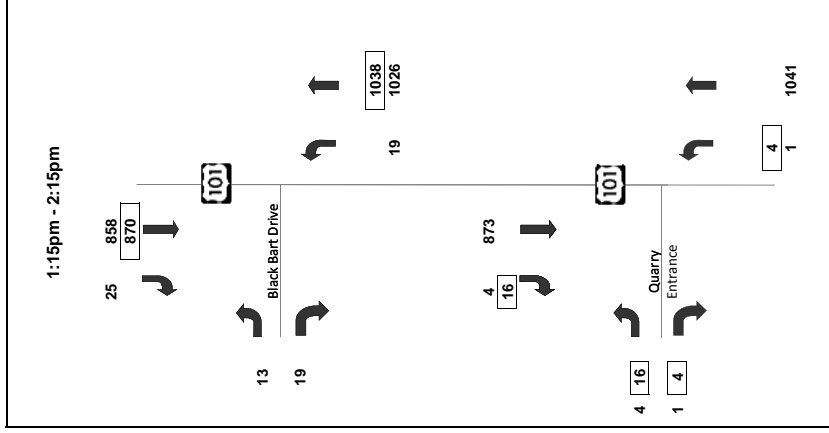
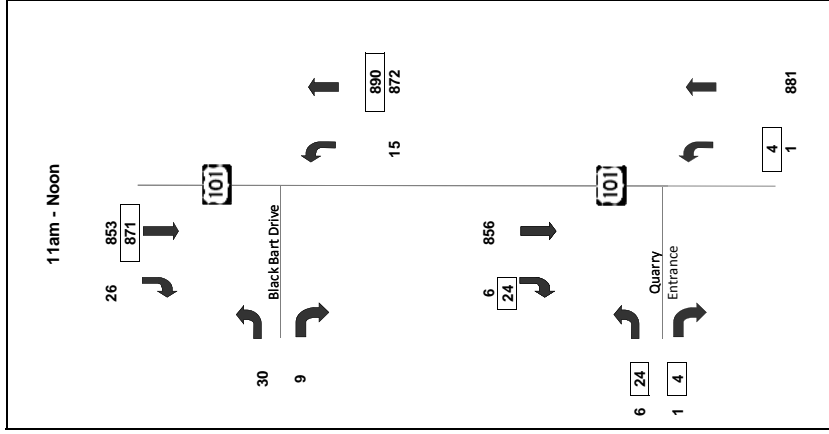
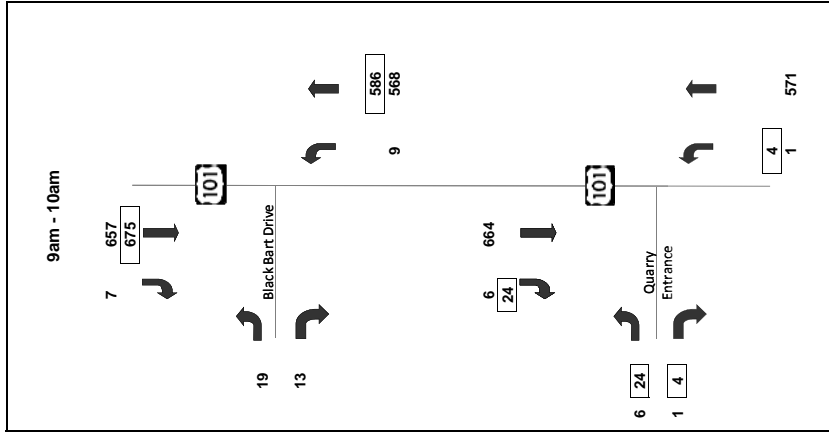
HARRIS QUARRY - TURN MOVEMENTS
2014 WITHOUT WILLITS BYPASS CONSTRUCTION
BASE CONDITIONS (75,000 CUBIC YARDS)
PRODUCTION:
JULY PEAK



Note:
 # = Trucks + Passenger Cars
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HARRIS QUARRY - TURN MOVEMENTS
2014 WITHOUT WILLITS BYPASS CONSTRUCTION
BASE CONDITIONS (75,000 CUBIC YARDS)
PRODUCTION:
OCTOBER PEAK

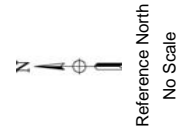
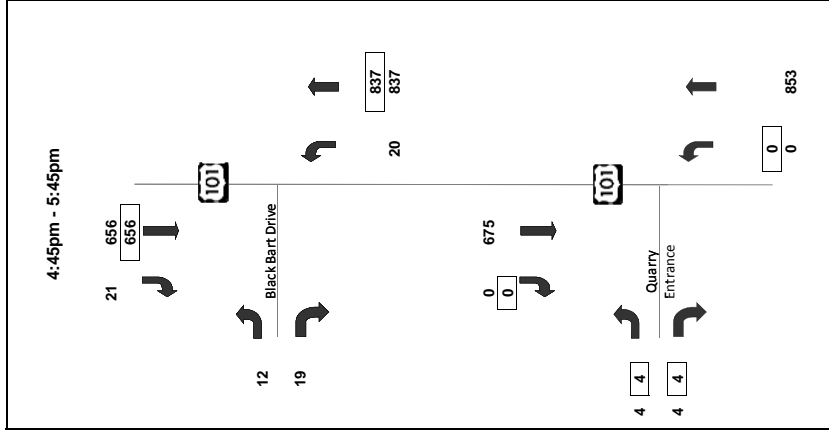
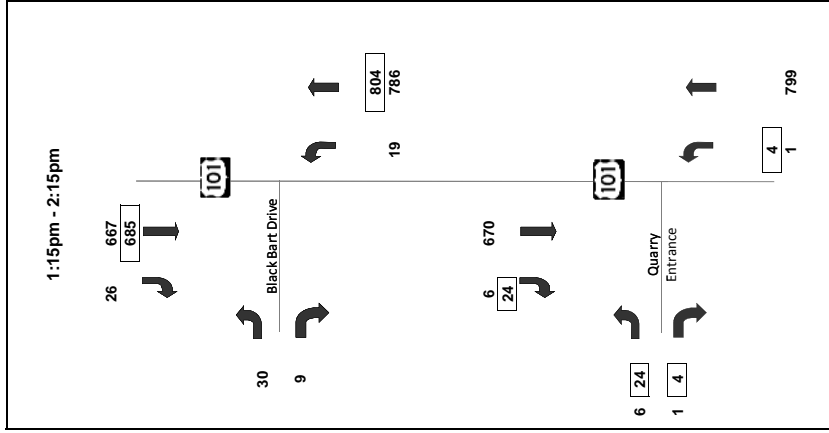
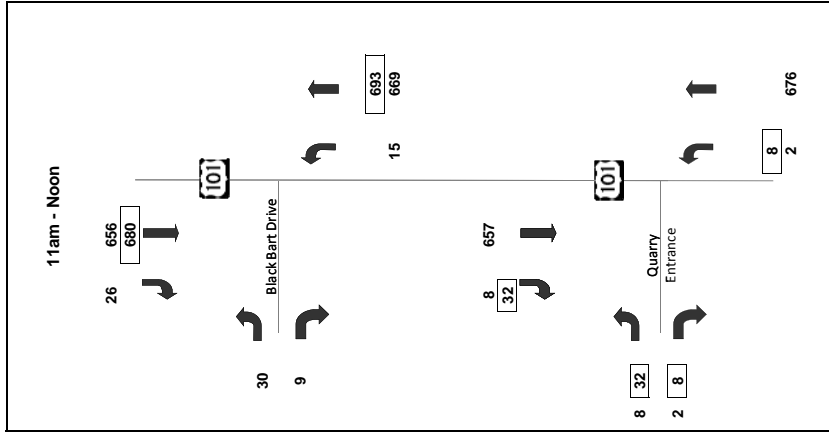
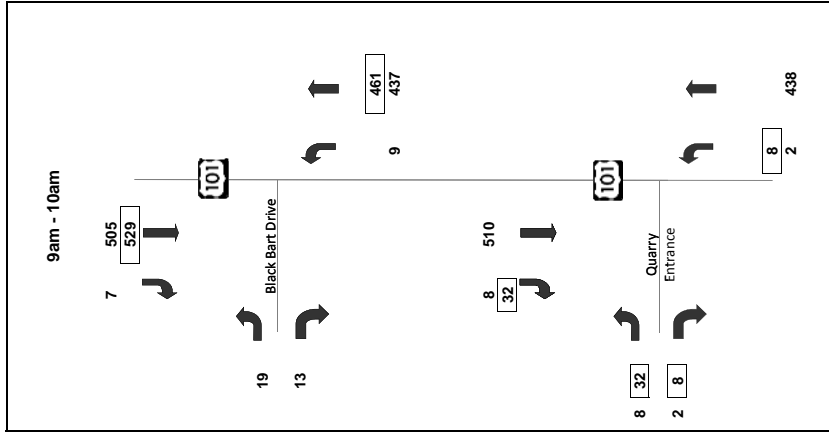


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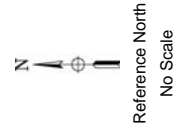
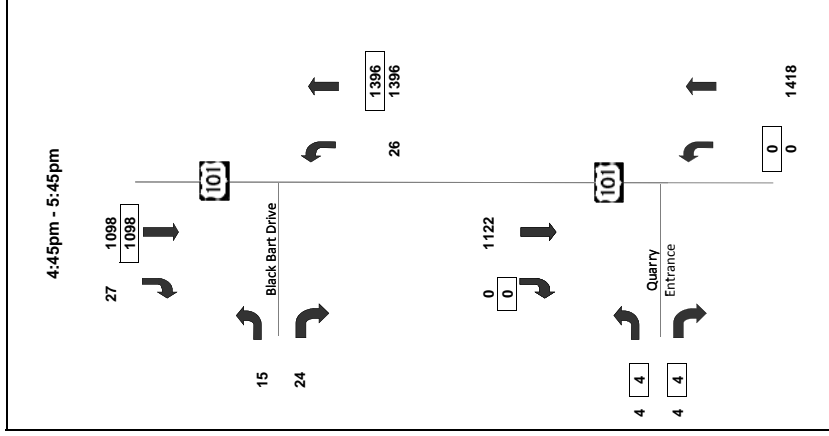
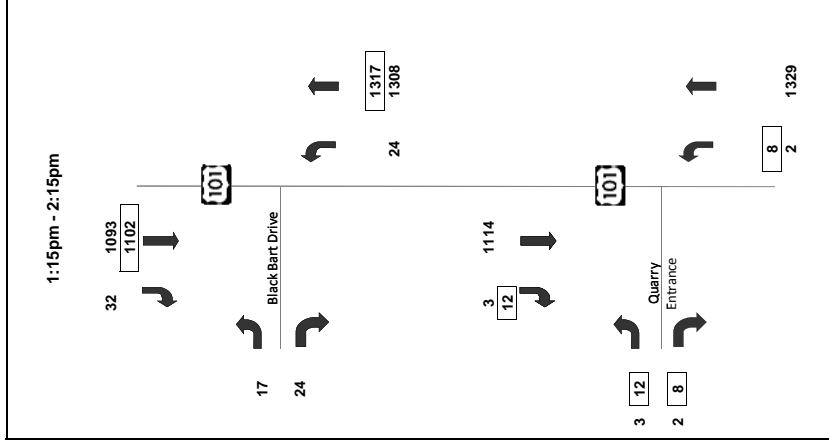
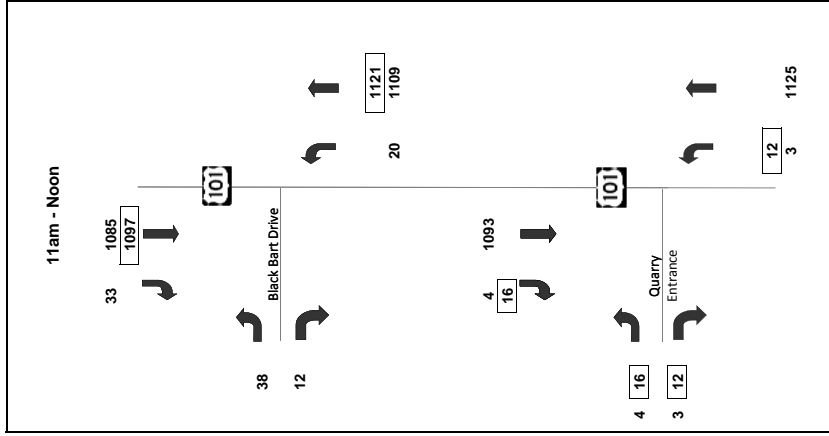
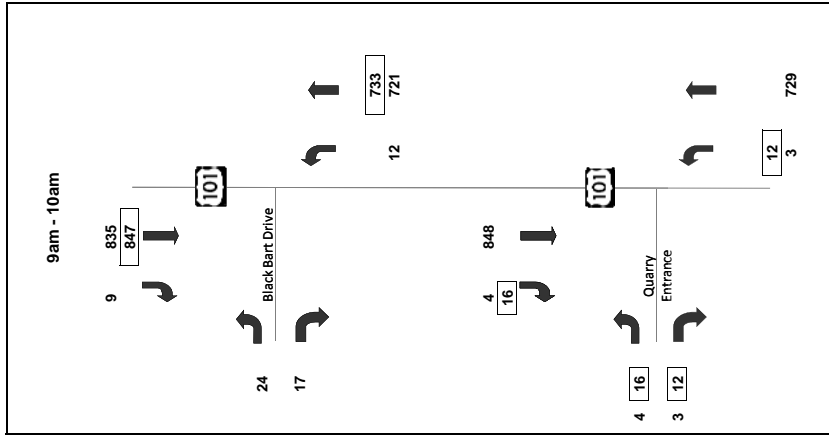
HARRIS QUARRY - TURN MOVEMENTS
2014 WITH WILLITS BYPASS CONSTRUCTION
BASE CONDITIONS (75,000 CUBIC YARDS)

PRODUCTION:
JULY PEAK



Note:
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 Where:
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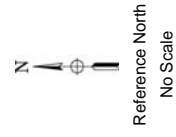
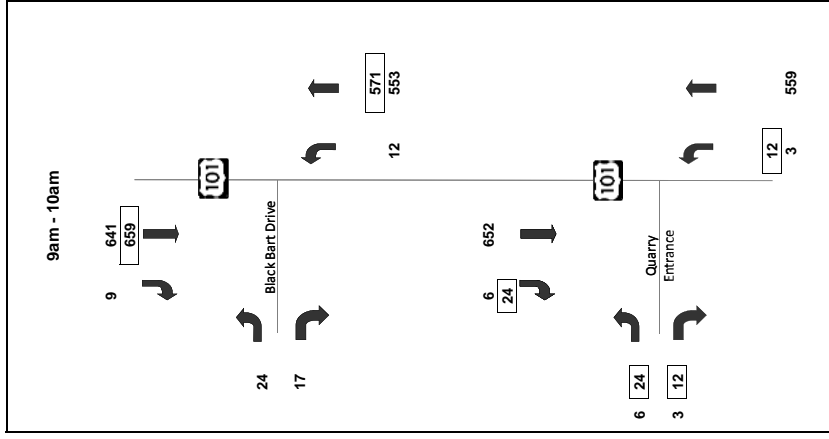
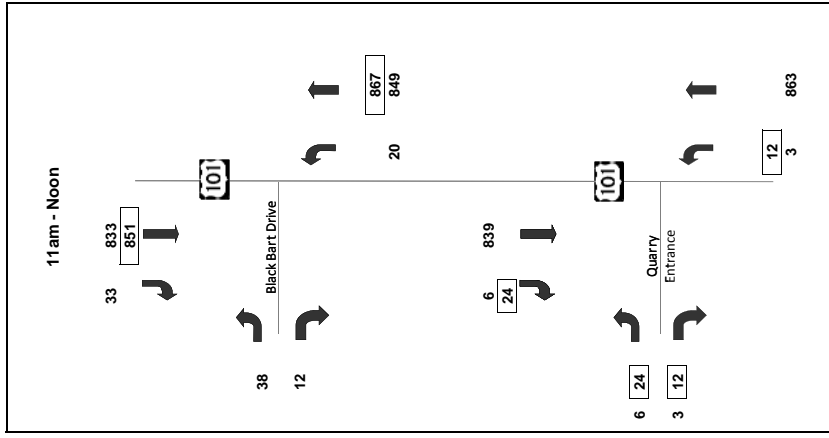
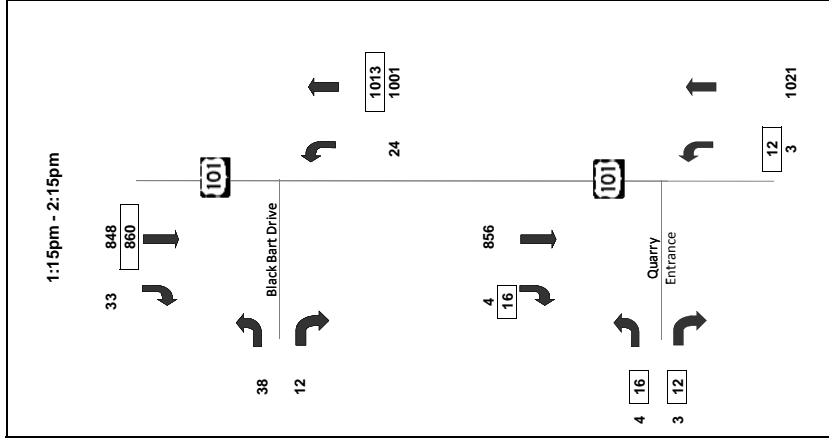
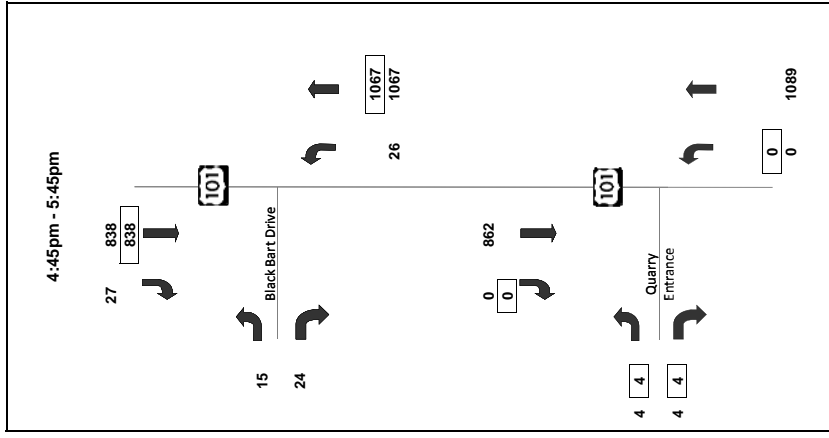
HARRIS QUARRY - TURN MOVEMENTS
2014 WITH WILLITS BYPASS CONSTRUCTION
BASE CONDITIONS (75,000 CUBIC YARDS)
PRODUCTION:
OCTOBER PEAK



Note:
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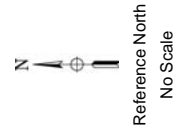
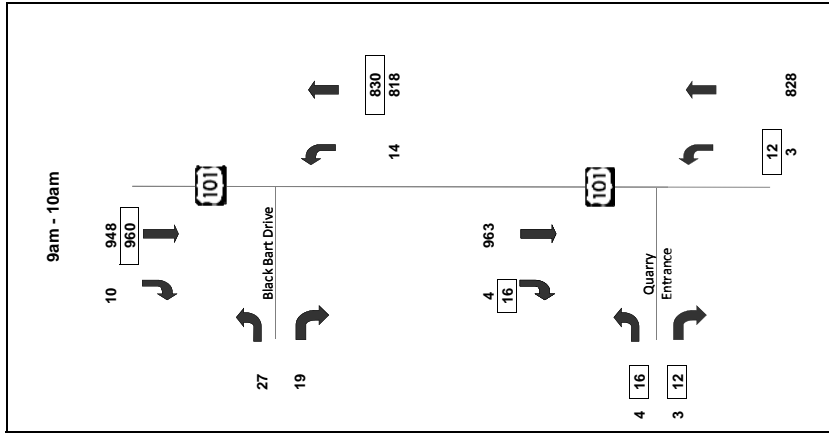
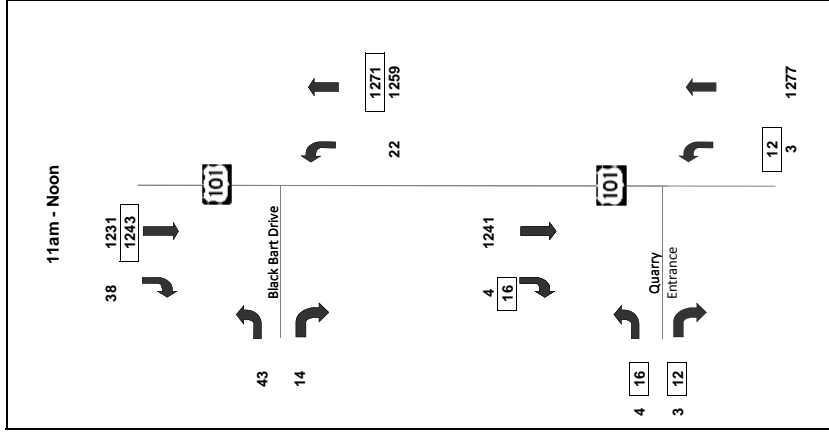
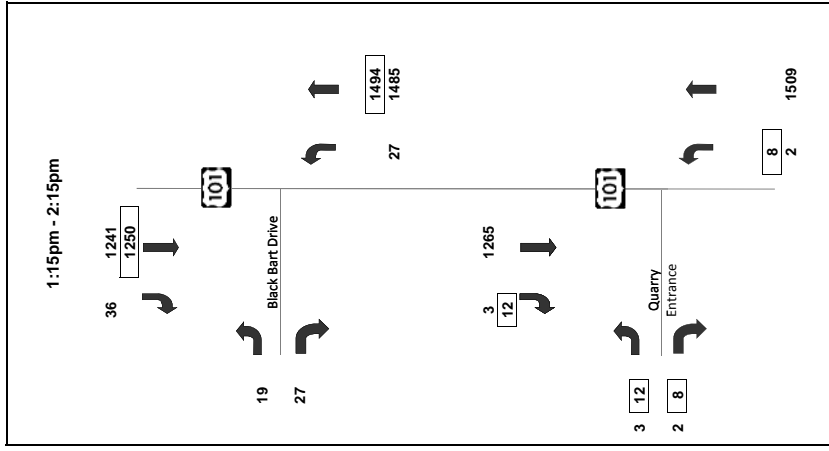
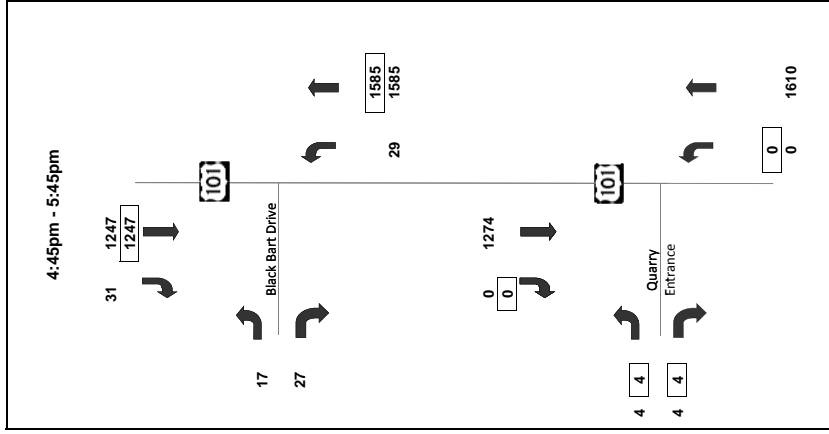
HARRIS QUARRY - TURN MOVEMENTS
2030
BASE CONDITIONS (75,000 CUBIC YARDS)
PRODUCTION:
JULY PEAK



Note:
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 # = Passenger Car Equivalent

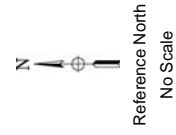
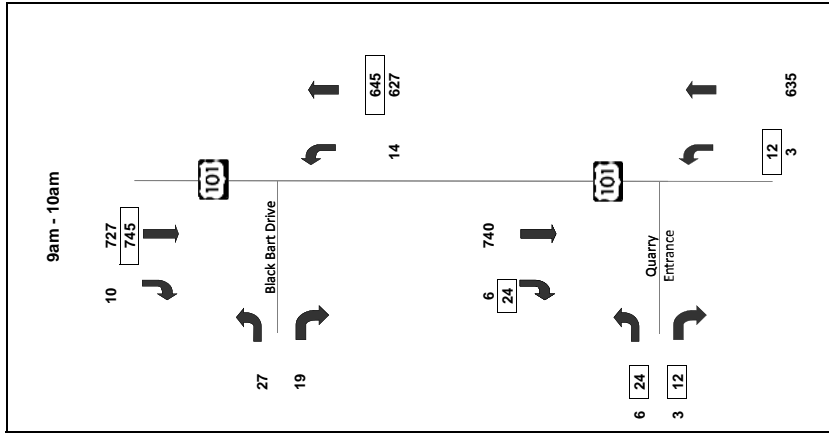
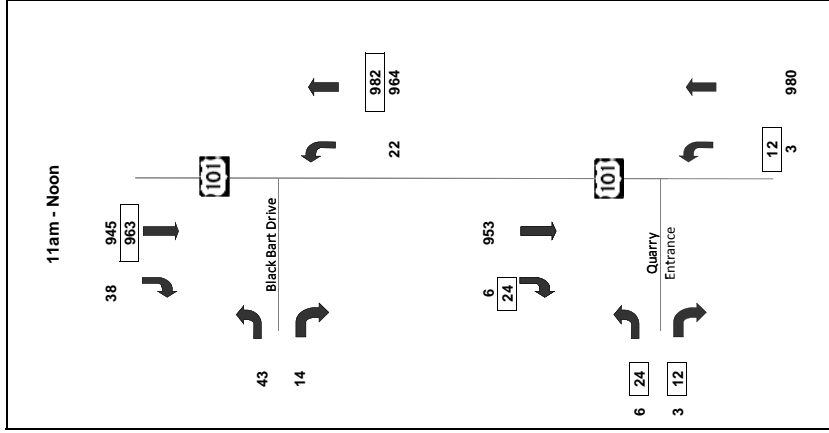
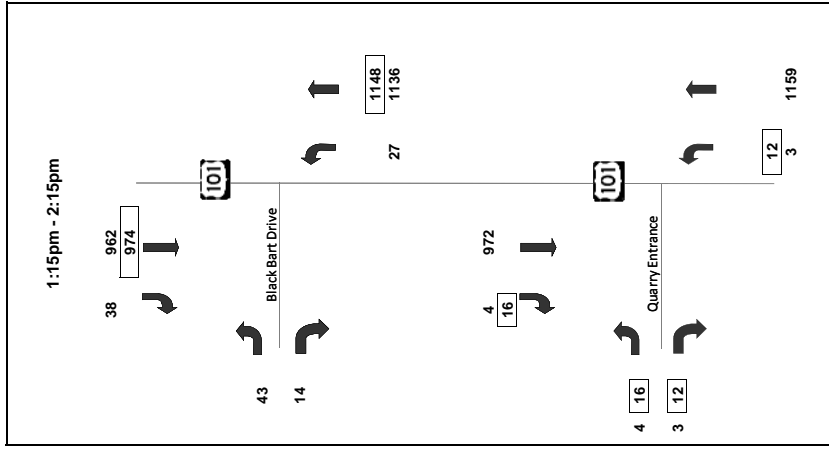
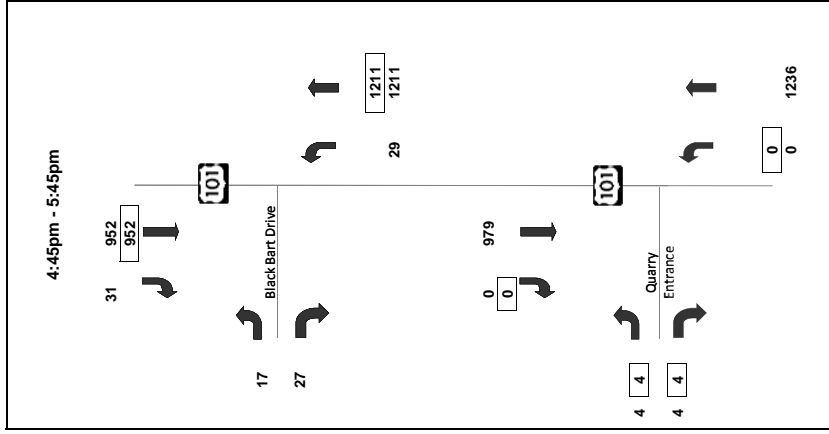
Where:
 1 Truck = 4 Passenger Car Equivalents

HARRIS QUARRY - TURN MOVEMENTS
2030
BASE CONDITIONS (75,000 CUBIC YARDS)
PRODUCTION:
OCTOBER PEAK



HARRIS QUARRY - TURN MOVEMENTS
2040
BASE CONDITIONS (75,000 CUBIC YARDS)
PRODUCTION:
JULY PEAK

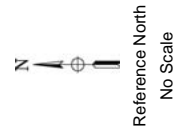
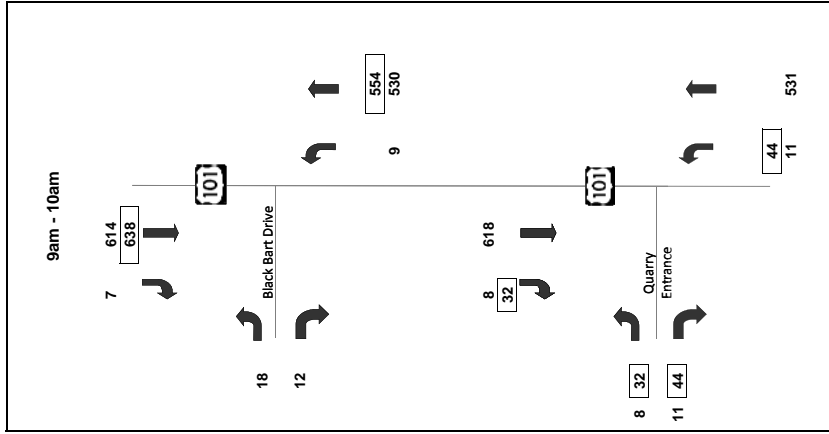
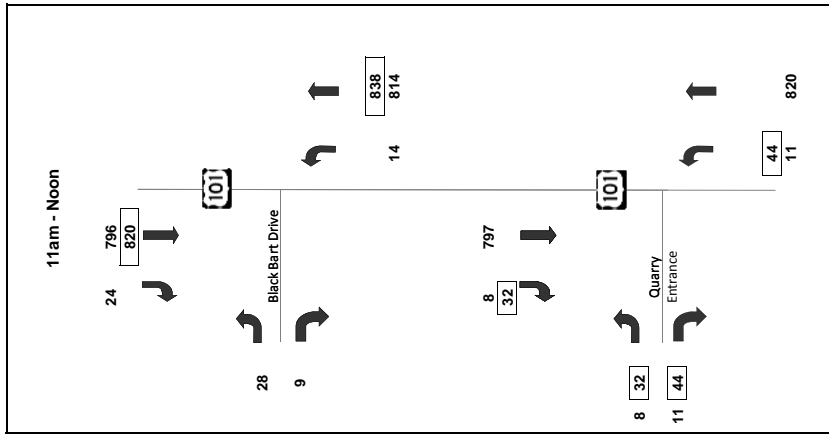
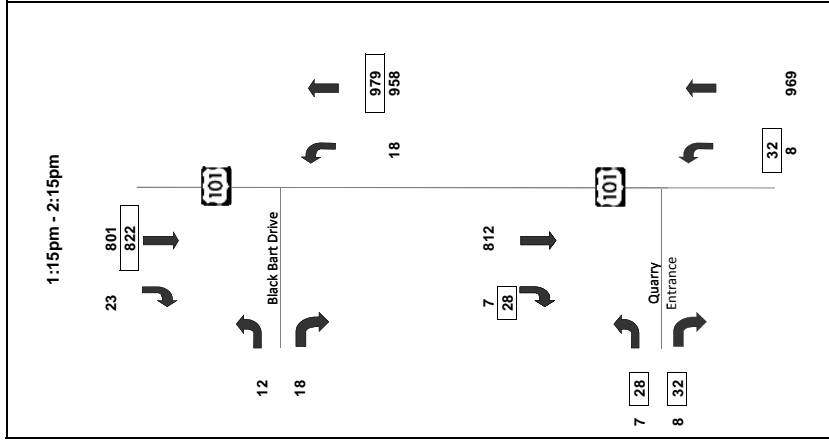
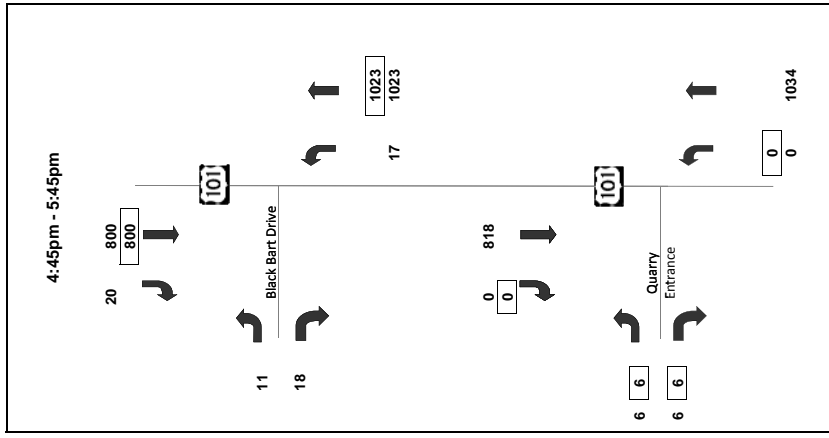
Note:
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Note:
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Where:
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HARRIS QUARRY - TURN MOVEMENTS
2040
BASE CONDITIONS (75,000 CUBIC YARDS)
PRODUCTION:
OCTOBER PEAK

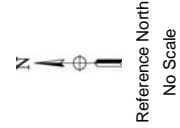
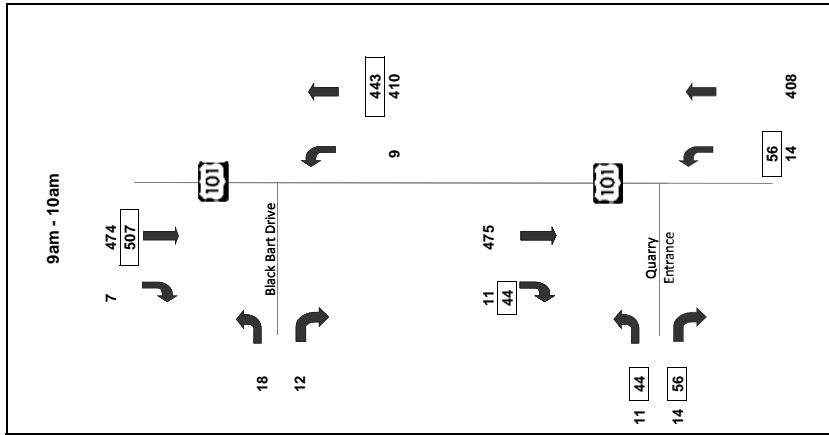
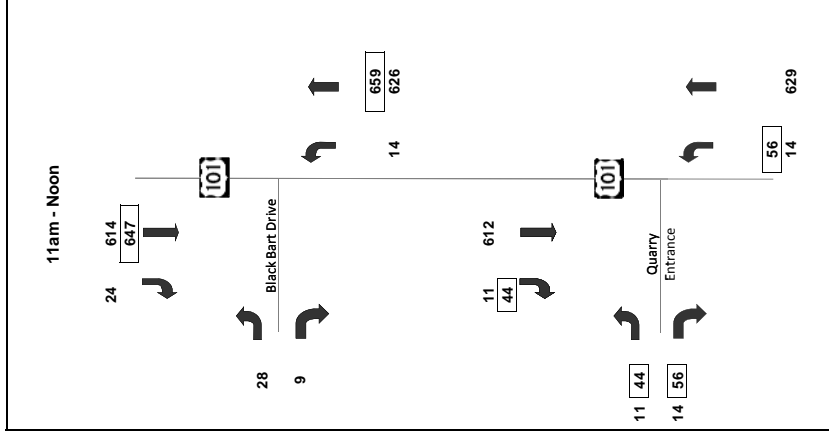
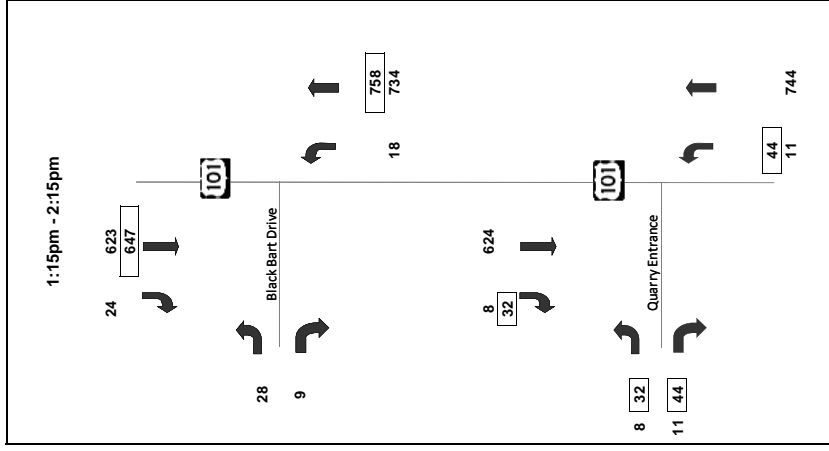
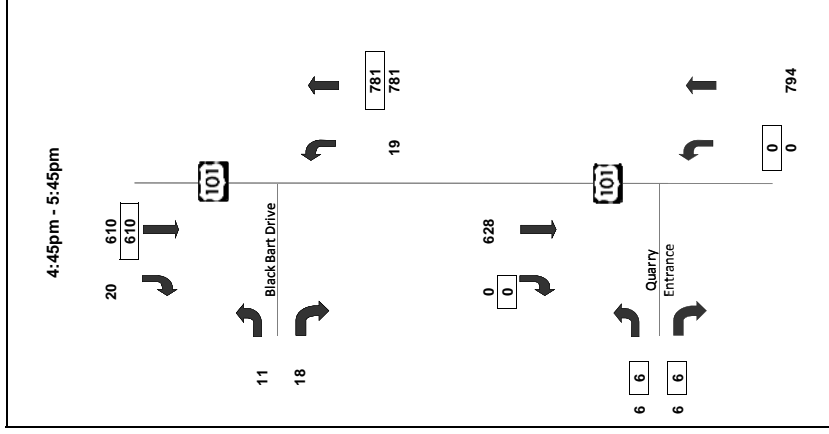


HARRIS QUARRY - TURN MOVEMENTS

2010
PROJECT CONDITIONS (200,000 CUBIC YARDS)
PRODUCTION:
JULY PEAK

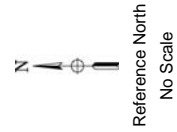
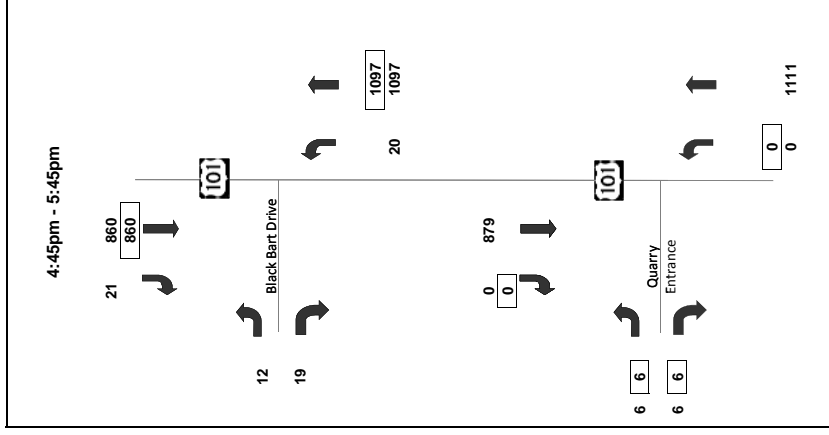
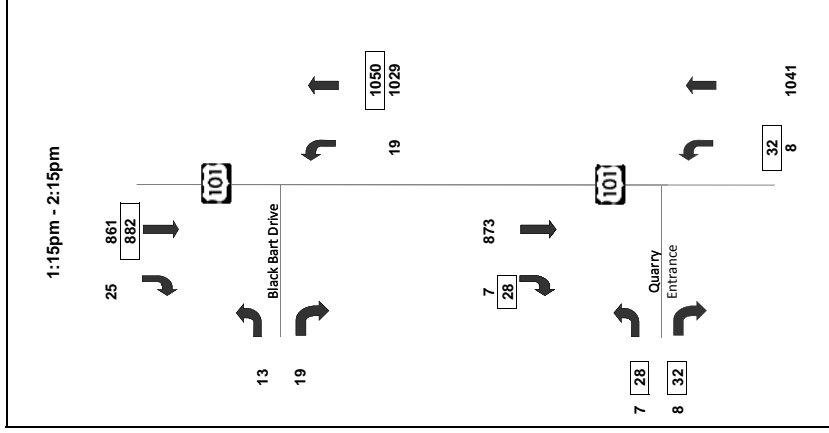
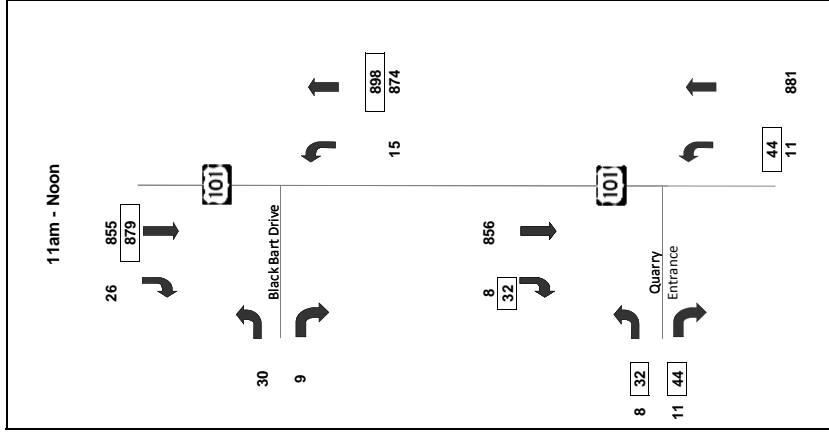
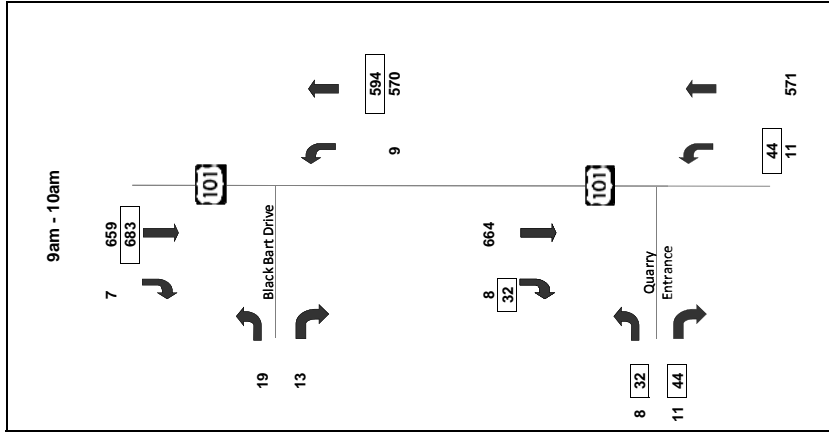
Note:
= Trucks + Passenger Cars
= Passenger Car Equivalent

Where:
1 Truck = 4 Passenger Car Equivalents



HARRIS QUARRY - TURN MOVEMENTS
2010
PROJECT CONDITIONS (200,000 CUBIC YARDS)
PRODUCTION:
OCTOBER PEAK

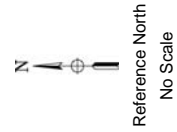
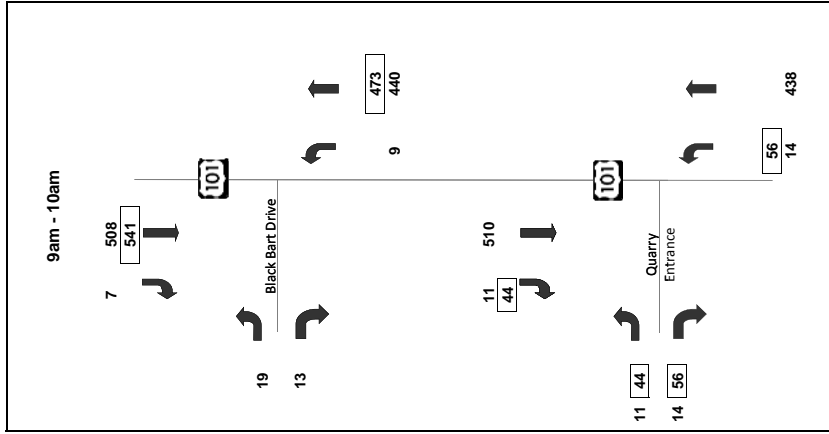
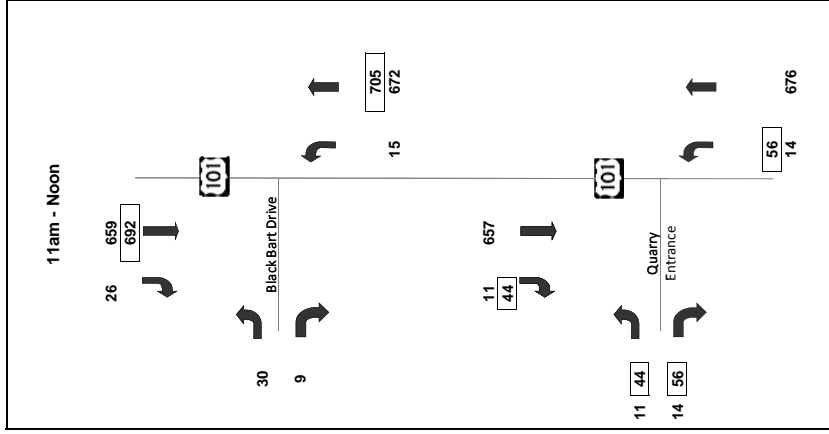
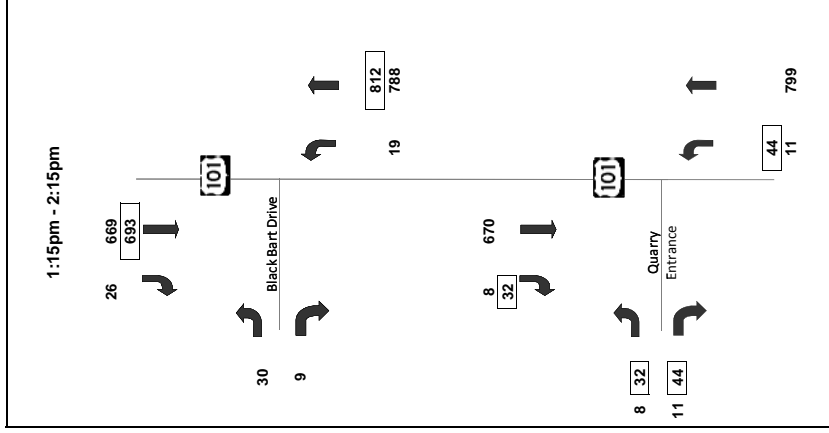
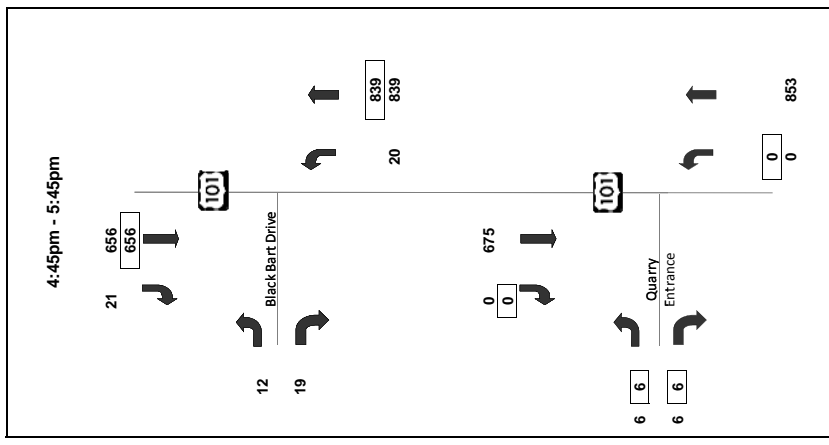
Note:
 # = Trucks + Passenger Cars
 # = Passenger Car Equivalent
 Where:
 1 Truck = 4 Passenger Car Equivalents



Note:
 # = Trucks + Passenger Cars
 # = Passenger Car Equivalent

Where:
 1 Truck = 4 Passenger Car Equivalents

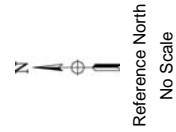
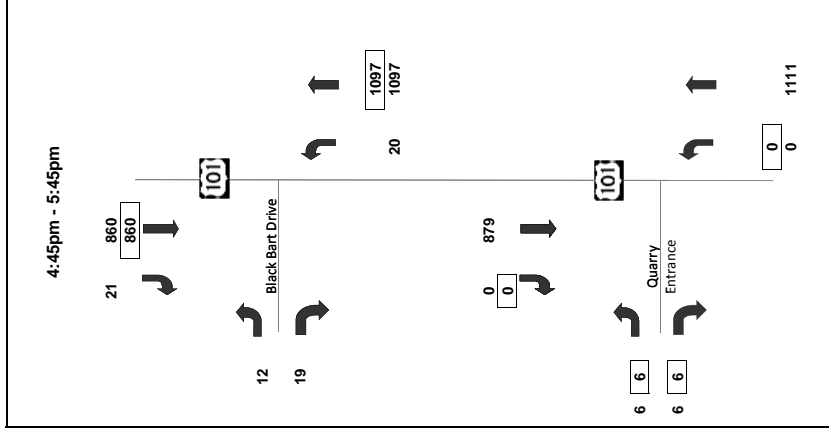
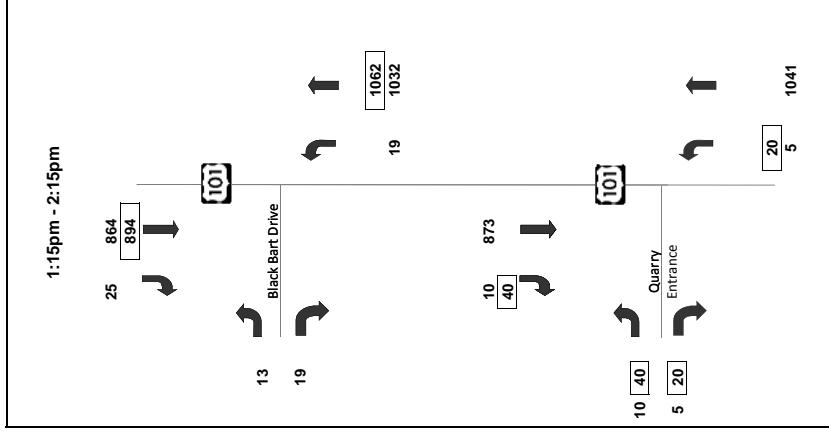
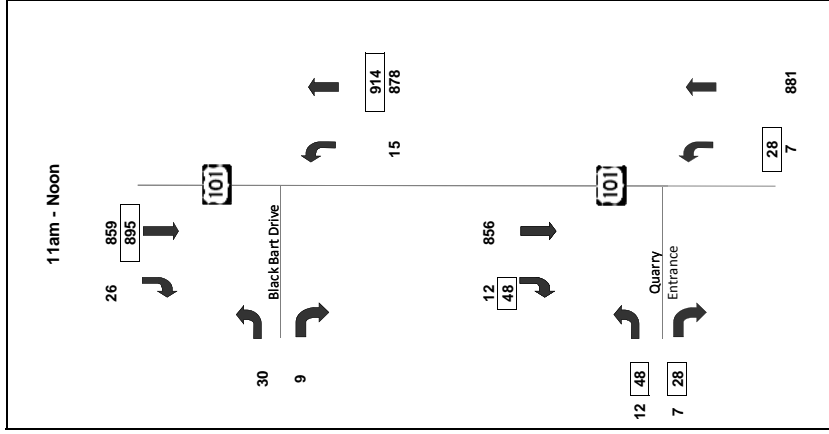
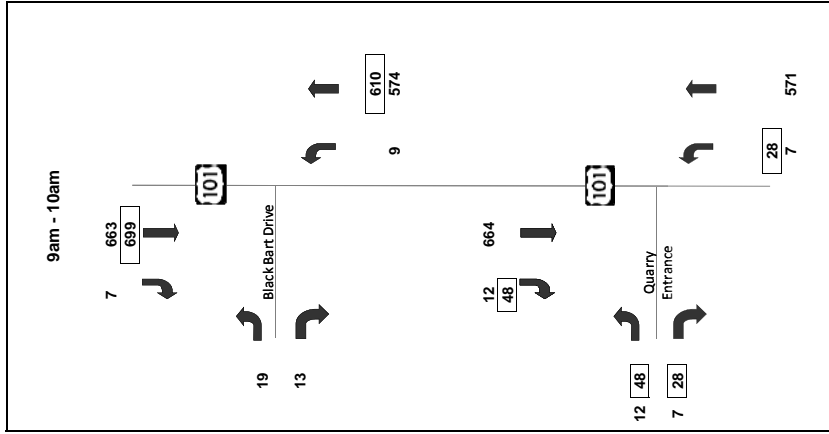
HARRIS QUARRY - TURN MOVEMENTS
2014 WITHOUT WILLITS BYPASS CONSTRUCTION
PROJECT CONDITIONS (200,000 CUBIC YARDS)
PRODUCTION:
JULY PEAK



Note:
 # = Trucks + Passenger Cars
 # = Passenger Car Equivalent

Where:
 1 Truck = 4 Passenger Car Equivalents

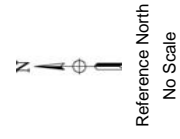
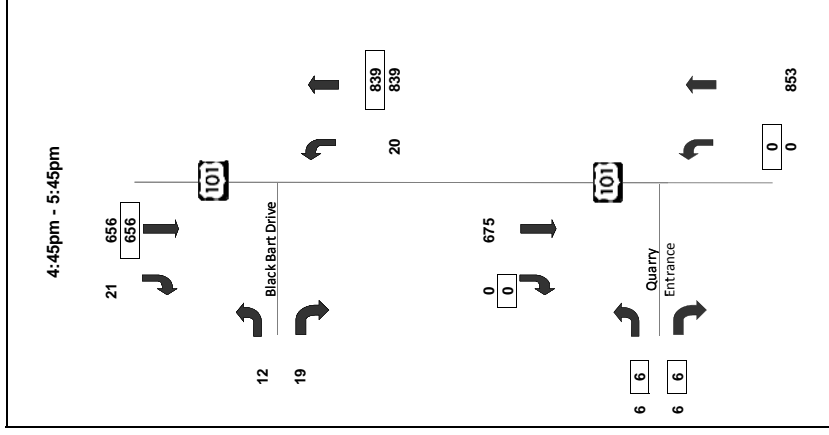
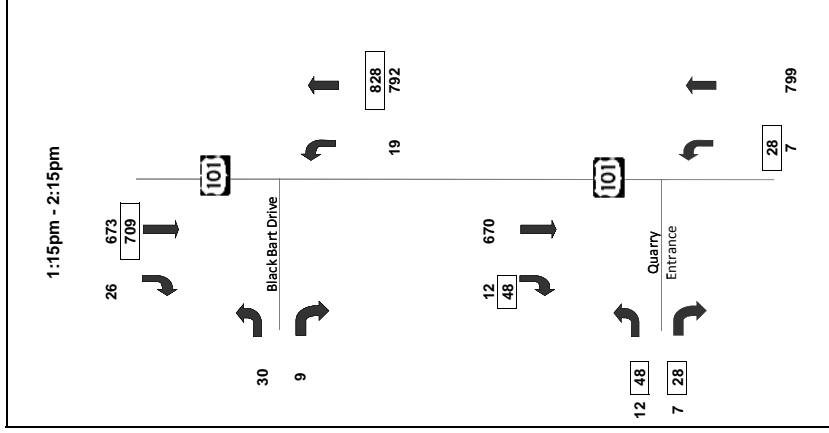
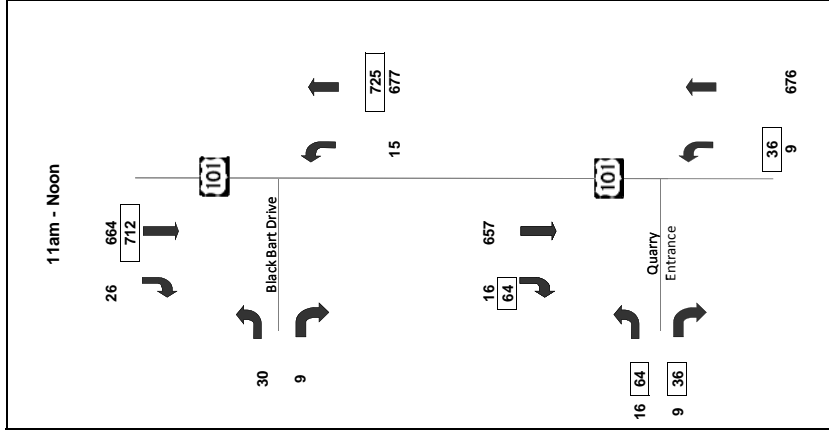
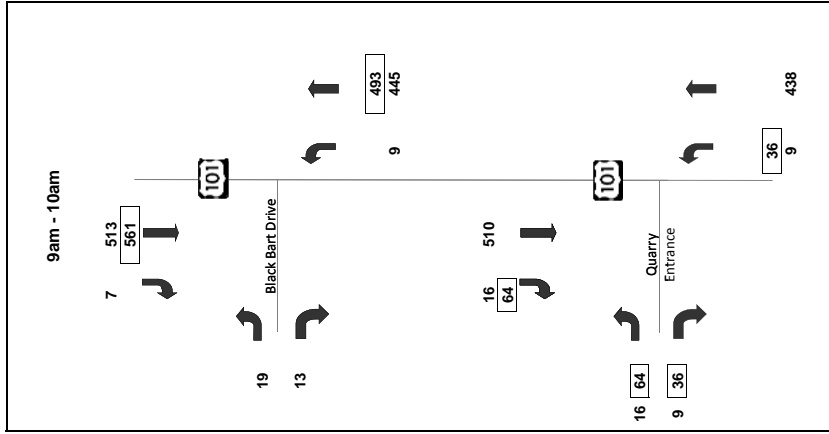
HARRIS QUARRY - TURN MOVEMENTS
2014 WITHOUT WILLITS BYPASS CONSTRUCTION
PROJECT CONDITIONS (200,000 CUBIC YARDS)
PRODUCTION:
OCTOBER PEAK



Note:
 # = Trucks + Passenger Cars
 # = Passenger Car Equivalent

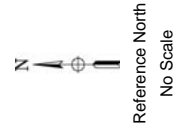
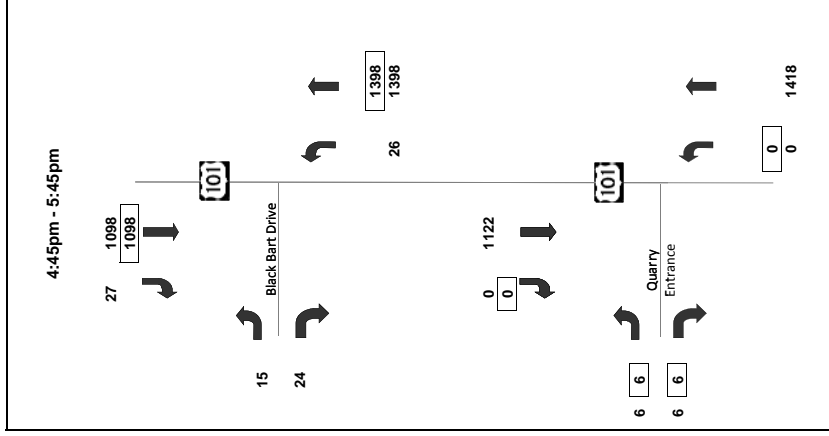
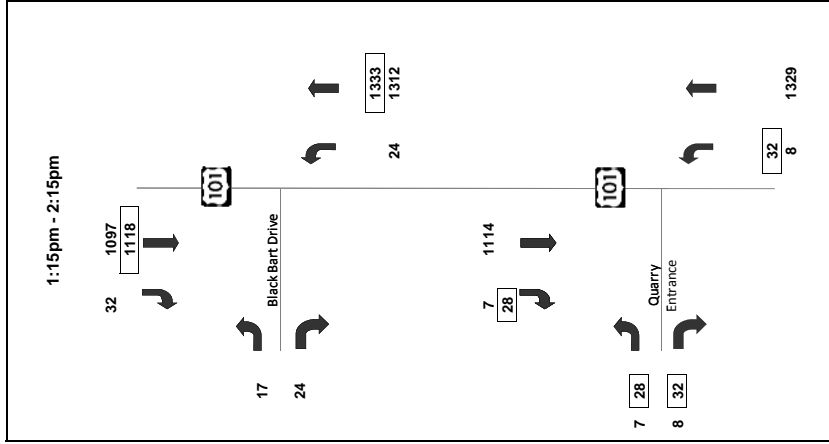
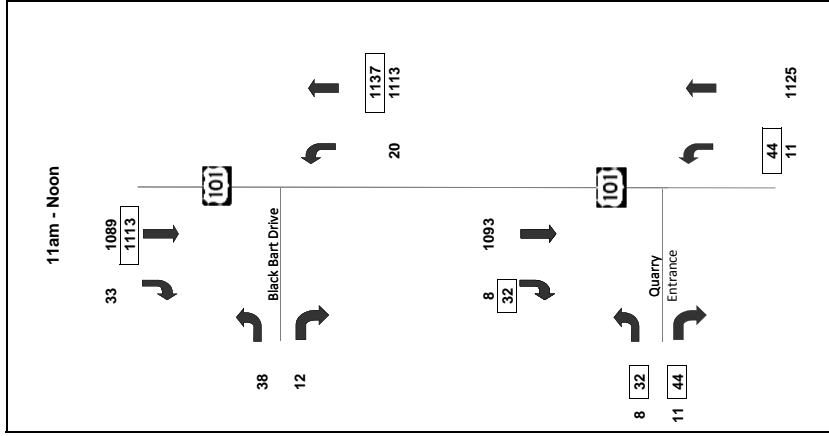
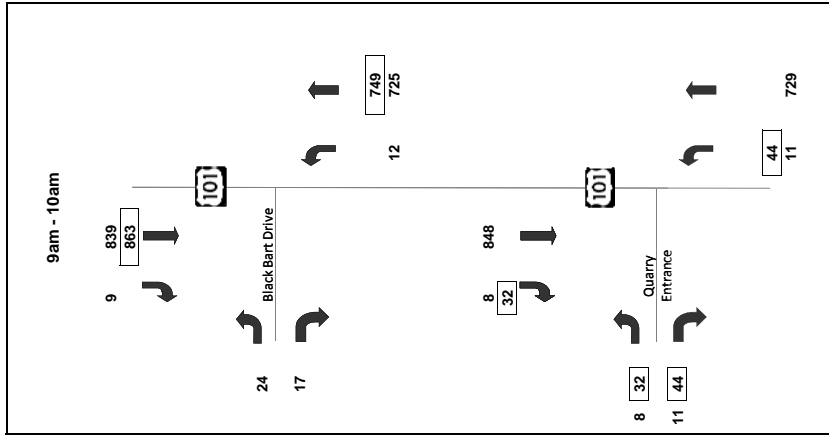
Where:
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HARRIS QUARRY - TURN MOVEMENTS
2014 WITH WILLITS BYPASS CONSTRUCTION
PROJECT CONDITIONS (200,000 CUBIC YARDS)
PRODUCTION:
JULY PEAK



Note:
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 Where:
 1 Truck = 4 Passenger Car Equivalents

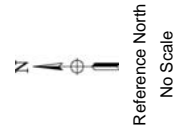
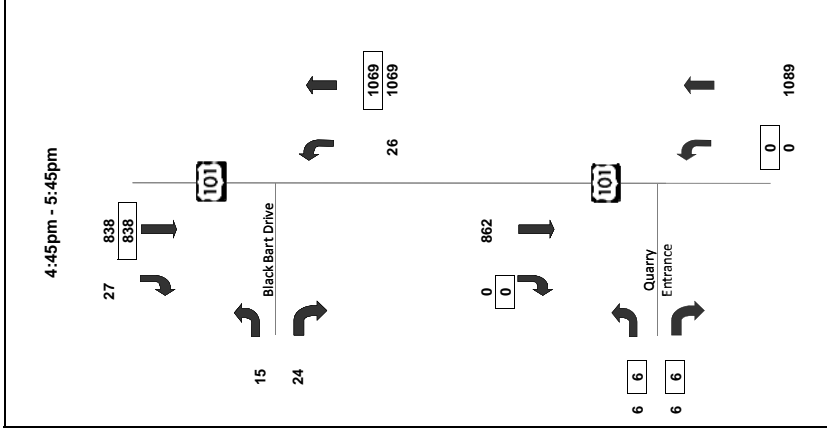
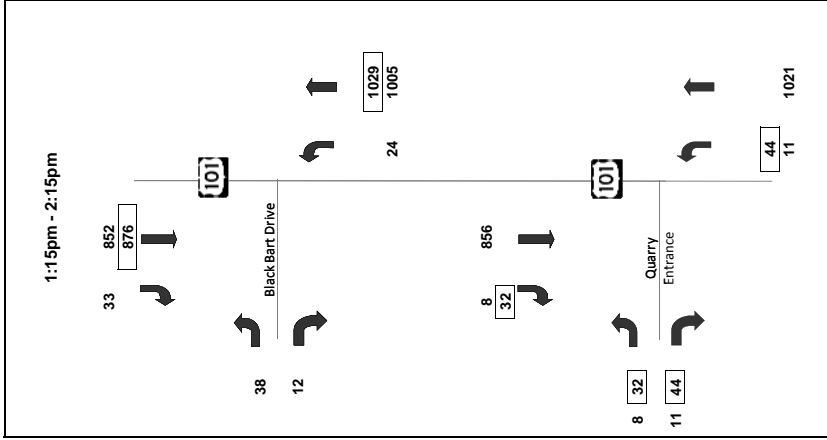
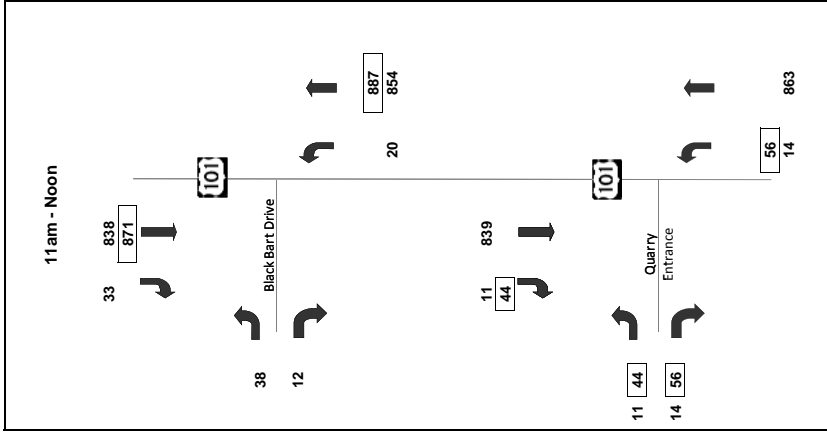
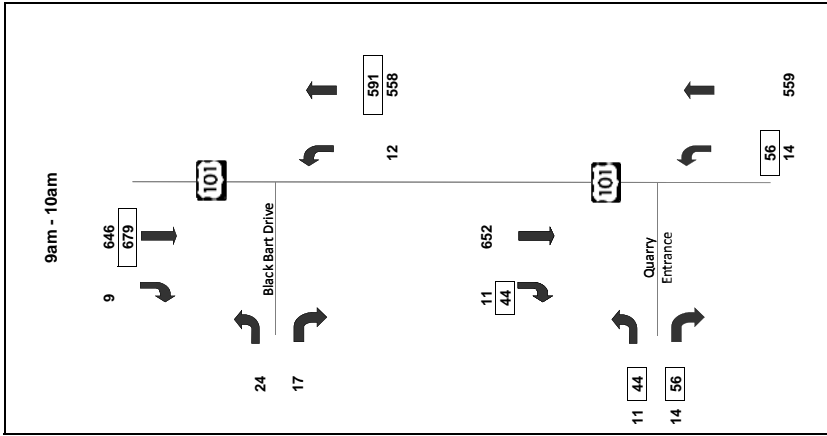
HARRIS QUARRY - TURN MOVEMENTS
2014 WITH WILLITS BYPASS CONSTRUCTION
PROJECT CONDITIONS (200,000 CUBIC YARDS)
PRODUCTION:
OCTOBER PEAK



HARRIS QUARRY - TURN MOVEMENTS

2030
PROJECT CONDITIONS (200,000 CUBIC YARDS)
PRODUCTION:
JULY PEAK

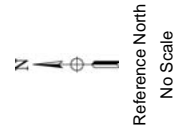
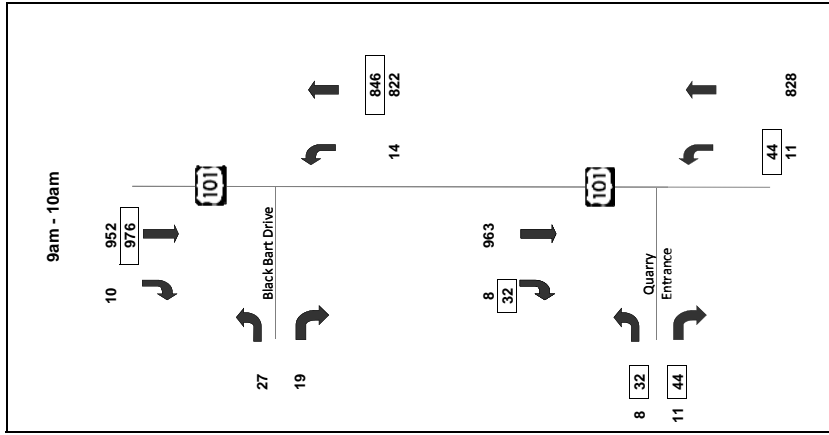
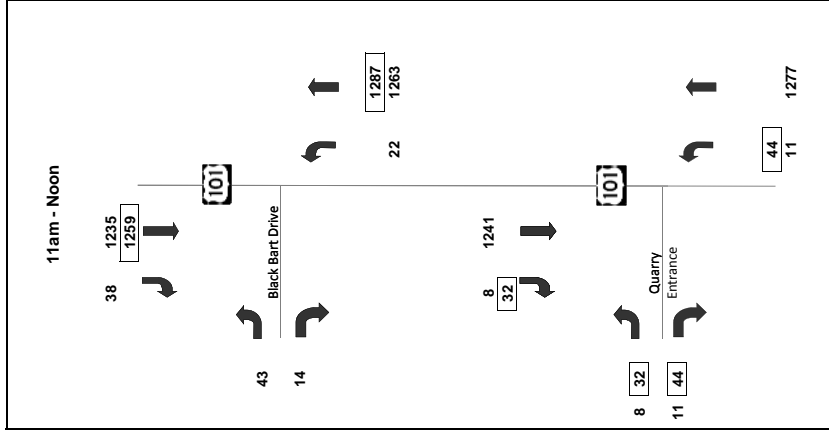
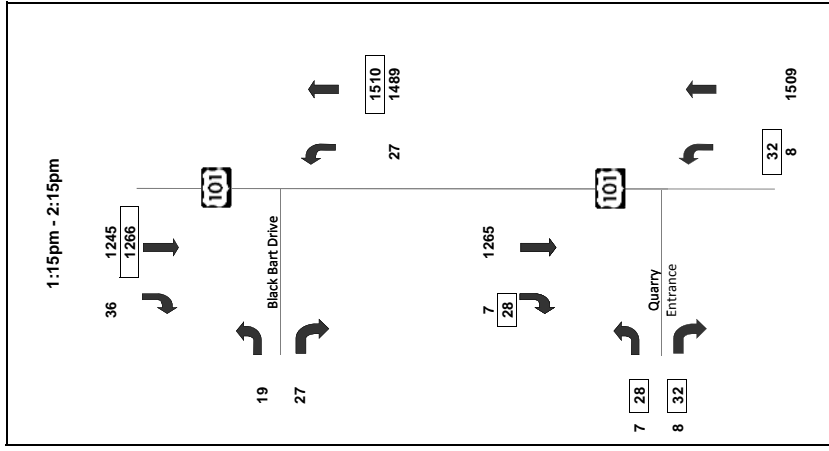
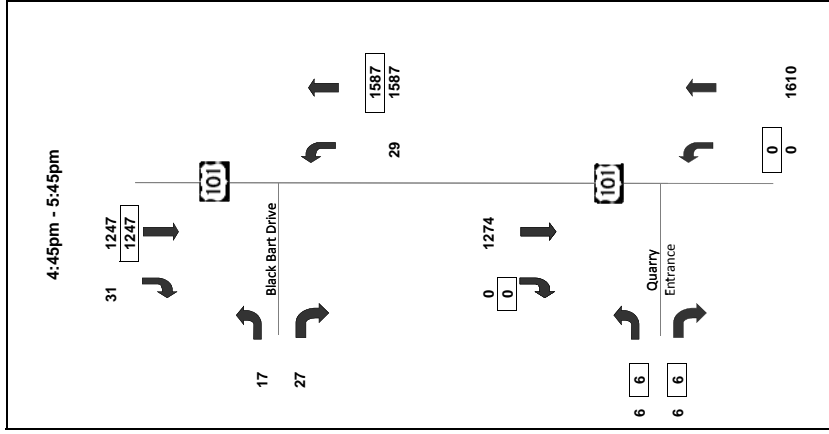
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 # = Trucks + Passenger Cars
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Where:
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HARRIS QUARRY - TURN MOVEMENTS
2030
PROJECT CONDITIONS (200,000 CUBIC YARDS)
PRODUCTION:
OCTOBER PEAK



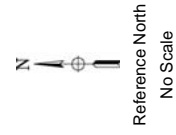
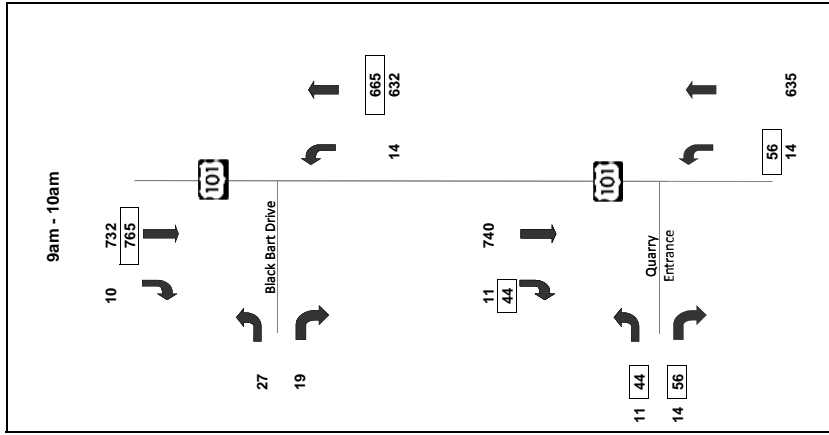
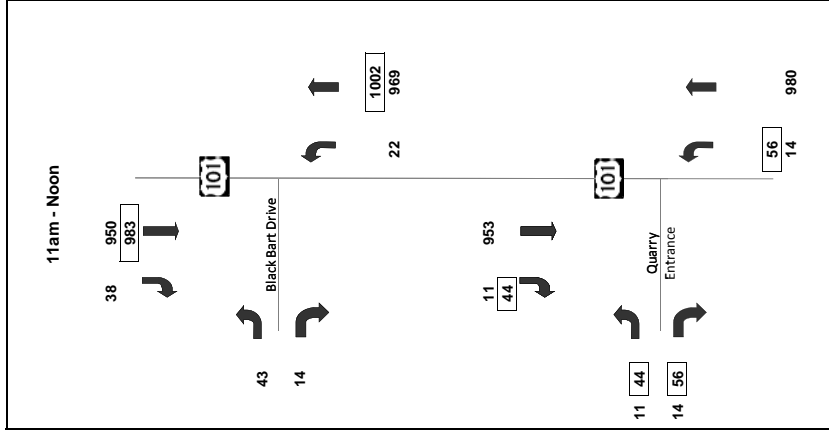
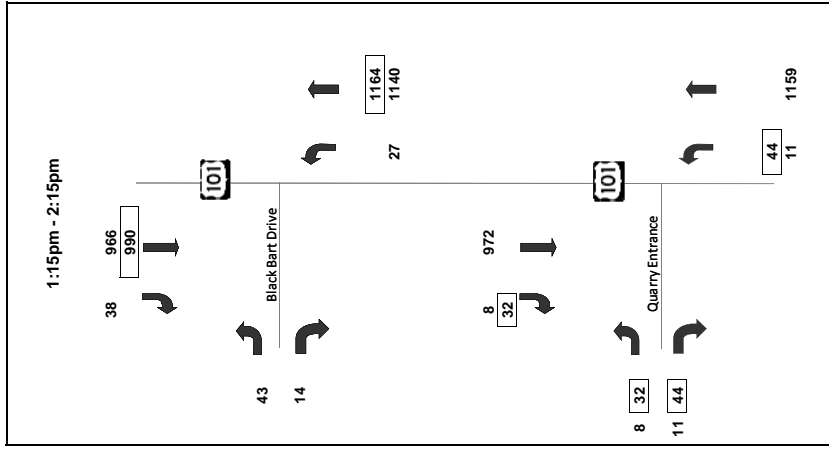
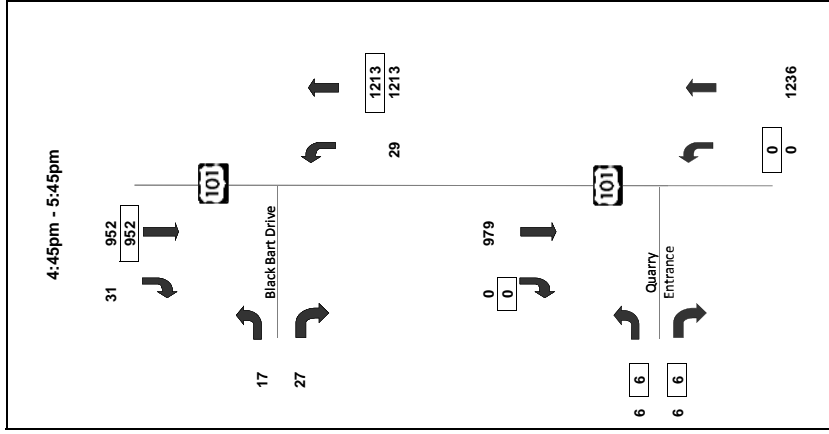
HARRIS QUARRY - TURN MOVEMENTS

2040 PROJECT CONDITIONS (200,000 CUBIC YARDS)

**PRODUCTION:
JULY PEAK**

Note:
= Trucks + Passenger Cars
= Passenger Car Equivalent

Where:
1 Truck = 4 Passenger Car Equivalents



HARRIS QUARRY - TURN MOVEMENTS

2040
PROJECT CONDITIONS (200,000 CUBIC YARDS)

PRODUCTION:
OCTOBER PEAK

Note:
= Trucks + Passenger Cars
= Passenger Car Equivalent

Where:
1 Truck = 4 Passenger Car Equivalents

Appendix E

July and October Turn Movements

HARRIS QUARRY DRIVEWAY - TURN MOVEMENTS

2010

BASE CONDITIONS (75,000 CUBIC YARDS)

July - Peak Production Days

	9:00am - 10:00am		11:00am - Noon		1:15pm - 2:15pm		4:45pm - 5:45pm	
	Vehicle	Truck	Vehicle	Truck	Vehicle	Truck	Vehicle	Truck
NB Left	0	3	0	3	0	2	0	0
NB Through	531	0	820	0	969	0	1034	0
SB Through	618	0	797	0	812	0	818	0
SB Right	0	4	0	4	0	3	0	0
EB Left	0	4	0	4	0	3	4	0
EB Right	0	3	0	3	0	2	4	0
			PCE		PCE		PCE	
			12		8		8	
Harris Access In	0	7	0	7	0	5	0	0
Harris Access Out	0	7	0	7	0	5	8	0
			28		20		20	
			28		20		20	

October - Peak Production Days

	9:00am - 10:00am		11:00am - Noon		1:15pm - 2:15pm		4:45pm - 5:45pm	
	Vehicle	Truck	Vehicle	Truck	Vehicle	Truck	Vehicle	Truck
NB Left	0	3	0	3	0	3	0	0
NB Through	408	0	629	0	744	0	794	0
SB Through	475	0	612	0	624	0	628	0
SB Right	0	6	0	6	0	4	0	0
EB Left	0	6	0	6	0	4	4	0
EB Right	0	3	0	3	0	3	4	0
			PCE		PCE		PCE	
			12		12		12	
Harris Access In	0	9	0	9	0	7	0	0
Harris Access Out	0	9	0	9	0	7	8	0
			36		28		28	
			36		28		28	

PCE = Passenger Care Equivalent
Where: 1 Truck = 4 Passenger Car Equivalents

*See "Turn Movement Key" for the Movement Letter Designations.

BLACK BART DRIVE - TURN MOVEMENTS

2010

BASE CONDITIONS (75,000 CUBIC YARDS)

July - Peak Production Days

	9:00am - 10:00am	11:00am - Noon	1:15pm - 2:15pm	4:45pm - 5:45pm
	Vehicle & Truck	Vehicle & Truck	Vehicle & Truck	Vehicle & Truck
	PCE	PCE	PCE	PCE
NB Left	G 9	14	18	19
NB Through	H 526	810	954	1019
SB Through	I 610	792	797	800
SB Right	J 7	24	23	20
EB Left	K 18	28	12	11
EB Right	L 12	9	18	18

October - Peak Production Days

	9:00am - 10:00am	11:00am - Noon	1:15pm - 2:15pm	4:45pm - 5:45pm
	Vehicle & Truck	Vehicle & Truck	Vehicle & Truck	Vehicle & Truck
	PCE	PCE	PCE	PCE
NB Left	G 9	14	18	19
NB Through	H 405	621	730	779
SB Through	I 469	609	619	610
SB Right	J 7	24	24	20
EB Left	K 18	28	28	11
EB Right	L 12	9	9	18

PCE = Passenger Care Equivalent
Where: 1 Truck = 4 Passenger Car Equivalents

*See "Turn Movement Key" for the Movement Letter Designations.

HARRIS QUARRY DRIVEWAY - TURN MOVEMENTS

2014 WITHOUT WILLITS BYPASS CONSTRUCTION BASE CONDITIONS (75,000 CUBIC YARDS)

July - Peak Production Days

	9:00am - 10:00am			11:00am - Noon			1:15pm - 2:15pm			4:45pm - 5:45pm		
	Vehicle	Truck	PCE	Vehicle	Truck	PCE	Vehicle	Truck	PCE	Vehicle	Truck	PCE
NB Left	0	3	12	0	3	12	0	2	8	0	0	0
NB Through	571	0	571	881	0	881	1041	0	1041	1111	0	1111
SB Through	664	0	664	856	0	856	873	0	873	879	0	879
SB Right	0	4	16	0	4	16	0	3	12	0	0	0
EB Left	0	4	16	0	4	16	0	3	12	4	0	4
EB Right	0	3	12	0	3	12	0	2	8	4	0	4
Harris Access In	0	7	28	0	7	28	0	5	20	0	0	0
Harris Access Out	0	7	28	0	7	28	0	5	20	8	0	8

October - Peak Production Days

	9:00am - 10:00am			11:00am - Noon			1:15pm - 2:15pm			4:45pm - 5:45pm		
	Vehicle	Truck	PCE	Vehicle	Truck	PCE	Vehicle	Truck	PCE	Vehicle	Truck	PCE
NB Left	0	3	12	0	3	12	0	3	12	0	0	0
NB Through	438	0	438	676	0	676	799	0	799	853	0	853
SB Through	510	0	510	657	0	657	670	0	670	675	0	675
SB Right	0	6	24	0	6	24	0	4	16	0	0	0
EB Left	0	6	24	0	6	24	0	4	16	4	0	4
EB Right	0	3	12	0	3	12	0	3	12	4	0	4
Harris Access In	0	9	36	0	9	36	0	7	28	0	0	0
Harris Access Out	0	9	36	0	9	36	0	7	28	8	0	8

PCE = Passenger Care Equivalent
Where: 1 Truck = 4 Passenger Car Equivalents

*See "Turn Movement Key" for the Movement Letter Designations.

BLACK BART DRIVE - TURN MOVEMENTS

2014 WITHOUT WILLITS BYPASS CONSTRUCTION BASE CONDITIONS (75,000 CUBIC YARDS)

July - Peak Production Days

	9:00am - 10:00am	11:00am - Noon	1:15pm - 2:15pm	4:45pm - 5:45pm
	Vehicle & Truck	Vehicle & Truck	Vehicle & Truck	Vehicle & Truck
	PCE	PCE	PCE	PCE
NB Left	G 9	15	19	20
NB Through	H 566	870	1025	1095
SB Through	I 655	851	857	860
SB Right	J 7	26	25	21
EB Left	K 19	30	13	12
EB Right	L 13	9	19	19

October - Peak Production Days

	9:00am - 10:00am	11:00am - Noon	1:15pm - 2:15pm	4:45pm - 5:45pm
	Vehicle & Truck	Vehicle & Truck	Vehicle & Truck	Vehicle & Truck
	PCE	PCE	PCE	PCE
NB Left	G 9	15	19	20
NB Through	H 435	667	784	837
SB Through	I 503	654	665	656
SB Right	J 7	26	26	21
EB Left	K 19	30	30	12
EB Right	L 13	9	9	19

PCE = Passenger Care Equivalent
Where: 1 Truck = 4 Passenger Car Equivalents

*See "Turn Movement Key" for the Movement Letter Designations.

HARRIS QUARRY DRIVEWAY - TURN MOVEMENTS

2014 WITH WILLITS BYPASS CONSTRUCTION BASE CONDITIONS (75,000 CUBIC YARDS)

July - Peak Production Days

	9:00am - 10:00am		11:00am - Noon		1:15pm - 2:15pm		4:45pm - 5:45pm	
	Vehicle	Truck	Vehicle	Truck	Vehicle	Truck	Vehicle	Truck
NB Left	0	1	0	1	0	1	0	0
NB Through	571	0	881	0	1041	0	1111	0
SB Through	664	0	856	0	873	0	879	0
SB Right	0	6	0	6	0	4	0	0
EB Left	0	6	0	6	0	4	4	0
EB Right	0	1	0	1	0	1	4	4
Harris Access In	0	7	0	7	0	5	0	0
Harris Access Out	0	7	0	7	0	5	8	0
			PCE	PCE	PCE	PCE	PCE	PCE
			4	881	4	1041	4	1111
			571	856	873	879	879	879
			664	0	0	0	0	0
			24	24	16	16	16	16
			24	0	0	0	4	4
			4	0	0	0	4	4

October - Peak Production Days

	9:00am - 10:00am		11:00am - Noon		1:15pm - 2:15pm		4:45pm - 5:45pm	
	Vehicle	Truck	Vehicle	Truck	Vehicle	Truck	Vehicle	Truck
NB Left	0	2	0	2	0	1	0	0
NB Through	438	0	676	0	799	0	853	0
SB Through	510	0	657	0	670	0	675	0
SB Right	0	8	0	8	0	6	0	0
EB Left	0	8	0	8	0	6	4	0
EB Right	0	2	0	2	0	1	4	4
Harris Access In	0	10	0	10	0	7	0	0
Harris Access Out	0	10	0	10	0	7	8	0
			PCE	PCE	PCE	PCE	PCE	PCE
			8	676	8	799	4	853
			438	657	670	675	675	675
			510	0	0	0	0	0
			32	32	24	24	24	24
			32	0	0	0	4	4
			8	0	0	0	4	4

PCE = Passenger Care Equivalent
Where: 1 Truck = 4 Passenger Car Equivalents

*See "Turn Movement Key" for the Movement Letter Designations.

BLACK BART DRIVE - TURN MOVEMENTS

2014 WITH WILLITS BYPASS CONSTRUCTION BASE CONDITIONS (75,000 CUBIC YARDS)

July - Peak Production Days

	9:00am - 10:00am	11:00am - Noon	1:15pm - 2:15pm	4:45pm - 5:45pm
	Vehicle & Truck	Vehicle & Truck	Vehicle & Truck	Vehicle & Truck
	PCE	PCE	PCE	PCE
NB Left	G 9	15	19	20
NB Through	H 568	872	1026	1095
SB Through	I 657	853	858	860
SB Right	J 7	26	25	21
EB Left	K 19	30	13	12
EB Right	L 13	9	19	19

October - Peak Production Days

	9:00am - 10:00am	11:00am - Noon	1:15pm - 2:15pm	4:45pm - 5:45pm
	Vehicle & Truck	Vehicle & Truck	Vehicle & Truck	Vehicle & Truck
	PCE	PCE	PCE	PCE
NB Left	G 9	15	19	20
NB Through	H 437	669	786	837
SB Through	I 505	656	667	656
SB Right	J 7	26	26	21
EB Left	K 19	30	30	12
EB Right	L 13	9	9	19

PCE = Passenger Care Equivalent
Where: 1 Truck = 4 Passenger Car Equivalents

*See "Turn Movement Key" for the Movement Letter Designations.

HARRIS QUARRY DRIVEWAY - TURN MOVEMENTS

2030

BASE CONDITIONS (75,000 CUBIC YARDS)

July - Peak Production Days

	9:00am - 10:00am			11:00am - Noon			1:15pm - 2:15pm			4:45pm - 5:45pm		
	Vehicle	Truck	PCE	Vehicle	Truck	PCE	Vehicle	Truck	PCE	Vehicle	Truck	PCE
NB Left	A	0	3	0	3	12	0	2	8	0	0	0
NB Through	B	729	0	1125	0	1125	1329	0	1329	1418	0	1418
SB Through	C	848	0	1093	0	1093	1114	0	1114	1122	0	1122
SB Right	D	0	4	0	4	16	0	3	12	0	0	0
EB Left	E	0	4	0	4	16	0	3	12	4	0	4
EB Right	F	0	3	0	3	12	0	2	8	4	0	4
Harris Access In	A+D	0	7	0	7	28	0	5	20	0	0	0
Harris Access Out	E+F	0	7	0	7	28	0	5	20	8	0	8

October - Peak Production Days

	9:00am - 10:00am			11:00am - Noon			1:15pm - 2:15pm			4:45pm - 5:45pm		
	Vehicle	Truck	PCE	Vehicle	Truck	PCE	Vehicle	Truck	PCE	Vehicle	Truck	PCE
NB Left	A	0	3	0	3	12	0	3	12	0	0	0
NB Through	B	559	0	863	0	863	1021	0	1021	1089	0	1089
SB Through	C	652	0	839	0	839	856	0	856	862	0	862
SB Right	D	0	6	0	6	24	0	4	16	0	0	0
EB Left	E	0	6	0	6	24	0	4	16	4	0	4
EB Right	F	0	3	0	3	12	0	3	12	4	0	4
Harris Access In	A+D	0	9	0	9	36	0	7	28	0	0	0
Harris Access Out	E+F	0	9	0	9	36	0	7	28	8	0	8

PCE = Passenger Care Equivalent
Where: 1 Truck = 4 Passenger Car Equivalents

*See "Turn Movement Key" for the Movement Letter Designations.

BLACK BART DRIVE - TURN MOVEMENTS

2030

BASE CONDITIONS (75,000 CUBIC YARDS)

July - Peak Production Days

	9:00am - 10:00am	11:00am - Noon	1:15pm - 2:15pm	4:45pm - 5:45pm
	Vehicle & Truck	Vehicle & Truck	Vehicle & Truck	Vehicle & Truck
	PCE	PCE	PCE	PCE
NB Left	12	20	24	26
NB Through	721	1109	1308	1396
SB Through	835	1085	1093	1098
SB Right	9	33	32	27
EB Left	24	38	17	15
EB Right	17	12	24	24

October - Peak Production Days

	9:00am - 10:00am	11:00am - Noon	1:15pm - 2:15pm	4:45pm - 5:45pm
	Vehicle & Truck	Vehicle & Truck	Vehicle & Truck	Vehicle & Truck
	PCE	PCE	PCE	PCE
NB Left	12	20	24	26
NB Through	553	849	1001	1067
SB Through	641	833	848	838
SB Right	9	33	33	27
EB Left	24	38	38	15
EB Right	17	12	12	24

PCE = Passenger Care Equivalent
Where: 1 Truck = 4 Passenger Car Equivalents

*See "Turn Movement Key" for the Movement Letter Designations.

HARRIS QUARRY DRIVEWAY - TURN MOVEMENTS

2040

BASE CONDITIONS (75,000 CUBIC YARDS)

July - Peak Production Days

	9:00am - 10:00am			11:00am - Noon			1:15pm - 2:15pm			4:45pm - 5:45pm		
	Vehicle	Truck	PCE	Vehicle	Truck	PCE	Vehicle	Truck	PCE	Vehicle	Truck	PCE
NB Left	A	0	3	0	3	12	0	2	8	0	0	0
NB Through	B	828	0	1277	0	1277	1509	0	1509	1610	0	1610
SB Through	C	963	0	1241	0	1241	1265	0	1265	1274	0	1274
SB Right	D	0	4	0	4	16	0	3	12	0	0	0
EB Left	E	0	4	0	4	16	0	3	12	4	0	4
EB Right	F	0	3	0	3	12	0	2	8	4	0	4
Harris Access In	A+D	0	7	0	7	28	0	5	20	0	0	0
Harris Access Out	E+F	0	7	0	7	28	0	5	20	8	0	8

October - Peak Production Days

	9:00am - 10:00am			11:00am - Noon			1:15pm - 2:15pm			4:45pm - 5:45pm		
	Vehicle	Truck	PCE	Vehicle	Truck	PCE	Vehicle	Truck	PCE	Vehicle	Truck	PCE
NB Left	A	0	3	0	3	12	0	3	12	0	0	0
NB Through	B	635	0	980	0	980	1159	0	1159	1236	0	1236
SB Through	C	740	0	953	0	953	972	0	972	979	0	979
SB Right	D	0	6	0	6	24	0	4	16	0	0	0
EB Left	E	0	6	0	6	24	0	4	16	4	0	4
EB Right	F	0	3	0	3	12	0	3	12	4	0	4
Harris Access In	A+D	0	9	0	9	36	0	7	28	0	0	0
Harris Access Out	E+F	0	9	0	9	36	0	7	28	8	0	8

PCE = Passenger Care Equivalent
Where: 1 Truck = 4 Passenger Car Equivalents

*See "Turn Movement Key" for the Movement Letter Designations.

BLACK BART DRIVE - TURN MOVEMENTS

2040

BASE CONDITIONS (75,000 CUBIC YARDS)

July - Peak Production Days

	9:00am - 10:00am	11:00am - Noon	1:15pm - 2:15pm	4:45pm - 5:45pm
	Vehicle & Truck	Vehicle & Truck	Vehicle & Truck	Vehicle & Truck
	PCE	PCE	PCE	PCE
NB Left	14	22	27	29
NB Through	818	1259	1485	1585
SB Through	948	1231	1241	1247
SB Right	10	38	36	31
EB Left	27	43	19	17
EB Right	19	14	27	27

October - Peak Production Days

	9:00am - 10:00am	11:00am - Noon	1:15pm - 2:15pm	4:45pm - 5:45pm
	Vehicle & Truck	Vehicle & Truck	Vehicle & Truck	Vehicle & Truck
	PCE	PCE	PCE	PCE
NB Left	14	22	27	29
NB Through	627	964	1136	1211
SB Through	727	945	962	952
SB Right	10	38	38	31
EB Left	27	43	43	17
EB Right	19	14	14	27

PCE = Passenger Care Equivalent
Where: 1 Truck = 4 Passenger Car Equivalents

*See "Turn Movement Key" for the Movement Letter Designations.

HARRIS QUARRY DRIVEWAY - TURN MOVEMENTS

2010

PROJECT CONDITIONS (200,000 CUBIC YARDS)

July - Peak Production Days

	9:00am - 10:00am			11:00am - Noon			1:15pm - 2:15pm			4:45pm - 5:45pm		
	Vehicle	Truck	PCE	Vehicle	Truck	PCE	Vehicle	Truck	PCE	Vehicle	Truck	PCE
NB Left	0	11	44	0	11	44	0	8	32	0	0	0
NB Through	531	0	531	820	0	820	969	0	969	1034	0	1034
SB Through	618	0	618	797	0	797	812	0	812	818	0	818
SB Right	0	8	32	0	8	32	0	7	28	0	0	0
EB Left	0	8	32	0	8	32	0	7	28	6	0	6
EB Right	0	11	44	0	11	44	0	8	32	6	0	6
Harris Access In	0	19	76	0	19	76	0	15	60	0	0	0
Harris Access Out	0	19	76	0	19	76	0	15	60	12	0	12

October - Peak Production Days

	9:00am - 10:00am			11:00am - Noon			1:15pm - 2:15pm			4:45pm - 5:45pm		
	Vehicle	Truck	PCE	Vehicle	Truck	PCE	Vehicle	Truck	PCE	Vehicle	Truck	PCE
NB Left	0	14	56	0	14	56	0	11	44	0	0	0
NB Through	408	0	408	629	0	629	744	0	744	794	0	794
SB Through	475	0	475	612	0	612	624	0	624	628	0	628
SB Right	0	11	44	0	11	44	0	8	32	0	0	0
EB Left	0	11	44	0	11	44	0	8	32	6	0	6
EB Right	0	14	56	0	14	56	0	11	44	6	0	6
Harris Access In	0	25	100	0	25	100	0	19	76	0	0	0
Harris Access Out	0	25	100	0	25	100	0	19	76	12	0	12

PCE = Passenger Care Equivalent
Where: 1 Truck = 4 Passenger Car Equivalents

*See "Turn Movement Key" for the Movement Letter Designations.

BLACK BART DRIVE - TURN MOVEMENTS

2010

PROJECT CONDITIONS (200,000 CUBIC YARDS)

July - Peak Production Days

	9:00am - 10:00am	11:00am - Noon	1:15pm - 2:15pm	4:45pm - 5:45pm
	Vehicle & Truck	Vehicle & Truck	Vehicle & Truck	Vehicle & Truck
	PCE	PCE	PCE	PCE
NB Left	G 9	14	18	17
NB Through	H 530	814	958	1023
SB Through	I 614	796	801	800
SB Right	J 7	24	23	20
EB Left	K 18	28	12	11
EB Right	L 12	9	18	18

October - Peak Production Days

	9:00am - 10:00am	11:00am - Noon	1:15pm - 2:15pm	4:45pm - 5:45pm
	Vehicle & Truck	Vehicle & Truck	Vehicle & Truck	Vehicle & Truck
	PCE	PCE	PCE	PCE
NB Left	G 9	14	18	19
NB Through	H 410	626	734	781
SB Through	I 474	614	623	610
SB Right	J 7	24	24	20
EB Left	K 18	28	28	11
EB Right	L 12	9	9	18

PCE = Passenger Care Equivalent
 Where: 1 Truck = 4 Passenger Car Equivalents

*See "Turn Movement Key" for the Movement Letter Designations.

HARRIS QUARRY DRIVEWAY - TURN MOVEMENTS

2014 WITHOUT WILLITS BYPASS CONSTRUCTION PROJECT CONDITIONS (200,000 CUBIC YARDS)

July - Peak Production Days

	9:00am - 10:00am			11:00am - Noon			1:15pm - 2:15pm			4:45pm - 5:45pm		
	Vehicle	Truck	PCE	Vehicle	Truck	PCE	Vehicle	Truck	PCE	Vehicle	Truck	PCE
NB Left	0	11	44	0	11	44	0	8	32	0	0	0
NB Through	571	0	571	881	0	881	1041	0	1041	1111	0	1111
SB Through	664	0	664	856	0	856	873	0	873	879	0	879
SB Right	0	8	32	0	8	32	0	7	28	0	0	0
EB Left	0	8	32	0	8	32	0	7	28	6	0	6
EB Right	0	11	44	0	11	44	0	8	32	6	0	6
Harris Access In	0	19	76	0	19	76	0	15	60	0	0	0
Harris Access Out	0	19	76	0	19	76	0	15	60	12	0	12

October - Peak Production Days

	9:00am - 10:00am			11:00am - Noon			1:15pm - 2:15pm			4:45pm - 5:45pm		
	Vehicle	Truck	PCE	Vehicle	Truck	PCE	Vehicle	Truck	PCE	Vehicle	Truck	PCE
NB Left	0	14	56	0	14	56	0	11	44	0	0	0
NB Through	438	0	438	676	0	676	799	0	799	853	0	853
SB Through	510	0	510	657	0	657	670	0	670	675	0	675
SB Right	0	11	44	0	11	44	0	8	32	0	0	0
EB Left	0	11	44	0	11	44	0	8	32	6	0	6
EB Right	0	14	56	0	14	56	0	11	44	6	0	6
Harris Access In	0	25	100	0	25	100	0	19	76	0	0	0
Harris Access Out	0	25	100	0	25	100	0	19	76	12	0	12

PCE = Passenger Care Equivalent
Where: 1 Truck = 4 Passenger Car Equivalents

*See "Turn Movement Key" for the Movement Letter Designations.

BLACK BART DRIVE - TURN MOVEMENTS

2014 WITHOUT WILLITS BYPASS CONSTRUCTION PROJECT CONDITIONS (200,000 CUBIC YARDS)

July - Peak Production Days

	9:00am - 10:00am	11:00am - Noon	1:15pm - 2:15pm	4:45pm - 5:45pm
	Vehicle & Truck	Vehicle & Truck	Vehicle & Truck	Vehicle & Truck
	PCE	PCE	PCE	PCE
NB Left	9	15	19	20
NB Through	570	874	1029	1097
SB Through	659	855	861	860
SB Right	7	26	25	21
EB Left	19	30	13	12
EB Right	13	9	19	19

October - Peak Production Days

	9:00am - 10:00am	11:00am - Noon	1:15pm - 2:15pm	4:45pm - 5:45pm
	Vehicle & Truck	Vehicle & Truck	Vehicle & Truck	Vehicle & Truck
	PCE	PCE	PCE	PCE
NB Left	9	15	19	20
NB Through	440	672	788	839
SB Through	508	659	669	656
SB Right	7	26	26	21
EB Left	19	30	30	12
EB Right	13	9	9	19

PCE = Passenger Care Equivalent
Where: 1 Truck = 4 Passenger Car Equivalents

*See "Turn Movement Key" for the Movement Letter Designations.

HARRIS QUARRY DRIVEWAY - TURN MOVEMENTS

2014 WITH WILLITS BYPASS CONSTRUCTION PROJECT CONDITIONS (200,000 CUBIC YARDS)

July - Peak Production Days

	9:00am - 10:00am			11:00am - Noon			1:15pm - 2:15pm			4:45pm - 5:45pm		
	Vehicle	Truck	PCE	Vehicle	Truck	PCE	Vehicle	Truck	PCE	Vehicle	Truck	PCE
NB Left	0	7	28	0	7	28	0	5	20	0	0	0
NB Through	571	0	571	881	0	881	1041	0	1041	1111	0	1111
SB Through	664	0	664	856	0	856	873	0	873	879	0	879
SB Right	0	12	48	0	12	48	0	10	40	0	0	0
EB Left	0	12	48	0	12	48	0	10	40	6	0	6
EB Right	0	7	28	0	7	28	0	5	20	6	0	6
Harris Access In	0	19	76	0	19	76	0	15	60	0	0	0
Harris Access Out	0	19	76	0	19	76	0	15	60	12	0	12

October - Peak Production Days

	9:00am - 10:00am			11:00am - Noon			1:15pm - 2:15pm			4:45pm - 5:45pm		
	Vehicle	Truck	PCE	Vehicle	Truck	PCE	Vehicle	Truck	PCE	Vehicle	Truck	PCE
NB Left	0	9	36	0	9	36	0	7	28	0	0	0
NB Through	438	0	438	676	0	676	799	0	799	853	0	853
SB Through	510	0	510	657	0	657	670	0	670	675	0	675
SB Right	0	16	64	0	16	64	0	12	48	0	0	0
EB Left	0	16	64	0	16	64	0	12	48	6	0	6
EB Right	0	9	36	0	9	36	0	7	28	6	0	6
Harris Access In	0	25	100	0	25	100	0	19	76	0	0	0
Harris Access Out	0	25	100	0	25	100	0	19	76	12	0	12

PCE = Passenger Care Equivalent
Where: 1 Truck = 4 Passenger Car Equivalents

*See "Turn Movement Key" for the Movement Letter Designations.

BLACK BART DRIVE - TURN MOVEMENTS

2014 WITH WILLITS BYPASS CONSTRUCTION PROJECT CONDITIONS (200,000 CUBIC YARDS)

July - Peak Production Days

	9:00am - 10:00am	11:00am - Noon	1:15pm - 2:15pm	4:45pm - 5:45pm
	Vehicle & Truck	Vehicle & Truck	Vehicle & Truck	Vehicle & Truck
	PCE	PCE	PCE	PCE
NB Left	9	15	19	20
NB Through	574	878	1032	1097
SB Through	663	859	864	860
SB Right	7	26	25	21
EB Left	19	30	13	12
EB Right	13	9	19	19

October - Peak Production Days

	9:00am - 10:00am	11:00am - Noon	1:15pm - 2:15pm	4:45pm - 5:45pm
	Vehicle & Truck	Vehicle & Truck	Vehicle & Truck	Vehicle & Truck
	PCE	PCE	PCE	PCE
NB Left	9	15	19	20
NB Through	445	677	792	839
SB Through	513	664	673	656
SB Right	7	26	26	21
EB Left	19	30	30	12
EB Right	13	9	9	19

PCE = Passenger Care Equivalent
Where: 1 Truck = 4 Passenger Car Equivalents

*See "Turn Movement Key" for the Movement Letter Designations.

HARRIS QUARRY DRIVEWAY - TURN MOVEMENTS

2030

PROJECT CONDITIONS (200,000 CUBIC YARDS)

July - Peak Production Days

	9:00am - 10:00am			11:00am - Noon			1:15pm - 2:15pm			4:45pm - 5:45pm		
	Vehicle	Truck	PCE	Vehicle	Truck	PCE	Vehicle	Truck	PCE	Vehicle	Truck	PCE
NB Left	0	11	44	0	11	44	0	8	32	0	0	0
NB Through	729	0	729	1125	0	1125	1329	0	1329	1418	0	1418
SB Through	848	0	848	1093	0	1093	1114	0	1114	1122	0	1122
SB Right	0	8	32	0	8	32	0	7	28	0	0	0
EB Left	0	8	32	0	8	32	0	7	28	6	0	6
EB Right	0	11	44	0	11	44	0	8	32	6	0	6
Harris Access In	0	19	76	0	19	76	0	15	60	0	0	0
Harris Access Out	0	19	76	0	19	76	0	15	60	12	0	12

October - Peak Production Days

	9:00am - 10:00am			11:00am - Noon			1:15pm - 2:15pm			4:45pm - 5:45pm		
	Vehicle	Truck	PCE	Vehicle	Truck	PCE	Vehicle	Truck	PCE	Vehicle	Truck	PCE
NB Left	0	14	56	0	14	56	0	11	44	0	0	0
NB Through	559	0	559	863	0	863	1021	0	1021	1089	0	1089
SB Through	652	0	652	839	0	839	856	0	856	862	0	862
SB Right	0	11	44	0	11	44	0	8	32	0	0	0
EB Left	0	11	44	0	11	44	0	8	32	6	0	6
EB Right	0	14	56	0	14	56	0	11	44	6	0	6
Harris Access In	0	25	100	0	25	100	0	19	76	0	0	0
Harris Access Out	0	25	100	0	25	100	0	19	76	12	0	12

PCE = Passenger Care Equivalent
Where: 1 Truck = 4 Passenger Car Equivalents

*See "Turn Movement Key" for the Movement Letter Designations.

BLACK BART DRIVE - TURN MOVEMENTS

2030

PROJECT CONDITIONS (200,000 CUBIC YARDS)

July - Peak Production Days

	9:00am - 10:00am	11:00am - Noon	1:15pm - 2:15pm	4:45pm - 5:45pm
	Vehicle & Truck	Vehicle & Truck	Vehicle & Truck	Vehicle & Truck
	PCE	PCE	PCE	PCE
NB Left	12	20	24	26
NB Through	725	1113	1312	1398
SB Through	839	1089	1097	1098
SB Right	9	33	32	27
EB Left	24	38	17	15
EB Right	17	12	24	24

October - Peak Production Days

	9:00am - 10:00am	11:00am - Noon	1:15pm - 2:15pm	4:45pm - 5:45pm
	Vehicle & Truck	Vehicle & Truck	Vehicle & Truck	Vehicle & Truck
	PCE	PCE	PCE	PCE
NB Left	12	20	24	26
NB Through	558	854	1005	1069
SB Through	646	838	852	838
SB Right	9	33	33	27
EB Left	24	38	38	15
EB Right	17	12	12	24

PCE = Passenger Care Equivalent
Where: 1 Truck = 4 Passenger Car Equivalents

*See "Turn Movement Key" for the Movement Letter Designations.

HARRIS QUARRY DRIVEWAY - TURN MOVEMENTS

2040

PROJECT CONDITIONS (200,000 CUBIC YARDS)

July - Peak Production Days

	9:00am - 10:00am			11:00am - Noon			1:15pm - 2:15pm			4:45pm - 5:45pm		
	Vehicle	Truck	PCE	Vehicle	Truck	PCE	Vehicle	Truck	PCE	Vehicle	Truck	PCE
NB Left	0	11	44	0	11	44	0	8	32	0	0	0
NB Through	828	0	828	1277	0	1277	1509	0	1509	1610	0	1610
SB Through	963	0	963	1241	0	1241	1265	0	1265	1274	0	1274
SB Right	0	8	32	0	8	32	0	7	28	0	0	0
EB Left	0	8	32	0	8	32	0	7	28	6	0	6
EB Right	0	11	44	0	11	44	0	8	32	6	0	6
Harris Access In	0	19	76	0	19	76	0	15	60	0	0	0
Harris Access Out	0	19	76	0	19	76	0	15	60	12	0	12

October - Peak Production Days

	9:00am - 10:00am			11:00am - Noon			1:15pm - 2:15pm			4:45pm - 5:45pm		
	Vehicle	Truck	PCE	Vehicle	Truck	PCE	Vehicle	Truck	PCE	Vehicle	Truck	PCE
NB Left	0	14	56	0	14	56	0	11	44	0	0	0
NB Through	635	0	635	980	0	980	1159	0	1159	1236	0	1236
SB Through	740	0	740	953	0	953	972	0	972	979	0	979
SB Right	0	11	44	0	11	44	0	8	32	0	0	0
EB Left	0	11	44	0	11	44	0	8	32	6	0	6
EB Right	0	14	56	0	14	56	0	11	44	6	0	6
Harris Access In	0	25	100	0	25	100	0	19	76	0	0	0
Harris Access Out	0	25	100	0	25	100	0	19	76	12	0	12

PCE = Passenger Care Equivalent
Where: 1 Truck = 4 Passenger Car Equivalents

*See "Turn Movement Key" for the Movement Letter Designations.

BLACK BART DRIVE - TURN MOVEMENTS

2040

PROJECT CONDITIONS (200,000 CUBIC YARDS)

July - Peak Production Days

	9:00am - 10:00am	11:00am - Noon	1:15pm - 2:15pm	4:45pm - 5:45pm
	Vehicle & Truck	Vehicle & Truck	Vehicle & Truck	Vehicle & Truck
	PCE	PCE	PCE	PCE
NB Left	14	22	27	29
NB Through	822	1263	1489	1587
SB Through	952	1235	1245	1247
SB Right	10	38	36	31
EB Left	27	43	19	17
EB Right	19	14	27	27

October - Peak Production Days

	9:00am - 10:00am	11:00am - Noon	1:15pm - 2:15pm	4:45pm - 5:45pm
	Vehicle & Truck	Vehicle & Truck	Vehicle & Truck	Vehicle & Truck
	PCE	PCE	PCE	PCE
NB Left	14	22	27	29
NB Through	632	969	1140	1213
SB Through	732	950	966	952
SB Right	10	38	38	31
EB Left	27	43	43	17
EB Right	19	14	14	27

PCE = Passenger Care Equivalent
Where: 1 Truck = 4 Passenger Car Equivalents

*See "Turn Movement Key" for the Movement Letter Designations.

Appendix F

VMT Gravity Model Calculations

Vehicle Miles Traveled for Aggregate Production - Base Permit Conditions

Location	POPULATION 2000		A	D	T	F	P	J	K	A*K	Adjusted Trips	A*K*D
	Incorporated	Un-incorporated										
Boonville	943	2221	3164	74	86	0.000135	3720	0.502974581	0.137640275	435	89	6586
Boonville	943	2221	3164	98	138	0.000053	3720	0.195337114	0.053454499	169	312	30576
Boonville	943	2221	3164	87	126	0.000063	4960	0.312421265	0.085494875	271	487	42369
Boonville	943	2221	3164	119	160	0.000039	1240	0.04843755	0.013255045	42	119	14161
Boonville	943	2221	3164	205	250	0.000016	992	0.015872	0.004343413	14	53	10865
Boonville	943	2221	3164	53.2	74	0.000183	9920	1.811541271	0.495732882	1568	615	32718
Boonville	943	2221	3164	119	156	0.000041	2480	0.10190664	0.027887012	88	41	4879
Boonville	943	2221	3164	65	90	0.000123	2480	0.30617284	0.083784977	265	159	10335
Boonville	943	2221	3164	133	160	0.000039	2480	0.096875	0.02651009	84	36	4788
Boonville	943	2221	3164	205	250	0.000016	2480	0.039968	0.010858533	34	115	23575
Boonville	943	2221	3164	123	154	0.000042	744	0.031371226	0.008584816	27	11	1353
Boonville	943	2221	3164	153	202	0.000025	1240	0.030389178	0.008316076	26	65	9945
Boonville	943	2221	3164	105	124	0.000065	2480	0.161290323	0.044137507	140	384	40320
				1539.2	1970		38936	3.654268937		3163	2486	

Location	POPULATION 2000		A	D	T	F	P	J	K	A*K	Adjusted Trips	A*K*D
	Incorporated	Un-incorporated										
Covelo	1838	536	2374	95	142	0.000050	3720	0.184487205	0.131208885	311	64	6080
Covelo	1838	536	2374	175	244	0.000017	3720	0.062483203	0.044438591	105	194	33950
Covelo	1838	536	2374	189	258	0.000015	4960	0.074514753	0.052995532	126	226	42714
Covelo	1838	536	2374	207	274	0.000013	1240	0.016516597	0.011746745	28	79	16353
Covelo	1838	536	2374	124	174	0.000033	992	0.032765227	0.02302911	55	208	25792
Covelo	1838	536	2374	144	206	0.000024	9920	0.233763785	0.166254813	395	155	22320
Covelo	1838	536	2374	105	194	0.000027	2480	0.065894356	0.046864633	111	52	5460
Covelo	1838	536	2374	166	224	0.000020	2480	0.04942602	0.03515221	83	50	8300
Covelo	1838	536	2374	48	76	0.000173	2480	0.429362881	0.30536657	725	306	14688
Covelo	1838	536	2374	125	174	0.000033	2480	0.081913066	0.058252777	138	466	56250
Covelo	1838	536	2374	114	192	0.000027	744	0.020182292	0.014353819	34	14	1596
Covelo	1838	536	2374	52	100	0.000100	1240	0.124	0.088189865	209	526	27352
Covelo	1838	536	2374	234	284	0.000012	2480	0.030747867	0.021868148	52	143	33462
				1778	2542		38936	1.406057253		2372	2483	

Location	POPULATION 2000		A	D	T	F	P	J	K	A*K	Adjusted Trips	A*K*D
	Incorporated	Un-incorporated										
FT Bragg/Mendocino	6771	3203	9974	81	100	0.000100	3720	0.372	0.093802153	936	192	15552
FT Bragg/Mendocino	6771	3203	9974	160	204	0.000024	3720	0.089388697	0.022539925	225	416	66560
FT Bragg/Mendocino	6771	3203	9974	174	218	0.000021	4960	0.104368319	0.026317132	262	471	81954
FT Bragg/Mendocino	6771	3203	9974	192	234	0.000018	1240	0.02264592	0.005710312	57	161	30912
FT Bragg/Mendocino	6771	3203	9974	116	164	0.000037	992	0.036882808	0.009300233	93	352	40832
FT Bragg/Mendocino	6771	3203	9974	129	166	0.000036	9920	0.359994194	0.090774813	905	355	45795
FT Bragg/Mendocino	6771	3203	9974	21	36	0.000772	2480	1.913580247	0.482521361	4813	2238	46998
FT Bragg/Mendocino	6771	3203	9974	152	182	0.000030	2480	0.074870185	0.018878991	188	113	11716
FT Bragg/Mendocino	6771	3203	9974	86	126	0.000063	2480	0.156210632	0.039389499	393	166	14276
FT Bragg/Mendocino	6771	3203	9974	116	144	0.000048	2480	0.119598765	0.030157585	301	1017	117972
FT Bragg/Mendocino	6771	3203	9974	25	34	0.000865	744	0.643598616	0.162287461	1619	676	16900
FT Bragg/Mendocino	6771	3203	9974	136	200	0.000025	1240	0.031	0.007816846	78	196	26656
FT Bragg/Mendocino	6771	3203	9974	219	244	0.000017	2480	0.041655469	0.01050369	105	288	63072
				1607	2052		38936	3.965793852		9975	6841	

Vehicle Miles Traveled for Aggregate Production - Base Permit Conditions

Hoplend	POPULATION 2000 Incorporated	Harris Quarry Base	Un-incorporated	Attractions	Roundtrip Distance	Travel Time	1/T ^{1/2}	Productions	F-P	F-P/Sum F-P	Trips	Miles Traveled
Hoplend	1005	Davis Pit	1229	2234	45	62	0.000260	3720	0.967741935	0.040733348	91	168
Hoplend	1005	Keithly Ranch	1229	2234	33	50	0.000400	4960	1.984	0.063508795	187	336
Hoplend	1005	DNA River	1229	2234	66	82	0.000149	1240	0.184414039	0.007762195	17	48
Hoplend	1005	Cooks Humboldt	1229	2234	191	230	0.000019	992	0.018752363	0.000789308	2	8
Hoplend	1005	Ford Gravel	1229	2234	23	40	0.000625	9920	6.2	0.260964985	583	5267
Hoplend	1005	Ten Mile	1229	2234	163	204	0.000024	2480	0.059592464	0.002508314	6	3
Hoplend	1005	Pieta	1229	2234	11	14	0.000510	2480	12.65306122	0.532581601	1190	713
Hoplend	1005	Layton Rock	1229	2234	119	140	0.000051	2480	0.126530612	0.005325816	12	5
Hoplend	1005	Cooks Valley	1229	2234	191	130	0.000059	2480	0.146745562	0.006176686	14	47
Hoplend	1005	Wisley Ranch	1229	2234	166	202	0.000025	744	0.018233507	0.000767469	2	1
Hoplend	1005	Coal Mine	1229	2234	138	196	0.000026	1240	0.032278217	0.001358627	3	8
Hoplend	1005	Syar Healdsburg	1229	2234	79	76	0.000173	2480	0.429362881	0.018072367	40	110
					1285	1489		38936	23.7579766		2235	1694

Laytonville	POPULATION 2000 Incorporated	Harris Quarry Base	Un-incorporated	Attractions	Roundtrip Distance	Travel Time	1/T ^{1/2}	Productions	F-P	F-P/Sum F-P	Trips	Miles Traveled
Laytonville	1310	Davis Pit	2695	4005	137	174	0.000033	3720	0.1228696	0.002312165	9	17
Laytonville	1310	Keithly Ranch	2695	4005	151	190	0.000028	4960	0.137396122	0.002585526	10	18
Laytonville	1310	DNA River	2695	4005	189	206	0.000024	1240	0.029220473	0.000549872	2	6
Laytonville	1310	Cooks Humboldt	2695	4005	73	90	0.000123	992	0.122469136	0.002304629	9	34
Laytonville	1310	Ford Gravel	2695	4005	106	138	0.000053	9920	0.52098971	0.009802298	39	15
Laytonville	1310	Ten Mile	2695	4005	53	110	0.000083	2480	0.204958678	0.003856921	15	7
Laytonville	1310	Pieta	2695	4005	129	154	0.000042	2480	0.104570754	0.001967817	8	5
Laytonville	1310	Layton Rock	2695	4005	5	7	0.020408	2480	50.6122449	0.952423239	3814	1612
Laytonville	1310	Cooks Valley	2695	4005	73	90	0.000123	2480	0.30617284	0.005761573	23	78
Laytonville	1310	Wisley Ranch	2695	4005	63	100	0.000100	744	0.0744	0.001400062	6	3
Laytonville	1310	Coal Mine	2695	4005	49	96	0.000109	1240	0.134548611	0.002531941	10	25
Laytonville	1310	Syar Healdsburg	2695	4005	196	216	0.000021	2480	0.053155007	0.001000273	4	11
					1262	1643		4003	53.14049768		4003	1842

Potter Valley	POPULATION 2000 Incorporated	Harris Quarry Base	Un-incorporated	Attractions	Roundtrip Distance	Travel Time	1/T ^{1/2}	Productions	F-P	F-P/Sum F-P	Trips	Miles Traveled
Potter Valley	1275	Davis Pit	616	1891	63	94	0.000113	3720	0.42100498	0.102585363	194	359
Potter Valley	1275	Keithly Ranch	616	1891	79	112	0.000080	4960	0.395408163	0.096348243	182	327
Potter Valley	1275	DNA River	616	1891	96	124	0.000065	1240	0.080645161	0.01965063	37	105
Potter Valley	1275	Cooks Humboldt	616	1891	175	226	0.000020	992	0.019422038	0.004732525	9	34
Potter Valley	1275	Ford Gravel	616	1891	51	80	0.000156	9920	1.55	0.377685111	714	280
Potter Valley	1275	Ten Mile	616	1891	147	200	0.000025	2480	0.062	0.015107404	29	13
Potter Valley	1275	Pieta	616	1891	74	96	0.000109	2480	0.269097222	0.065570332	124	74
Potter Valley	1275	Layton Rock	616	1891	103	134	0.000056	2480	0.138115393	0.033654276	64	27
Potter Valley	1275	Cooks Valley	616	1891	175	226	0.000020	2480	0.048555094	0.011831314	22	74
Potter Valley	1275	Wisley Ranch	616	1891	150	198	0.000026	744	0.018977655	0.004624244	9	4
Potter Valley	1275	Coal Mine	616	1891	122	124	0.000027	1240	0.033637153	0.008196291	15	38
Potter Valley	1275	Syar Healdsburg	616	1891	142	158	0.000040	2480	0.099343054	0.024206705	46	126
					1421	1902		4003	4.103947849		1891	1552

Vehicle Miles Traveled for Aggregate Production - Base Permit Conditions

Redwood Valley	Harris Quarry Base	POPULATION 2000		Attractions	Roundtrip Distance	Travel Time	1/T^2	Productions	F-P	F-P/Sum F-P	Trips	Miles Traveled
		Incorporated	Un-incorporated									
Redwood Valley	Harris Quarry Base	4992	2624	7616	19	26	0.001479	3720	5.50295858	0.483384384	3681	14326
Redwood Valley	Davis Pit	4992	2624	7616	61	86	0.000135	3720	0.502974581	0.044181699	336	37881
Redwood Valley	Keithly Ranch	4992	2624	7616	78	104	0.000092	4960	0.458579882	0.040282032	307	43056
Redwood Valley	DNA River	4992	2624	7616	94	116	0.000074	1240	0.0921522	0.008094725	62	16544
Redwood Valley	Cooks Humboldt	4992	2624	7616	149	190	0.000028	992	0.027479224	0.002413798	18	10132
Redwood Valley	Ford Gravel	4992	2624	7616	33	52	0.000370	9920	3.668639053	0.322256256	2454	31746
Redwood Valley	Ten Mile	4992	2624	7616	121	164	0.000037	2480	0.09220702	0.00809954	62	3509
Redwood Valley	Pieta	4992	2624	7616	56	70	0.000204	2480	0.506122449	0.04445821	339	11368
Redwood Valley	Layton Rock	4992	2624	7616	78	102	0.000096	2480	0.238369858	0.020938603	159	5226
Redwood Valley	Cooks Valley	4992	2624	7616	149	190	0.000028	2480	0.068698061	0.006034494	46	23095
Redwood Valley	Wisley Ranch	4992	2624	7616	125	162	0.000038	744	0.028349337	0.002490229	19	1000
Redwood Valley	Coal Mine	4992	2624	7616	96	156	0.000041	1240	0.05095332	0.004475781	34	8256
Redwood Valley	Syar Healdsburg	4992	2624	7616	124	130	0.000059	2480	0.146745562	0.01289025	98	33356
Redwood Valley		4992	2624	7616	1183	1548			11.38422913		7615	3950

Ukiah	Harris Quarry Base	POPULATION 2000		Attractions	Roundtrip Distance	Travel Time	1/T^2	Productions	F-P	F-P/Sum F-P	Trips	Miles Traveled
		Incorporated	Un-incorporated									
Ukiah	Harris Quarry Base	24762	3935	28697	32	36	0.000772	3720	2.87037037	0.186326304	5347	35040
Ukiah	Davis Pit	24762	3935	28697	72	94	0.000113	3720	0.42100498	0.027328983	784	104328
Ukiah	Keithly Ranch	24762	3935	28697	62	84	0.000142	4960	0.702947846	0.045630931	1309	145824
Ukiah	DNA River	24762	3935	28697	95	116	0.000074	1240	0.0921522	0.005981938	172	46265
Ukiah	Cooks Humboldt	24762	3935	28697	163	200	0.000025	992	0.027479224	0.001609859	46	28362
Ukiah	Ford Gravel	24762	3935	28697	17	32	0.000977	9920	9.6875	0.628851275	18046	120326
Ukiah	Ten Mile	24762	3935	28697	135	176	0.000032	2480	0.080061983	0.005197118	149	9315
Ukiah	Pieta	24762	3935	28697	39	50	0.000400	2480	0.992	0.064394371	1848	43134
Ukiah	Layton Rock	24762	3935	28697	91	112	0.000080	2480	0.197704082	0.012833699	368	14196
Ukiah	Cooks Valley	24762	3935	28697	163	200	0.000025	2480	0.062	0.004024648	115	63407
Ukiah	Wisley Ranch	24762	3935	28697	138	174	0.000033	744	0.02457392	0.001595184	46	2622
Ukiah	Coal Mine	24762	3935	28697	110	166	0.000036	1240	0.044999274	0.002921068	84	23210
Ukiah	Syar Healdsburg	24762	3935	28697	107	110	0.000083	2480	0.204958678	0.013304622	382	112243
Ukiah		24762	3935	28697	1224	1550			15.40507333		28696	15634

Willits/Brooktrails	Harris Quarry Base	POPULATION 2000		Attractions	Roundtrip Distance	Travel Time	1/T^2	Productions	F-P	F-P/Sum F-P	Trips	Miles Traveled
		Incorporated	Un-incorporated									
Willits/Brooktrails	Harris Quarry Base	5336	2893	8229	11	14	0.005102	3720	18.97959184	0.833617016	6660	15455
Willits/Brooktrails	Davis Pit	5336	2893	8229	91	116	0.000074	4960	0.276456599	0.01214246	100	16835
Willits/Brooktrails	Keithly Ranch	5336	2893	8229	105	130	0.000059	4960	0.293491124	0.012890646	106	19950
Willits/Brooktrails	DNA River	5336	2893	8229	122	146	0.000047	1240	0.058172265	0.002555028	21	7198
Willits/Brooktrails	Cooks Humboldt	5336	2893	8229	119	148	0.000046	992	0.045288532	0.001989152	16	7259
Willits/Brooktrails	Ford Gravel	5336	2893	8229	59	78	0.000164	9920	1.630506246	0.071614699	589	13629
Willits/Brooktrails	Ten Mile	5336	2893	8229	91	122	0.000067	2480	0.166621876	0.007318326	60	2548
Willits/Brooktrails	Pieta	5336	2893	8229	82	96	0.000109	2480	0.269097222	0.011819223	97	4756
Willits/Brooktrails	Layton Rock	5336	2893	8229	48	60	0.000278	2480	0.688888889	0.03025721	249	5040
Willits/Brooktrails	Cooks Valley	5336	2893	8229	112	148	0.000046	2480	0.113221329	0.00497288	41	15668
Willits/Brooktrails	Wisley Ranch	5336	2893	8229	94	120	0.000069	744	0.051666667	0.002269291	19	752
Willits/Brooktrails	Coal Mine	5336	2893	8229	66	114	0.000077	1240	0.095413974	0.004190749	34	5676
Willits/Brooktrails	Syar Healdsburg	5336	2893	8229	150	158	0.000040	2480	0.099343054	0.004363321	36	14850
Willits/Brooktrails		5336	2893	8229	1150	1450			22.767760		8228	2654

Vehicle Miles Traveled for Aggregate Production - Project Conditions

POPULATION 2000	A	D	T	F	P	J	K	A*K	Initial	Reduction	Final	VMT		
													Incorporated	Un-incorporated
Boonville	Harris Quarry Base	3164	74	86	0.000135	10160	1.373715522	0.303562878	961	326	127	199	14726	
Boonville	Davis Pit	3164	98	138	0.000053	3720	0.195337114	0.043168329	137	348	18	330	32340	
Boonville	Kethly Ranch	3164	87	126	0.000063	4960	0.312421265	0.069043223	218	525	29	496	43152	
Boonville	DNA River	3164	119	160	0.000039	1240	0.0484375	0.010704397	34	131	4	127	15113	
Boonville	Cooks Humboldt	3164	205	250	0.000016	992	0.015872	0.003507617	11	53	1	52	10660	
Boonville	Ford Gravel	3164	53.2	74	0.000183	9920	1.811541271	0.400339738	1267	673	167	506	26919.2	
Boonville	Ten Mile	3164	119	156	0.000041	2480	0.101906664	0.022520755	71	39	9	30	3570	
Boonville	Pieta	3164	65	90	0.000123	2480	0.30617284	0.067662356	214	162	28	134	8710	
Boonville	Layton Rock	3164	133	160	0.000039	2480	0.098875	0.021408793	32	9	9	23	3059	
Boonville	Cooks Valley	3164	205	250	0.000016	2480	0.039968	0.00769042	68	120	4	116	23780	
Boonville	Wisley Ranch	3164	123	154	0.000042	744	0.031371226	0.006932853	22	3	3	8	984	
Boonville	Coal Mine	3164	153	202	0.000025	1240	0.030389178	0.006715826	21	69	3	66	10098	
Boonville	Syar Healdsburg	3164	105	124	0.000065	2480	0.161230323	0.035644192	113	414	15	389	41895	
												2803	2486	
												Total Base Trips		2486
												Trips to be Reduced		417

POPULATION 2000	A	D	T	F	P	J	K	A*K	Initial	Reduction	Final	VMT		
													Incorporated	Un-incorporated
Covelo	Harris Quarry Base	2374	95	142	0.000050	10160	0.50386828	0.29202335	693	235	83	152	14440	
Covelo	Davis Pit	2374	175	244	0.000017	3720	0.062483203	0.036212945	86	218	10	208	36400	
Covelo	Kethly Ranch	2374	189	258	0.000015	4960	0.074514753	0.043185984	103	248	12	236	44604	
Covelo	DNA River	2374	207	274	0.000013	1240	0.016516597	0.009572406	23	88	3	85	17595	
Covelo	Cooks Humboldt	2374	336	374	0.000033	992	0.032765227	0.018989509	45	218	5	213	26412	
Covelo	Ford Gravel	2374	144	206	0.00024	9920	0.233763785	0.135480812	322	171	38	133	19152	
Covelo	Ten Mile	2374	105	194	0.000027	2480	0.065894556	0.038189923	91	50	11	39	4095	
Covelo	Pieta	2374	166	224	0.000020	2480	0.04942602	0.028645487	68	51	8	43	7138	
Covelo	Layton Rock	2374	48	76	0.000173	2480	0.429362881	0.248842786	591	279	71	208	9984	
Covelo	Cooks Valley	2374	125	174	0.000033	2480	0.081913066	0.047473772	113	482	13	469	58625	
Covelo	Wisley Ranch	2374	114	192	0.000027	744	0.020182292	0.011696907	28	14	3	11	1254	
Covelo	Coal Mine	2374	52	100	0.000100	1240	0.1124	0.071865797	171	559	20	539	28028	
Covelo	Syar Healdsburg	2374	234	284	0.000012	2480	0.030747867	0.017620323	42	154	5	149	34866	
												2767	284	
												Total Base Trips		2483
												Trips to be Reduced		284

POPULATION 2000	A	D	T	F	P	J	K	A*K	Initial	Reduction	Final	VMT		
													Incorporated	Un-incorporated
Ft Bragg/Mendocino	Harris Quarry Base	9974	81	100	0.000100	10160	1.016	0.220400311	2198	745	226	519	42039	
Ft Bragg/Mendocino	Davis Pit	9974	160	204	0.000024	3720	0.089388697	0.01939104	193	490	20	470	75200	
Ft Bragg/Mendocino	Kethly Ranch	9974	174	218	0.000021	4960	0.104368319	0.022840561	226	544	23	521	90654	
Ft Bragg/Mendocino	DNA River	9974	192	234	0.000018	1240	0.02264592	0.004912567	49	188	5	183	35136	
Ft Bragg/Mendocino	Cooks Humboldt	9974	323	364	0.000037	992	0.036882808	0.008000967	80	387	8	379	43964	
Ft Bragg/Mendocino	Ford Gravel	9974	129	166	0.000036	9920	0.359994194	0.078093339	779	414	80	334	43086	
Ft Bragg/Mendocino	Ten Mile	9974	21	36	0.000772	2480	1.913580247	0.415111892	4140	2275	426	1849	38829	
Ft Bragg/Mendocino	Pieta	9974	152	182	0.000030	2480	0.074870185	0.016241547	162	123	17	106	16112	
Ft Bragg/Mendocino	Layton Rock	9974	86	126	0.000063	2480	0.156210632	0.033886685	338	160	35	125	10750	
Ft Bragg/Mendocino	Cooks Valley	9974	116	144	0.000048	2480	0.11958765	0.025944493	259	1106	27	1079	125164	
Ft Bragg/Mendocino	Wisley Ranch	9974	25	34	0.000065	744	0.643598616	0.139615488	1393	687	143	544	13600	
Ft Bragg/Mendocino	Coal Mine	9974	136	200	0.000025	1240	0.031	0.006724813	67	219	7	212	28832	
Ft Bragg/Mendocino	Syar Healdsburg	9974	219	244	0.000017	2480	0.041655469	0.009036298	90	330	9	321	70299	
												7668	1026	
												Total Base Trips		6641
												Trips to be Reduced		1027

Vehicle Miles Traveled for Aggregate Production - Project Conditions

POPULATION 2000	Incorporated	Un-incorporated	Attractions	Roundtrip Distance	Travel Time	1/T * 2	Productions	F/P	F/P/Sum F/P	A*/K Trips	Initial Adjusted Trips	Reduction	Final Adjusted Trips	VMT	
															1229
Hopland	1005	1229	2234	2234	60	63	0.000252	10160	2.58983875	0.100858279	225	76	44	32	1920
Hopland	1005	1229	2234	2234	45	62	0.000260	3720	0.967741935	0.038129271	85	216	17	199	8955
Hopland	1005	1229	2234	2234	33	50	0.000400	4960	0.07817009	0.007265959	175	421	34	367	12771
Hopland	1005	1229	2234	2234	66	82	0.000149	1240	0.184414039	0.007265959	16	61	3	58	3828
Hopland	1005	1229	2234	2234	191	230	0.000019	992	0.018752363	0.000738848	2	10	0	10	1910
Hopland	1005	1229	2234	2234	23	40	0.000625	9920	0.244281531	0.002347958	546	290	107	183	4209
Hopland	1005	1229	2234	2234	163	204	0.000024	2480	0.059592464	0.002347958	5	3	1	2	326
Hopland	1005	1229	2234	2234	11	14	0.005102	2480	12.65306122	0.498533737	1114	844	218	626	6886
Hopland	1005	1229	2234	2234	119	140	0.000051	2480	0.128530612	0.004985337	11	5	2	3	357
Hopland	1005	1229	2234	2234	191	130	0.000059	2480	0.146745562	0.005781811	13	55	3	52	9932
Hopland	1005	1229	2234	2234	166	202	0.000025	744	0.018233507	0.000718405	2	0	0	1	166
Hopland	1005	1229	2234	2234	138	196	0.000026	1240	0.032278217	0.00127177	3	10	1	9	1242
Hopland	1005	1229	2234	2234	79	76	0.000173	2480	0.423362881	0.016917004	38	139	7	132	10428
Total Base Trips														1694	
Trips to be Reduced														437	

POPULATION 2000	Incorporated	Un-incorporated	Attractions	Roundtrip Distance	Travel Time	1/T * 2	Productions	F/P	F/P/Sum F/P	A*/K Trips	Initial Adjusted Trips	Reduction	Final Adjusted Trips	VMT	
															2695
Laytonville	1310	2695	4005	4005	58	72	0.000193	10160	1.959876543	0.036038549	144	49	9	40	2320
Laytonville	1310	2695	4005	4005	137	174	0.000033	3720	0.1228696	0.002258347	9	23	1	22	3014
Laytonville	1310	2695	4005	4005	151	190	0.000028	4960	0.137396122	0.002526464	10	24	1	23	3473
Laytonville	1310	2695	4005	4005	169	206	0.000024	1240	0.029220473	0.00537311	2	8	0	8	1352
Laytonville	1310	2695	4005	4005	73	90	0.000123	992	0.122469136	0.002251984	9	44	1	43	3139
Laytonville	1310	2695	4005	4005	106	138	0.000053	9920	0.520898971	0.009578738	38	20	2	18	1908
Laytonville	1310	2695	4005	4005	53	110	0.000083	2480	0.204958678	0.00768816	15	8	1	7	371
Laytonville	1310	2695	4005	4005	129	154	0.000042	2480	0.104570754	0.001922865	8	6	0	6	774
Laytonville	1310	2695	4005	4005	5	7	0.020408	2480	50.6122449	0.930668718	3727	1762	233	1529	7645
Laytonville	1310	2695	4005	4005	73	90	0.000123	2480	0.30617284	0.005629959	23	98	1	97	7081
Laytonville	1310	2695	4005	4005	63	100	0.000100	744	0.0744	0.00136808	5	2	0	2	126
Laytonville	1310	2695	4005	4005	49	96	0.000109	1240	0.134548611	0.002474103	10	33	1	32	1568
Laytonville	1310	2695	4005	4005	196	216	0.000021	2480	0.053155007	0.000977423	4	15	0	15	2940
Total Base Trips														1842	
Trips to be Reduced														250	

POPULATION 2000	Incorporated	Un-incorporated	Attractions	Roundtrip Distance	Travel Time	1/T * 2	Productions	F/P	F/P/Sum F/P	A*/K Trips	Initial Adjusted Trips	Reduction	Final Adjusted Trips	VMT	
															616
Potter Valley	1275	616	1891	1891	44	62	0.000260	10160	2.643080125	0.45733679	865	293	58	235	10340
Potter Valley	1275	616	1891	1891	63	94	0.000113	3720	0.42100498	0.07284723	138	350	9	341	21483
Potter Valley	1275	616	1891	1891	79	112	0.000080	4960	0.395408163	0.068418168	129	310	9	301	23779
Potter Valley	1275	616	1891	1891	96	124	0.000065	1240	0.080645161	0.013954174	26	100	2	98	9408
Potter Valley	1275	616	1891	1891	175	226	0.000020	992	0.019422038	0.003360629	6	29	0	29	5075
Potter Valley	1275	616	1891	1891	51	80	0.000156	9920	1.55	0.268199219	507	269	34	235	11985
Potter Valley	1275	616	1891	1891	147	200	0.000025	2480	0.062	0.010727969	11	11	1	10	1470
Potter Valley	1275	616	1891	1891	74	96	0.000109	2480	0.269097222	0.046562364	88	67	6	61	4514
Potter Valley	1275	616	1891	1891	103	134	0.000056	2480	0.138115393	0.023989349	45	21	3	18	1854
Potter Valley	1275	616	1891	1891	175	226	0.000020	2480	0.04855094	0.008401573	16	68	1	67	11725
Potter Valley	1275	616	1891	1891	150	198	0.000026	744	0.018977655	0.003283737	6	3	0	3	450
Potter Valley	1275	616	1891	1891	122	192	0.000027	1240	0.033637153	0.005820296	11	36	1	35	4270
Potter Valley	1275	616	1891	1891	142	158	0.000040	2480	0.095343054	0.017189503	33	121	2	119	16898
Total Base Trips														1552	
Trips to be Reduced														126	

Vehicle Miles Traveled for Aggregate Production - Project Conditions

POPULATION 2000		Attractions	Roundtrip Distance	Travel Time	1/T * 2	Productions	F/P	F/P/Sum F/P	A/K Trips	Adjusted Trips	Initial	Reduction	Final Adjusted Trips	VMT
Incorporated	Un-incorporated													
Redwood Valley	4992	2624	7616	19	0.001479	10160	15.02595858	0.718745591	5474	1855	1692	163	32148	
Redwood Valley	4992	2624	7616	61	0.000135	3720	0.502974581	0.024053275	183	465	5	5	28060	
Redwood Valley	4992	2624	7616	78	0.000092	4960	0.459579882	0.02193023	167	402	5	5	30866	
Redwood Valley	4992	2624	7616	94	0.000074	1240	0.0921522	0.004406907	34	131	1	1	12220	
Redwood Valley	4992	2624	7616	149	0.000028	992	0.027479224	0.001314113	10	48	0	0	7152	
Redwood Valley	4992	2624	7616	33	0.000370	9920	3.668639053	0.175441837	1336	709	40	40	22077	
Redwood Valley	4992	2624	7616	121	0.000204	2480	0.09220702	0.004409529	34	19	1	1	2178	
Redwood Valley	4992	2624	7616	56	0.000204	2480	0.506122449	0.024203813	184	139	5	5	7504	
Redwood Valley	4992	2624	7616	78	0.000096	2480	0.238368688	0.011398335	87	41	3	3	2864	
Redwood Valley	4992	2624	7616	149	0.000028	2480	0.068698061	0.003285282	25	107	1	1	15794	
Redwood Valley	4992	2624	7616	125	0.000038	744	0.028349337	0.001355723	10	5	0	0	625	
Redwood Valley	4992	2624	7616	96	0.000041	1240	0.05095332	0.002436692	19	62	1	1	5856	
Redwood Valley	4992	2624	7616	124	0.000059	2480	0.146745562	0.007017673	53	194	2	2	23808	
				1183	1548		20.91086635			4177	227	227	3950	
Total Base Trips										3950				
Trips to be Reduced										227				

POPULATION 2000		Attractions	Roundtrip Distance	Travel Time	1/T * 2	Productions	F/P	F/P/Sum F/P	A/K Trips	Adjusted Trips	Initial	Reduction	Final Adjusted Trips	VMT
Incorporated	Un-incorporated													
Ukiah	24762	3935	28697	32	0.000772	10160	7.839506173	0.384775974	11042	3742	1083	58	85088	
Ukiah	24762	3935	28697	72	0.000113	3720	0.42100498	0.020663623	593	1506	58	1448	104256	
Ukiah	24762	3935	28697	62	0.000142	4960	0.702947846	0.034501847	990	2383	97	2286	141732	
Ukiah	24762	3935	28697	95	0.000074	1240	0.0921522	0.004522983	130	499	13	486	46170	
Ukiah	24762	3935	28697	163	0.000025	992	0.0248	0.001217225	35	169	3	166	27058	
Ukiah	24762	3935	28697	17	0.000977	9920	9.6875	0.475478579	13645	7246	1338	5908	100436	
Ukiah	24762	3935	28697	135	0.000032	2480	0.080061983	0.003929575	113	62	11	51	6885	
Ukiah	24762	3935	28697	39	0.000400	2480	0.992	0.048680006	1397	1058	137	921	35919	
Ukiah	24762	3935	28697	91	0.000080	2480	0.197704082	0.009703644	278	131	27	104	9464	
Ukiah	24762	3935	28697	163	0.000025	2480	0.062	0.003043063	87	371	9	362	59006	
Ukiah	24762	3935	28697	138	0.000033	744	0.02457392	0.001206129	35	17	3	14	1832	
Ukiah	24762	3935	28697	110	0.000036	1240	0.044959274	0.002208639	63	206	6	200	22000	
Ukiah	24762	3935	28697	107	0.000083	2480	0.204958678	0.010059712	289	1059	28	1031	110317	
				1224	1550		20.37420913			18449	2813	2813	15634	
Total Base Trips										15634				
Trips to be Reduced										2813				

POPULATION 2000		Attractions	Roundtrip Distance	Travel Time	1/T * 2	Productions	F/P	F/P/Sum F/P	A/K Trips	Adjusted Trips	Initial	Reduction	Final Adjusted Trips	VMT
Incorporated	Un-incorporated													
Willits/Brooktrails	5336	2893	8229	11	0.005102	10160	51.83673469	0.931897988	7669	2599	577	2022	22242	
Willits/Brooktrails	5336	2893	8229	91	0.000074	3720	0.276456599	0.004970015	41	104	3	101	9191	
Willits/Brooktrails	5336	2893	8229	105	0.000059	4960	0.293491124	0.005276254	43	103	3	100	10500	
Willits/Brooktrails	5336	2893	8229	122	0.000047	1240	0.058172265	0.01045795	9	35	1	34	4148	
Willits/Brooktrails	5336	2893	8229	119	0.000046	992	0.045288532	0.000814177	7	34	1	33	3927	
Willits/Brooktrails	5336	2893	8229	59	0.000164	9920	1.630506246	0.029312523	241	128	18	110	6490	
Willits/Brooktrails	5336	2893	8229	91	0.000067	2480	0.166621876	0.002995455	25	14	2	12	1092	
Willits/Brooktrails	5336	2893	8229	82	0.000109	2480	0.269097222	0.004837711	40	30	3	27	2214	
Willits/Brooktrails	5336	2893	8229	48	0.000278	2480	0.688888889	0.012384541	102	48	8	40	1820	
Willits/Brooktrails	5336	2893	8229	112	0.000046	2480	0.113221329	0.002035443	17	73	1	72	8064	
Willits/Brooktrails	5336	2893	8229	94	0.000069	744	0.051666667	0.000928841	8	4	1	3	282	
Willits/Brooktrails	5336	2893	8229	66	0.000077	1240	0.095413974	0.00171531	14	46	1	45	2970	
Willits/Brooktrails	5336	2893	8229	150	0.000040	2480	0.099343054	0.001785946	15	55	1	54	8100	
				1150	45376		55.624902			3273	620	2653	619	
Total Base Trips										2654				
Trips to be Reduced										619				

Vehicle Miles Traveled for Asphalt Production - Base Permit Conditions

POPULATION 2000	Incorporated	Un-incorporated	A	D	T	F	P	J	K	A^K	Adjusted Trips	A*K'D
Boonville	943	2221	3164	74	86	0.000135208	0	0	0	0	0	0
Boonville	943	2221	3164	53.2	74	0.000183	7500	1.369612856	0.868076803	2747	384	20429
Boonville	943	2221	3164	119	156	0.000041	2500	0.102728468	0.065110516	206	29	3451
Boonville	943	2221	3164	135	154	0.000042	2500	0.105414066	0.06681268	211	239	32265
				381.2	470		12500	1.577755391		3164	652	56145

POPULATION 2000	Incorporated	Un-incorporated	A	D	T	F	P	J	K	A^K	Adjusted Trips	Miles Traveled
Covelo	1838	536	2374	95	142	4.95933E-05	0	0	0	0	0	0
Covelo	1838	536	2374	144	206	0.000024	7500	0.176736733	0.658192032	1563	218	31392
Covelo	1838	536	2374	105	194	0.000027	2500	0.066425763	0.247378725	587	82	8610
Covelo	1838	536	2374	264	314	0.000010	2500	0.025355998	0.094429243	224	254	67056
				608	856		12500	0.268518494		2374	554	107058

POPULATION 2000	Incorporated	Un-incorporated	A	D	T	F	P	J	K	A^K	Adjusted Trips	Miles Traveled
Ft Bragg/Mendocino	6771	3203	9974	81	100	0.0001	0	0	0	0	0	0
Ft Bragg/Mendocino	6771	3203	9974	129	166	0.000036	7500	0.272173029	0.121805711	1215	170	21930
Ft Bragg/Mendocino	6771	3203	9974	21	36	0.000772	2500	1.929012346	0.86329171	8610	1203	25263
Ft Bragg/Mendocino	6771	3203	9974	249	274	0.000013	2500	0.033239959	0.014902579	149	169	42081
				480	576		12500	2.234484965		9974	1542	89274

POPULATION 2000	Incorporated	Un-incorporated	A	D	T	F	P	J	K	A^K	Adjusted Trips	Miles Traveled
Hopland	1005	1229	2234	60	63	0.000251953	0	0	0	0	0	0
Hopland	1005	1229	2234	23	40	0.000625	7500	4.6875	0.945443036	2112	295	6785
Hopland	1005	1229	2234	163	204	0.000024	2500	0.060073049	0.012116404	27	4	652
Hopland	1005	1229	2234	109	109	0.000084	2500	0.210419998	0.042440559	95	108	11772
				355	416		12500	4.957993047	1	2234	407	19209

POPULATION 2000	Incorporated	Un-incorporated	A	D	T	F	P	J	K	A^K	Adjusted Trips	Miles Traveled
Laytonville	1310	2695	4005	58	72	0.000192901	0	0	0	0	0	0
Laytonville	1310	2695	4005	106	138	0.000053	7500	0.393324827	0.613675396	2458	343	36358
Laytonville	1310	2695	4005	53	110	0.000083	2500	0.20661157	0.321951357	1289	180	9540
Laytonville	1310	2695	4005	226	246	0.000017	2500	0.041311389	0.064373247	258	293	66218
				443	566		2500	0.641747786		4005	816	112116

Vehicle Miles Traveled for Asphalt Production - Base Permit Conditions

Location	POPULATION 2000		Un-incorporated	Attractions	Roundtrip Distance	Travel Time	1/T^2	Productions	F*P	F*P/Sum F*P	Trips	Adjusted Trips	Miles Traveled
	Incorporated	Un-incorporated											
Potter Valley	1275	616	616	1891	44	62	0.000260146	0	0	0	0	0	0
Potter Valley	1275	616	616	1891	51	80	0.000156	7500	1.171875	0.897913946	1698	237	12087
Potter Valley	1275	616	616	1891	147	200	0.000025	2500	0.0625	0.047888744	91	13	1911
Potter Valley	1275	616	616	1891	172	188	0.000028	2500	0.070733364	0.054197311	102	116	19952
					414	530			1.305108364		1891	366	33950

Location	POPULATION 2000		Un-incorporated	Attractions	Roundtrip Distance	Travel Time	1/T^2	Productions	F*P	F*P/Sum F*P	Trips	Adjusted Trips	Miles Traveled
	Incorporated	Un-incorporated											
Redwood Valley	4992	2624	2624	7616	19	26	0.00147929	0	0	0	0	0	0
Redwood Valley	4992	2624	2624	7616	33	52	0.000370	7500	2.773668639	0.931470714	7094	991	32703
Redwood Valley	4992	2624	2624	7616	121	164	0.000037	2500	0.092950625	0.031215259	238	33	3993
Redwood Valley	4992	2624	2624	7616	154	150	0.000044	2500	0.111111111	0.037314027	284	322	49588
					327	392			2.977730375		7616	1346	86284

Location	POPULATION 2000		Un-incorporated	Attractions	Roundtrip Distance	Travel Time	1/T^2	Productions	F*P	F*P/Sum F*P	Trips	Adjusted Trips	Miles Traveled
	Incorporated	Un-incorporated											
Ukiah	24762	3935	3935	28697	32	36	0.000771605	0	0	0	0	0	0
Ukiah	24762	3935	3935	28697	17	32	0.0009377	7500	7.32421875	0.972351903	27904	3898	66266
Ukiah	24762	3935	3935	28697	135	176	0.000032	2500	0.080707645	0.010714622	307	43	5805
Ukiah	24762	3935	3935	28697	137	140	0.000051	2500	0.12755102	0.016933475	486	551	75487
					321	384			7.532477415		28697	4492	147558

Location	POPULATION 2000		Un-incorporated	Attractions	Roundtrip Distance	Travel Time	1/T^2	Productions	F*P	F*P/Sum F*P	Trips	Adjusted Trips	Miles Traveled
	Incorporated	Un-incorporated											
Willits/Brooktrails	5336	2893	2893	8229	11	14	0.005102041	0	0	0	0	0	0
Willits/Brooktrails	5336	2893	2893	8229	59	78	0.000164	7500	1.232741617	0.837778727	6894	963	56817
Willits/Brooktrails	5336	2893	2893	8229	91	122	0.000067	2500	0.167965601	0.114150447	939	131	11921
Willits/Brooktrails	5336	2893	2893	8229	180	188	0.000028	2500	0.070733364	0.048070826	396	449	80820
					341	402		12500	1.471441		8229	1543	149558

Vehicle Miles Traveled for Asphalt Production - Base Permit Conditions

POPULATION 2000	Incorporated	Un-incorporated	A	D	T	Travel Time	F	P	J	K	A^K	Adjusted Trips	A*K'D
Boonville	943	2221	3164	74	86	0.000135	3640	0.492157923	0.237767408	752	75	5550	
Boonville	943	2221	3164	53.2	74	0.000183	7500	1.369612856	0.661676432	2094	363	19312	
Boonville	943	2221	3164	119	156	0.000041	2500	0.102728468	0.049629358	157	32	3808	
Boonville	943	2221	3164	135	154	0.000042	2500	0.105414066	0.050926802	161	250	33750	
				381.2	470		16140	2.069913314		3164	720		

POPULATION 2000	Incorporated	Un-incorporated	Attractions	Roundtrip Distance	Travel Time	1/T^2	Productions	F*P	F*P*Sum F*P	Trips	Miles Traveled	
												Covelo
Covelo	1838	536	2374	144	206	0.000024	7500	0.176736733	0.3935895	934	162	23328
Covelo	1838	536	2374	105	194	0.000027	2500	0.066425763	0.147928969	351	71	7455
Covelo	1838	536	2374	264	314	0.000010	2500	0.025355998	0.056467348	134	208	54912
				608	856		16140	0.449038232		2373	536	

POPULATION 2000	Incorporated	Un-incorporated	Attractions	Roundtrip Distance	Travel Time	1/T^2	Productions	F*P	F*P*Sum F*P	Trips	Miles Traveled	
												Fl Bragg/Mendocino
Fl Bragg/Mendocino	6771	3203	9974	129	166	0.000036	7500	0.272173029	0.104742969	1045	181	23349
Fl Bragg/Mendocino	6771	3203	9974	21	36	0.000772	2500	1.929012346	0.742360403	7404	1496	31416
Fl Bragg/Mendocino	6771	3203	9974	249	274	0.000013	2500	0.033299959	0.012815002	128	199	49551
				480	576		16140	2.598484965		9974	2014	

POPULATION 2000	Incorporated	Un-incorporated	Attractions	Roundtrip Distance	Travel Time	1/T^2	Productions	F*P	F*P*Sum F*P	Trips	Miles Traveled	
												Hopland
Hopland	1005	1229	2234	23	40	0.000625	7500	4.6875	0.797858674	1782	309	7107
Hopland	1005	1229	2234	163	204	0.000024	2500	0.060073049	0.010225025	23	5	815
Hopland	1005	1229	2234	109	109	0.000084	2500	0.210419998	0.035815556	80	124	13516
				355	416		16140	5.875100631	1	2234	473	

POPULATION 2000	Incorporated	Un-incorporated	Attractions	Roundtrip Distance	Travel Time	1/T^2	Productions	F*P	F*P*Sum F*P	Trips	Miles Traveled	
												Laytonville
Laytonville	1310	2695	4005	106	138	0.000053	7500	0.393824827	0.293044423	1174	204	21624
Laytonville	1310	2695	4005	53	110	0.000083	2500	0.20661157	0.153739339	616	124	6572
Laytonville	1310	2695	4005	226	246	0.000017	2500	0.041311389	0.030739738	123	191	43166
				443	566		2500	1.34390828		4006	726	

Vehicle Miles Traveled for Asphalt Production - Base Permit Conditions

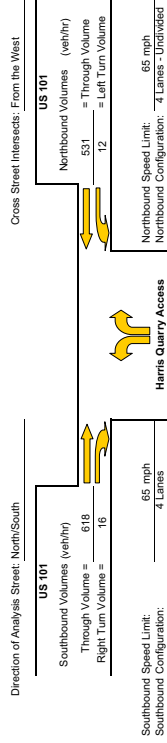
POPULATION 2000														
Incorporated	Un-incorporated	Attractions	Roundtrip Distance	Travel Time	1/T^2	Productions	F*P	F*P/Sum F*P	Trips	Miles Traveled	POPULATION 2000			
											Incorporated	Un-incorporated	Attractions	Roundtrip Distance
Potter Valley	Harris Quarry AC	616	1891	44	62	0.000260	3640	0.946930281	0.420476924	795	79	3476		
Potter Valley	Granite	616	1891	51	80	0.000156	7500	1.171875	0.520361852	984	171	8721		
Potter Valley	Baxman	616	1891	147	200	0.000025	2500	0.0625	0.027752632	52	11	1617		
Potter Valley	Syar/Bodean	616	1891	172	188	0.000028	2500	0.070733364	0.031408592	59	92	15824		
			414	530			2.252038644			1890	353			
POPULATION 2000														
Incorporated	Un-incorporated	Attractions	Roundtrip Distance	Travel Time	1/T^2	Productions	F*P	F*P/Sum F*P	Trips	Miles Traveled	POPULATION 2000			
											Incorporated	Un-incorporated	Attractions	Roundtrip Distance
Redwood Valley	Harris Quarry AC	2624	7616	19	26	0.001479	3640	5.384615385	0.643912072	4904	486	9234		
Redwood Valley	Granite	2624	7616	33	52	0.000370	7500	2.773668639	0.331685477	2526	438	14454		
Redwood Valley	Baxman	2624	7616	121	164	0.000037	2500	0.092950625	0.011115377	85	17	2057		
Redwood Valley	Syar/Bodean	2624	7616	154	150	0.000044	2500	0.111111111	0.013287075	101	157	24178		
			327	392			8.362345759			7616	1098			
POPULATION 2000														
Incorporated	Un-incorporated	Attractions	Roundtrip Distance	Travel Time	1/T^2	Productions	F*P	F*P/Sum F*P	Trips	Miles Traveled	POPULATION 2000			
											Incorporated	Un-incorporated	Attractions	Roundtrip Distance
Ukiah	Harris Quarry AC	3935	28697	32	36	0.000772	3640	2.808641975	0.271599415	7794	773	24736		
Ukiah	Granite	3935	28697	17	32	0.000977	7500	7.32421875	0.708261695	20325	3528	59976		
Ukiah	Baxman	3935	28697	135	176	0.000032	2500	0.080707645	0.007804537	224	45	6075		
Ukiah	Syar/Bodean	3935	28697	137	140	0.000051	2500	0.12755102	0.012334353	354	550	75350		
			321	384			10.34111939			28697	4896			
POPULATION 2000														
Incorporated	Un-incorporated	Attractions	Roundtrip Distance	Travel Time	1/T^2	Productions	F*P	F*P/Sum F*P	Trips	Miles Traveled	POPULATION 2000			
											Incorporated	Un-incorporated	Attractions	Roundtrip Distance
Willits/Brooktrails	Harris Quarry AC	2893	8229	11	14	0.005102	3640	18.57142857	0.926585532	7625	756	8316		
Willits/Brooktrails	Granite	2893	8229	59	78	0.000164	7500	1.232741617	0.061505247	506	88	5192		
Willits/Brooktrails	Baxman	2893	8229	91	122	0.000067	2500	0.167965601	0.008380317	69	14	1274		
Willits/Brooktrails	Syar/Bodean	2893	8229	180	188	0.000028	2500	0.070733364	0.003529104	29	45	8100		
			341	402			16140	20.042869		8229	903			

Appendix G

Warrant Calculations

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2010 Base Conditions - July Peak 9-10am



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 634
 Advancing Volume Va = 634
 If AV < Va then warrant is met

Right Turn Lane Warranted: **NO**

Southbound Right Turn Taper Warrants

(evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 800
 Advancing Volume Va = 634
 If AV < Va then warrant is met

Right Turn Taper Warranted: **NO**

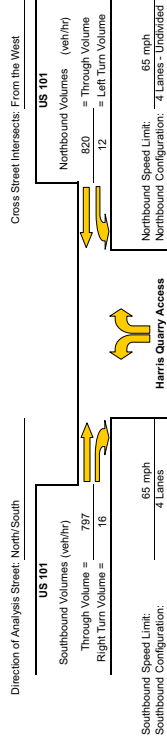
Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottel in 1981.

The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2010 Base Conditions - July Peak 11am-Noon



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 813
 Advancing Volume Va = 813
 If AV < Va then warrant is met

Right Turn Lane Warranted: **NO**

Southbound Right Turn Taper Warrants

(evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 800
 Advancing Volume Va = 813
 If AV < Va then warrant is met

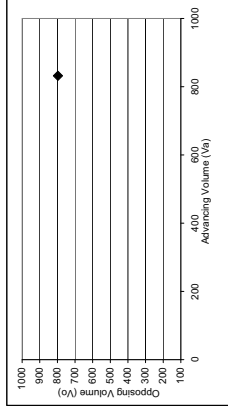
Right Turn Taper Warranted: **YES**

Northbound Left Turn Lane Warrants

Left Turn Volume Threshold LIVol = 11.7 veh/hr

Left Turn Volume V = 12 veh/hr

If V > LIVol then warrant is met



Study Intersection

Left Turn Lane Warranted: **YES**

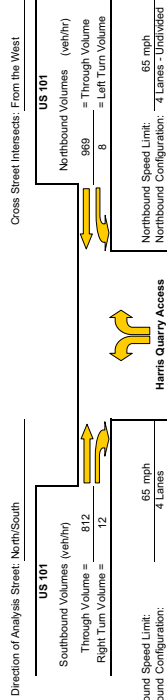
Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottel in 1981.

The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2010 Base Conditions - July Peak, 1:15-2:15pm



Southbound Right Turn Lane Warrants
 Left Turn Volume Threshold L/Vol = 11.5 veh/hr
 Left Turn Volume V = 8 veh/hr
 If V>L/Vol then warrant is met

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 824
 Advancing Volume Va =
 If AV<Va then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants
 (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 833.33333
 Advancing Volume Va = 824
 If AV<Va then warrant is met

Right Turn Taper Warranted: NO

Left Turn Lane Warranted: NO

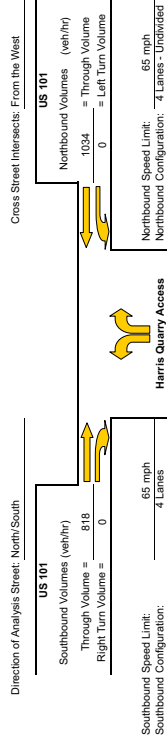
Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottel in 1981.

The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2010 Base Conditions - July Peak, 4:45-5:45pm



Southbound Right Turn Lane Warrants
 Left Turn Volume Threshold L/Vol = 11.4 veh/hr
 Left Turn Volume V = 0 veh/hr
 If V>L/Vol then warrant is met

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV =
 Advancing Volume Va =
 If AV<Va then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants
 (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 1333.33333
 Advancing Volume Va = 818
 If AV<Va then warrant is met

Right Turn Taper Warranted: NO

Left Turn Lane Warranted: NO

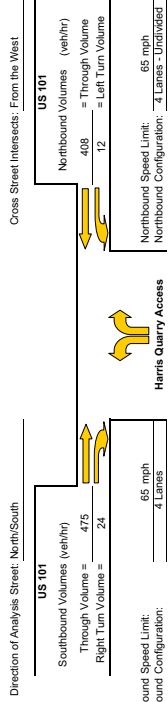
Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottel in 1981.

The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2010 Base Conditions - October Peak, 9-10am



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 499
 Advancing Volume Va = 499
 If AV/Va then warrant is met

Right Turn Lane Warranted: **NO**

Southbound Right Turn Taper Warrants

(evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 533.333333
 Advancing Volume Va = 499
 If AV/Va then warrant is met

Right Turn Taper Warranted: **NO**

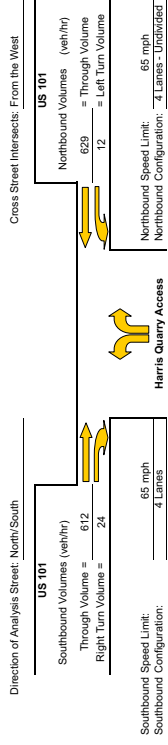
Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottel in 1981.

The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2010 Base Conditions - October Peak, 11am-Noon



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 636
 Advancing Volume Va = 636
 If AV/Va then warrant is met

Right Turn Lane Warranted: **NO**

Southbound Right Turn Taper Warrants

(evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 533.333333
 Advancing Volume Va = 636
 If AV/Va then warrant is met

Right Turn Taper Warranted: **YES**

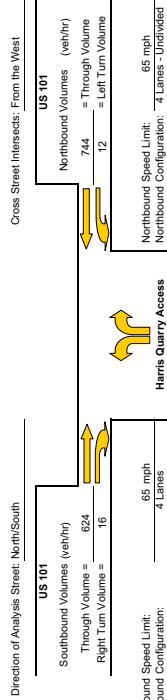
Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottel in 1981.

The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2010 Base Conditions - October Peak, 11:15-2:15pm



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 640
 Advancing Volume Va = 640
 If AV/Va then warrant is met

Right Turn Lane Warranted: **NO**

Southbound Right Turn Taper Warrants

(evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 800
 Advancing Volume Va = 640
 If AV/Va then warrant is met

Right Turn Taper Warranted: **NO**

Northbound Left Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 628
 Advancing Volume Va = 628
 If AV/Va then warrant is met

Right Turn Lane Warranted: **NO**

Southbound Right Turn Lane Warrants

(evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

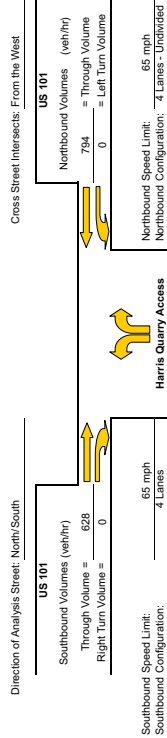
2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 1333.33333
 Advancing Volume Va = 628
 If AV/Va then warrant is met

Right Turn Taper Warranted: **NO**

Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.
 The right turn lane and taper analysis is based on work conducted by Cottel in 1981.
 The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2010 Base Conditions - October Peak, 4:45-5:45pm



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 628
 Advancing Volume Va = 628
 If AV/Va then warrant is met

Right Turn Lane Warranted: **NO**

Southbound Right Turn Taper Warrants

(evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 1333.33333
 Advancing Volume Va = 628
 If AV/Va then warrant is met

Right Turn Taper Warranted: **NO**

Northbound Left Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 14.6
 Advancing Volume Va = 14.6
 If AV/Va then warrant is met

Right Turn Lane Warranted: **NO**

Southbound Right Turn Lane Warrants

(evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

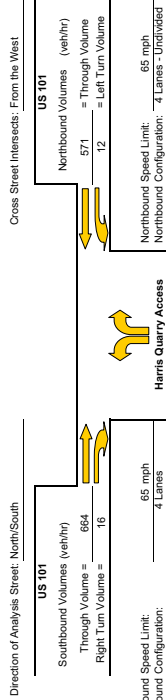
2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 1333.33333
 Advancing Volume Va = 628
 If AV/Va then warrant is met

Right Turn Taper Warranted: **NO**

Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.
 The right turn lane and taper analysis is based on work conducted by Cottel in 1981.
 The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris, Quarry Access
 Study Scenario: 2014, without W/Flts Bypass, Base Conditions - July Peak 9-10am



Southbound Right Turn Lane Warrants
 1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 680
 Advancing Volume Va = -
 If AV < Va then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants
 (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 800
 Advancing Volume Va = 680
 If AV < Va then warrant is met

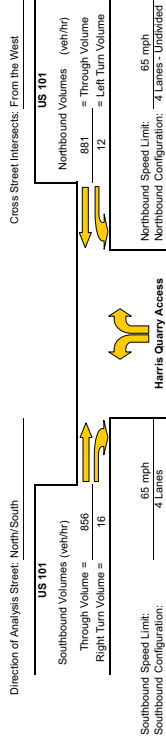
Right Turn Taper Warranted: NO

Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottel in 1981.
 The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris, Quarry Access
 Study Scenario: 2014, without W/Flts Bypass, Base Conditions - July Peak 11am-Noon



Southbound Right Turn Lane Warrants
 1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 872
 Advancing Volume Va = -
 If AV < Va then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants
 (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 800
 Advancing Volume Va = 872
 If AV < Va then warrant is met

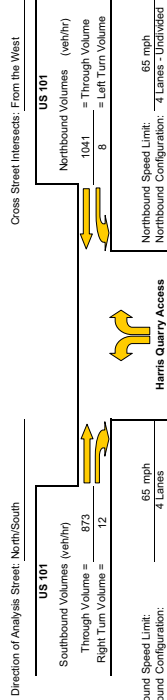
Right Turn Taper Warranted: YES

Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottel in 1981.
 The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101 Harris Quarry Access
 Study Scenario: 2014 without WFLA Bypass Base Conditions - July Peak - 1:15-2:15pm



Southbound Right Turn Lane Warrants
 1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 885
 Advancing Volume Va = 885
 If AV < Va then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants
 (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 833.33333
 Advancing Volume Va = 885
 If AV < Va then warrant is met

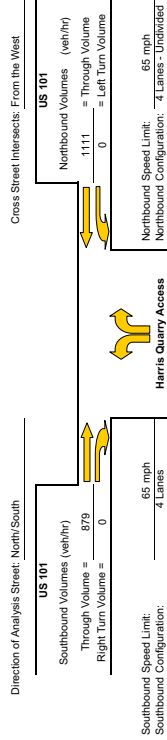
Right Turn Taper Warranted: NO

Left Turn Lane Warranted: NO

Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.
 The right turn lane and taper analysis is based on work conducted by Cottell in 1981.
 The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101 Harris Quarry Access
 Study Scenario: 2014 without WFLA Bypass Base Conditions - July Peak - 4:45-5:45pm



Southbound Right Turn Lane Warrants
 1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 879
 Advancing Volume Va = 879
 If AV < Va then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants
 (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 1333.33333
 Advancing Volume Va = 879
 If AV < Va then warrant is met

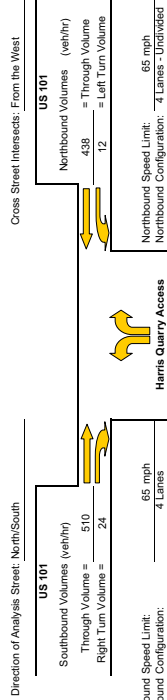
Right Turn Taper Warranted: NO

Left Turn Lane Warranted: NO

Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.
 The right turn lane and taper analysis is based on work conducted by Cottell in 1981.
 The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101 Harris Quarry Access
 Study Scenario: 2014 without Willis Bypass, Base Conditions - October Peak, 9-10am



Southbound Right Turn Lane Warrants
 1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 534
 Advancing Volume Va = 534
 If AV/Va then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants
 (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

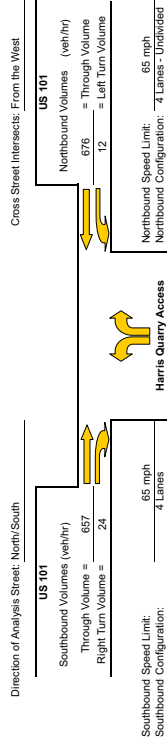
2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 533.33333
 Advancing Volume Va = 534
 If AV/Va then warrant is met

Right Turn Taper Warranted: YES

Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.
 The right turn lane and taper analysis is based on work conducted by Cottel in 1981.
 The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101 Harris Quarry Access
 Study Scenario: 2014 without Willis Bypass, Base Conditions - October Peak, 11am-Noon



Southbound Right Turn Lane Warrants
 1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 681
 Advancing Volume Va = 681
 If AV/Va then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants
 (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

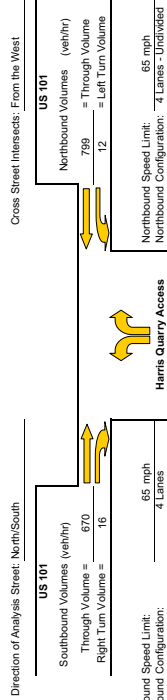
2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 633.33333
 Advancing Volume Va = 681
 If AV/Va then warrant is met

Right Turn Taper Warranted: YES

Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.
 The right turn lane and taper analysis is based on work conducted by Cottel in 1981.
 The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101 Harris Quarry Access
 Study Scenario: 2014 without Willis Bypass, Base Conditions - October Peak, 1:15-2:15pm



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 686
 Advancing Volume Va = 686
 If AV<Va then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants

- (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 800
 Advancing Volume Va = 686
 If AV<Va then warrant is met

Right Turn Taper Warranted: NO

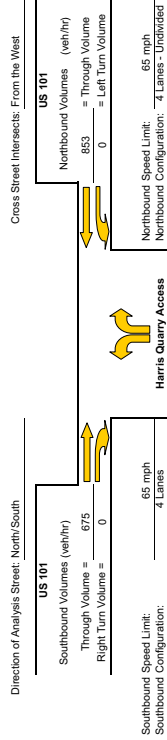
Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottell in 1981.

The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101 Harris Quarry Access
 Study Scenario: 2014 without Willis Bypass, Base Conditions - October Peak, 4:45-5:45pm



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 675
 Advancing Volume Va = 675
 If AV<Va then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants

- (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 1333.33333
 Advancing Volume Va = 675
 If AV<Va then warrant is met

Right Turn Taper Warranted: NO

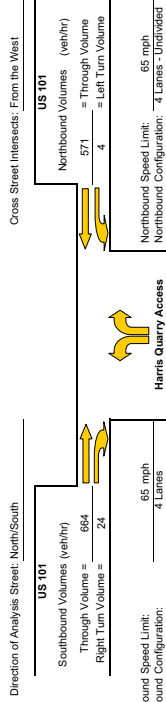
Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottell in 1981.

The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris, Quarry Access
 Study Scenario: 2014 with Willis Bypass, Base Conditions - July Peak 9-10am



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = -
 Advancing Volume Va = 688
 If AV<Va then warrant is met

Right Turn Lane Warranted: **NO**

Southbound Right Turn Taper Warrants

(evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 533.333333
 Advancing Volume Va = 688
 If AV<Va then warrant is met

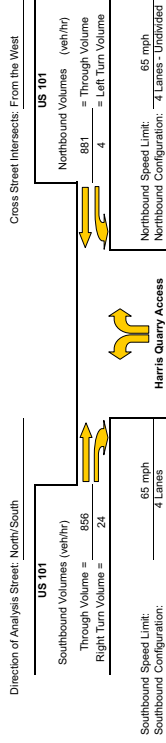
Right Turn Taper Warranted: **YES**

Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottel in 1981.
 The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris, Quarry Access
 Study Scenario: 2014 with Willis Bypass, Base Conditions - July Peak 11am-Noon



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = -
 Advancing Volume Va = 880
 If AV<Va then warrant is met

Right Turn Lane Warranted: **NO**

Southbound Right Turn Taper Warrants

(evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 533.333333
 Advancing Volume Va = 880
 If AV<Va then warrant is met

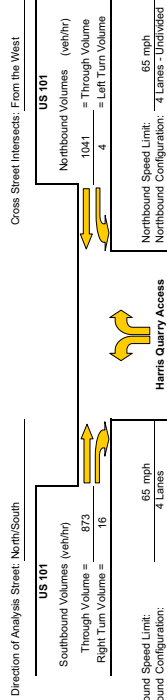
Right Turn Taper Warranted: **YES**

Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottel in 1981.
 The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris, Quarry Access
 Study Scenario: 2014 with Willis Bypass, Base Conditions - July Peak, 1:15-2:15pm



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 889
 Advancing Volume Va = 0
 If AV < Va then warrant is met

Right Turn Lane Warranted: **NO**

Southbound Right Turn Taper Warrants

(evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

Advancing Volume Threshold AV = 800
 Advancing Volume Va = 889
 If AV < Va then warrant is met

Right Turn Taper Warranted: **YES**

Northbound Left Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 879
 Advancing Volume Va = 0
 If AV < Va then warrant is met

Right Turn Lane Warranted: **NO**

Southbound Right Turn Taper Warrants

(evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

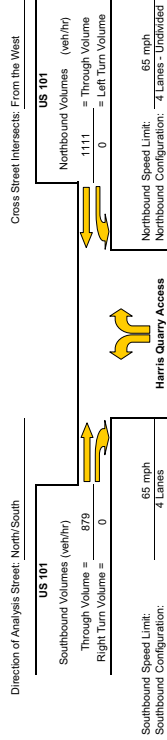
Thresholds not met, continue to next step

Advancing Volume Threshold AV = 1333.33333
 Advancing Volume Va = 879
 If AV < Va then warrant is met

Right Turn Taper Warranted: **NO**

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris, Quarry Access
 Study Scenario: 2014 with Willis Bypass, Base Conditions - July Peak, 4:45-5:45pm



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 879
 Advancing Volume Va = 0
 If AV < Va then warrant is met

Right Turn Lane Warranted: **NO**

Southbound Right Turn Taper Warrants

(evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

Advancing Volume Threshold AV = 1333.33333
 Advancing Volume Va = 879
 If AV < Va then warrant is met

Right Turn Taper Warranted: **NO**

Northbound Left Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 879
 Advancing Volume Va = 0
 If AV < Va then warrant is met

Right Turn Lane Warranted: **NO**

Southbound Right Turn Taper Warrants

(evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

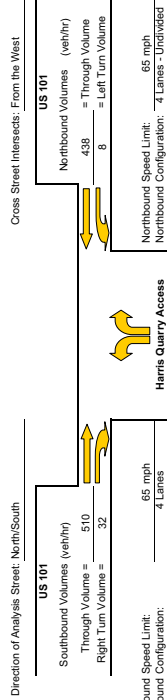
Advancing Volume Threshold AV = 1333.33333
 Advancing Volume Va = 879
 If AV < Va then warrant is met

Right Turn Taper Warranted: **NO**

Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.
 The right turn lane and taper analysis is based on work conducted by Cottel in 1981.
 The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2014 with Willis Bypass, Base Conditions - October Peak, 9-10am



Southbound Right Turn Lane Warrants
 1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 542
 Advancing Volume Va = 542
 If AV<Va then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants
 (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 266.666667
 Advancing Volume Va = 542
 If AV<Va then warrant is met Yes

Right Turn Taper Warranted: YES

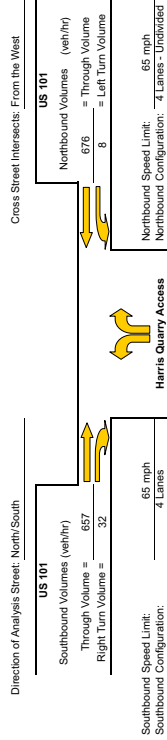
Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottel in 1981.

The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2014 with Willis Bypass, Base Conditions - October Peak, 11am-Noon



Southbound Right Turn Lane Warrants
 1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 689
 Advancing Volume Va = 689
 If AV<Va then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants
 (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 266.666667
 Advancing Volume Va = 689
 If AV<Va then warrant is met Yes

Right Turn Taper Warranted: YES

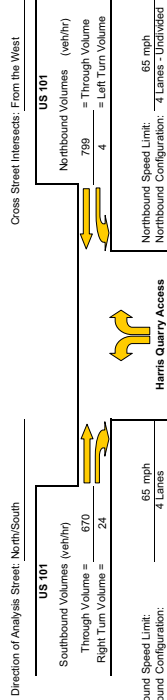
Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottel in 1981.

The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2014 with Willis Bypass, Base Conditions - October Peak, 11:15-2:15pm



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 694
 Advancing Volume Va = 24
 If AV > Va then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 533.33333
 Advancing Volume Va = 694
 If AV > Va then warrant is met

Right Turn Taper Warranted: YES

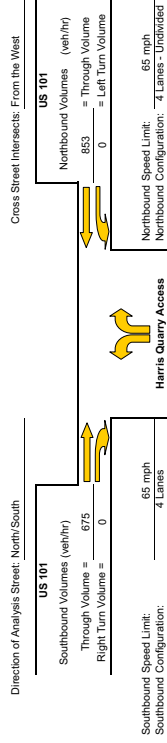
Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottel in 1981.

The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2014 with Willis Bypass, Base Conditions - October Peak, 4:45-5:45pm



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 675
 Advancing Volume Va = 0
 If AV > Va then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 1333.33333
 Advancing Volume Va = 675
 If AV > Va then warrant is met

Right Turn Taper Warranted: NO

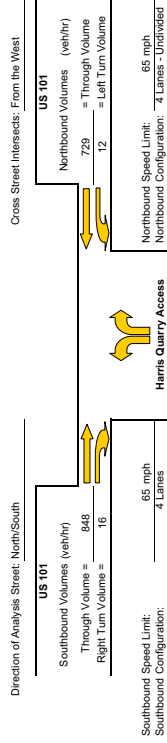
Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottel in 1981.

The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2030 Base Conditions - July Peak 9-10am



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = -
 Advancing Volume Va = 864
 If AV < Va then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants

(evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

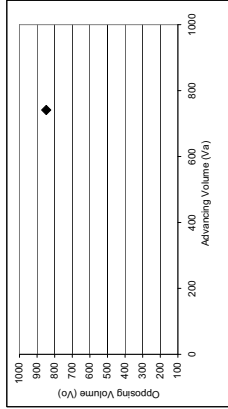
Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 800
 Advancing Volume Va = 864
 If AV < Va then warrant is met

Right Turn Taper Warranted: YES

Northbound Left Turn Lane Warrants

Left Turn Volume Threshold LIVol 10.9 veh/hr
 Left Turn Volume V = 12 veh/hr
 If V > LIVol then warrant is met



◆ Study Intersection

Left Turn Lane Warranted: YES

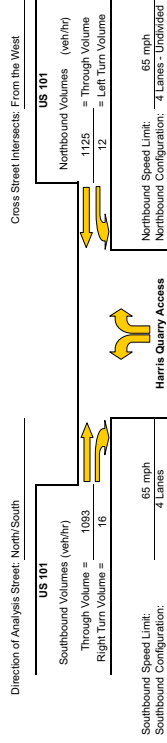
Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottel in 1981.

The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2030 Base Conditions - July Peak 11am-Noon



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = -
 Advancing Volume Va = 1109
 If AV < Va then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants

(evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

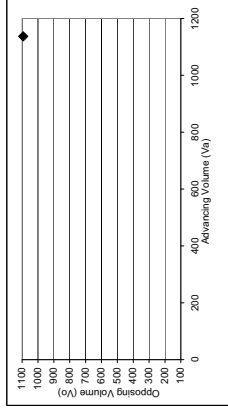
Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 800
 Advancing Volume Va = 1109
 If AV < Va then warrant is met

Right Turn Taper Warranted: YES

Northbound Left Turn Lane Warrants

Left Turn Volume Threshold LIVol 7.9 veh/hr
 Left Turn Volume V = 12 veh/hr
 If V > LIVol then warrant is met



◆ Study Intersection

Left Turn Lane Warranted: YES

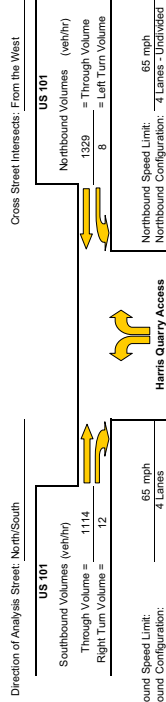
Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottel in 1981.

The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2030 Base Conditions - July Peak, 1:15-2:15pm



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = -
 Advancing Volume Va = 1126
 If AV < Va then warrant is met

Right Turn Lane Warranted: **NO**

Southbound Right Turn Taper Warrants

(evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 833.33333
 Advancing Volume Va = 1126
 If AV < Va then warrant is met Yes

Right Turn Taper Warranted: **YES**

Northbound Left Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 7.7
 Advancing Volume Va = 8
 If AV < Va then warrant is met

Right Turn Lane Warranted: **YES**

Northbound Left Turn Taper Warrants

(evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 1333.33333
 Advancing Volume Va = 1122
 If AV < Va then warrant is met No

Right Turn Taper Warranted: **NO**

Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = -
 Advancing Volume Va = 1122
 If AV < Va then warrant is met

Right Turn Lane Warranted: **NO**

Southbound Right Turn Taper Warrants

(evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 1333.33333
 Advancing Volume Va = 1122
 If AV < Va then warrant is met No

Right Turn Taper Warranted: **NO**

Northbound Left Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 7.6
 Advancing Volume Va = 0
 If AV < Va then warrant is met

Right Turn Lane Warranted: **NO**

Northbound Left Turn Taper Warrants

(evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

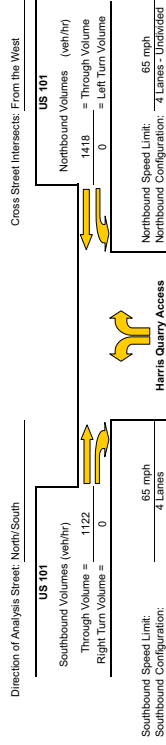
2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 1122
 Advancing Volume Va = 1122
 If AV < Va then warrant is met No

Right Turn Taper Warranted: **NO**

Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.
 The right turn lane and taper analysis is based on work conducted by Cottel in 1981.
 The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2030 Base Conditions - July Peak, 4:45-5:45pm



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = -
 Advancing Volume Va = 1122
 If AV < Va then warrant is met

Right Turn Lane Warranted: **NO**

Southbound Right Turn Taper Warrants

(evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 1333.33333
 Advancing Volume Va = 1122
 If AV < Va then warrant is met No

Right Turn Taper Warranted: **NO**

Northbound Left Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 7.6
 Advancing Volume Va = 0
 If AV < Va then warrant is met

Right Turn Lane Warranted: **NO**

Northbound Left Turn Taper Warrants

(evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

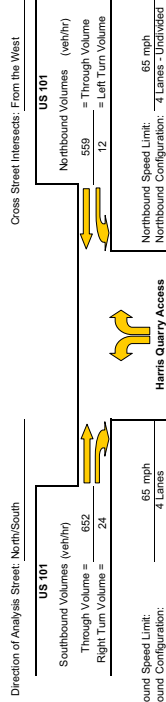
2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 1122
 Advancing Volume Va = 1122
 If AV < Va then warrant is met No

Right Turn Taper Warranted: **NO**

Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.
 The right turn lane and taper analysis is based on work conducted by Cottel in 1981.
 The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2030 Base Conditions - October Peak, 9-10am



Southbound Right Turn Lane Warrants
 1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = -
 Advancing Volume Va = 676
 If AV<Va then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants
 (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 533.333333
 Advancing Volume Va = 676
 If AV<Va then warrant is met
 Yes

Right Turn Taper Warranted: YES

Left Turn Lane Warranted: NO

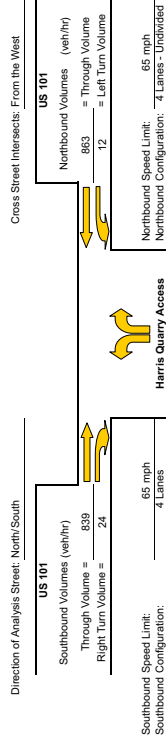
Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottel in 1981.

The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2030 Base Conditions - October Peak, 11am-Noon



Southbound Right Turn Lane Warrants
 1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = -
 Advancing Volume Va = 863
 If AV<Va then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants
 (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

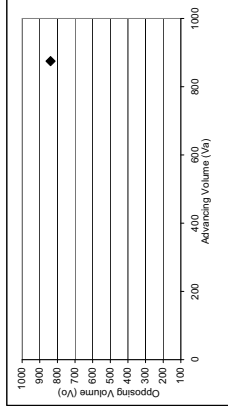
Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 533.333333
 Advancing Volume Va = 863
 If AV<Va then warrant is met
 Yes

Right Turn Taper Warranted: YES

Left Turn Lane Warranted: YES

Northbound Left Turn Lane Warrants
 Left Turn Volume Threshold LIVol = 11.1 veh/hr
 Left Turn Volume V = 12 veh/hr
 If V>LIVol then warrant is met



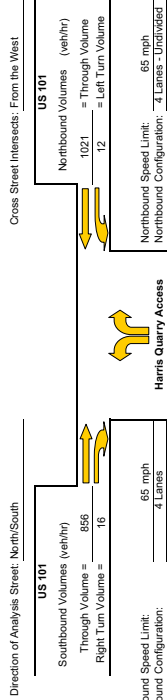
Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottel in 1981.

The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2030 Base Conditions - October Peak, 11:15-2:15pm



Southbound Right Turn Lane Warrants
 1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = -
 Advancing Volume Va = 872
 If AV < Va then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants
 (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 800
 Advancing Volume Va = 872
 If AV < Va then warrant is met

Right Turn Taper Warranted: YES

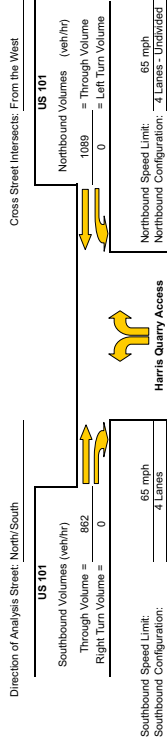
Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottel in 1981.

The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2030 Base Conditions - October Peak, 4:45-5:45pm



Southbound Right Turn Lane Warrants
 1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = -
 Advancing Volume Va = 862
 If AV < Va then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants
 (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 1333.33333
 Advancing Volume Va = 862
 If AV < Va then warrant is met

Right Turn Taper Warranted: NO

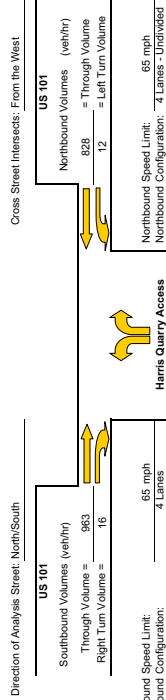
Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottel in 1981.

The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2042 Base Conditions - July Peak 9-10am



Southbound Right Turn Lane Warrants
 1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 979
 Advancing Volume Va = 979
 If AV < Va then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants
 (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

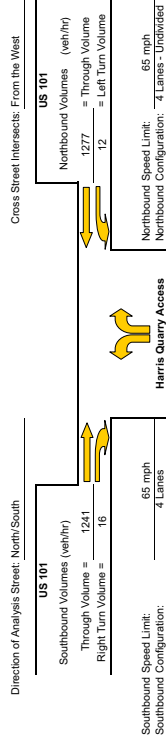
2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 800
 Advancing Volume Va = 979
 If AV < Va then warrant is met

Right Turn Taper Warranted: YES

Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.
 The right turn lane and taper analysis is based on work conducted by Cottel in 1981.
 The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2042 Base Conditions - July Peak 11am-Noon



Southbound Right Turn Lane Warrants
 1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 1257
 Advancing Volume Va = 1257
 If AV < Va then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants
 (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

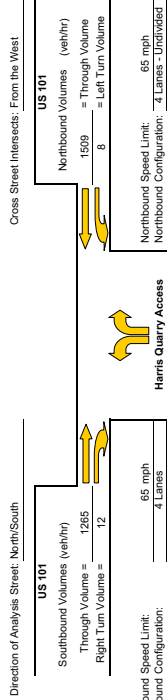
2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 800
 Advancing Volume Va = 1257
 If AV < Va then warrant is met

Right Turn Taper Warranted: YES

Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.
 The right turn lane and taper analysis is based on work conducted by Cottel in 1981.
 The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2040 Base Conditions - July Peak, 1:15-2:15pm



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 1277
 Advancing Volume Va = 12
 If AV > Va then warrant is met

Right Turn Lane Warranted: **NO**

Southbound Right Turn Taper Warrants

(evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 833.33333
 Advancing Volume Va = 1277
 If AV > Va then warrant is met Yes

Right Turn Taper Warranted: **YES**

Northbound Left Turn Lane Warrants

Left Turn Volume Threshold LIVol 6.3 veh/hr
 Left Turn Volume Vt 8 veh/hr
 If Vt > LIVol then warrant is met

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 1274
 Advancing Volume Va = 8
 If AV > Va then warrant is met

Right Turn Lane Warranted: **NO**

Southbound Right Turn Taper Warrants

(evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 1333.33333
 Advancing Volume Va = 1274
 If AV > Va then warrant is met Yes

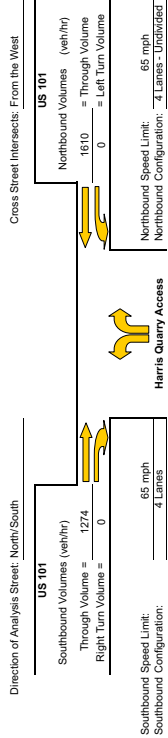
Right Turn Taper Warranted: **YES**

Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.
 The right turn lane and taper analysis is based on work conducted by Cottel in 1981.

The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2040 Base Conditions - July Peak, 4:45-5:45pm



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 1274
 Advancing Volume Va = 0
 If AV > Va then warrant is met

Right Turn Lane Warranted: **NO**

Southbound Right Turn Taper Warrants

(evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 1333.33333
 Advancing Volume Va = 1274
 If AV > Va then warrant is met No

Right Turn Taper Warranted: **NO**

Northbound Left Turn Lane Warrants

Left Turn Volume Threshold LIVol 6.2 veh/hr
 Left Turn Volume Vt 0 veh/hr
 If Vt > LIVol then warrant is met

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 1274
 Advancing Volume Va = 0
 If AV > Va then warrant is met

Right Turn Lane Warranted: **NO**

Southbound Right Turn Taper Warrants

(evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 1333.33333
 Advancing Volume Va = 1274
 If AV > Va then warrant is met No

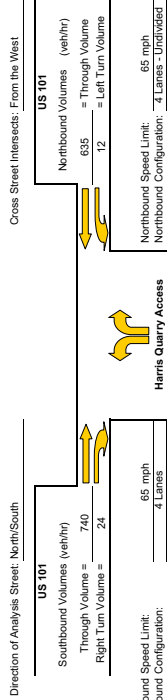
Right Turn Taper Warranted: **NO**

Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.
 The right turn lane and taper analysis is based on work conducted by Cottel in 1981.

The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2040 Base Conditions - October Peak, 9-10am



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 764
 Advancing Volume Va = -
 If AV < Va then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants

(evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 533.333333
 Advancing Volume Va = 764
 If AV < Va then warrant is met
 Yes

Right Turn Taper Warranted: YES

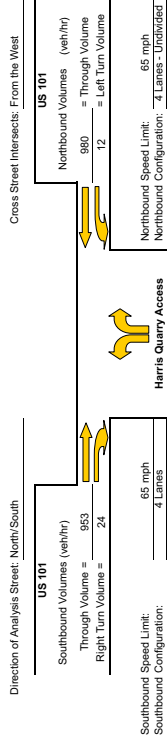
Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottel in 1981.

The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2040 Base Conditions - October Peak, 11am-Noon



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 977
 Advancing Volume Va = -
 If AV < Va then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants

(evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 533.333333
 Advancing Volume Va = 977
 If AV < Va then warrant is met
 Yes

Right Turn Taper Warranted: YES

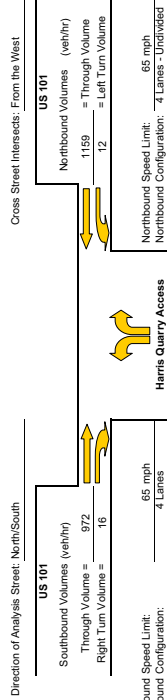
Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottel in 1981.

The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2040 Base Conditions - October Peak, 11:15-2:15pm



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 988
 Advancing Volume Va = -
 If AV/Va then warrant is met

Right Turn Lane Warranted: **NO**

Southbound Right Turn Taper Warrants

(evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

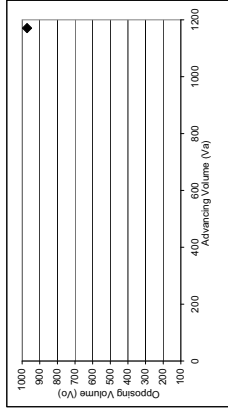
Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 800
 Advancing Volume Va = 988
 If AV/Va then warrant is met

Right Turn Taper Warranted: **YES**

Northbound Left Turn Lane Warrants

Left Turn Volume Threshold LIVol = 9.3 veh/hr
 Left Turn Volume Vt = 12 veh/hr
 If Vt > LIVol then warrant is met

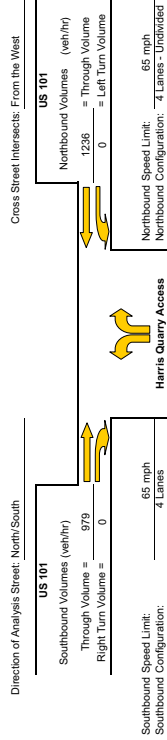


Study Intersection

Left Turn Lane Warranted: **YES**

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2040 Base Conditions - October Peak, 4:45-5:45pm



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 979
 Advancing Volume Va = -
 If AV/Va then warrant is met

Right Turn Lane Warranted: **NO**

Southbound Right Turn Taper Warrants

(evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

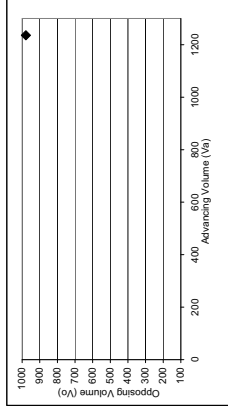
Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 1333.33333
 Advancing Volume Va = 979
 If AV/Va then warrant is met

Right Turn Taper Warranted: **NO**

Northbound Left Turn Lane Warrants

Left Turn Volume Threshold LIVol = 9.2 veh/hr
 Left Turn Volume Vt = 0 veh/hr
 If Vt > LIVol then warrant is met



Study Intersection

Left Turn Lane Warranted: **NO**

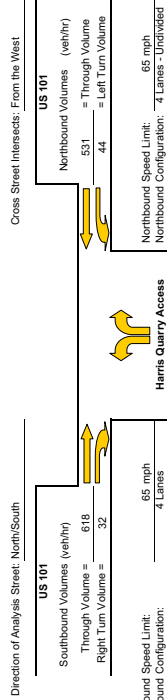
Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottel in 1981.

The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2010 Project Conditions - 300 Peak S-10am



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 650
 Advancing Volume Va = 650
 If AV<Va then warrant is met

Right Turn Lane Warranted: **NO**

Southbound Right Turn Taper Warrants

(evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 266.666667
 Advancing Volume Va = 650
 If AV<Va then warrant is met

Right Turn Taper Warranted: **YES**

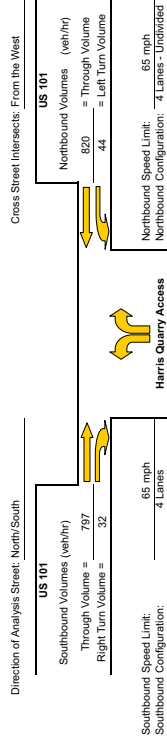
Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottel in 1981.

The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2010 Project Conditions - 300 Peak T-1am-Noon



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 650
 Advancing Volume Va = 650
 If AV<Va then warrant is met

Right Turn Lane Warranted: **NO**

Southbound Right Turn Taper Warrants

(evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 266.666667
 Advancing Volume Va = 829
 If AV<Va then warrant is met

Right Turn Taper Warranted: **YES**

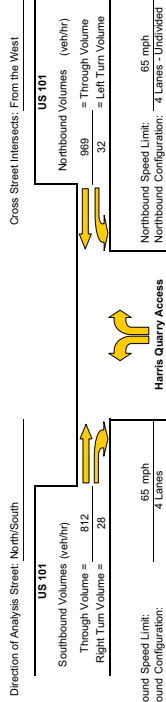
Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottel in 1981.

The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2010 Project Conditions - 300 Peak, 1:15-2:15pm



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 840
 Advancing Volume Va = 840
 If AV < Va then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants

(evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

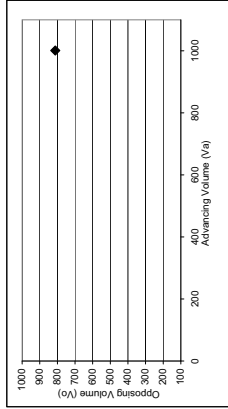
Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 400
 Advancing Volume Va = 840
 If AV < Va then warrant is met

Right Turn Taper Warranted: YES

Northbound Left Turn Lane Warrants

Left Turn Volume Threshold LIVol = 11.5 veh/hr
 Left Turn Volume V = 32 veh/hr
 If V > LIVol then warrant is met



◆ Study Intersection

Left Turn Lane Warranted: YES

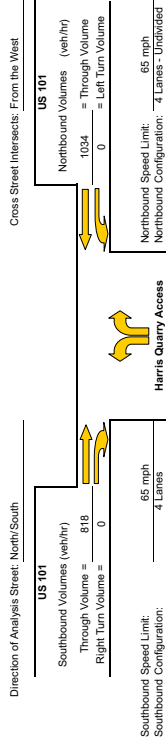
Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottel in 1981.

The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2010 Project Conditions - 300 Peak, 4:45-5:45pm



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 818
 Advancing Volume Va = 818
 If AV < Va then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants

(evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

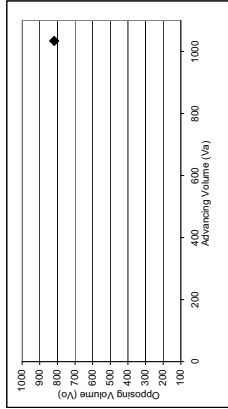
Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 1333.33333
 Advancing Volume Va = 818
 If AV < Va then warrant is met

Right Turn Taper Warranted: NO

Northbound Left Turn Lane Warrants

Left Turn Volume Threshold LIVol = 11.4 veh/hr
 Left Turn Volume V = 0 veh/hr
 If V > LIVol then warrant is met



◆ Study Intersection

Left Turn Lane Warranted: NO

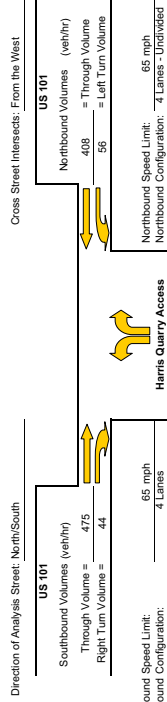
Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottel in 1981.

The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2010 Project Conditions - October Peak, 9-10am



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 1144
 Advancing Volume Va = 519
 If AV/Va then warrant is met No

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

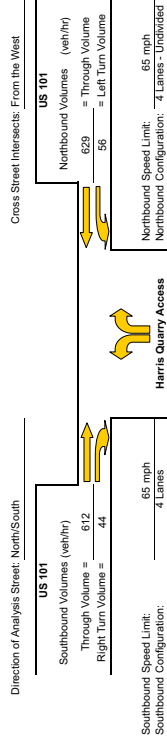
2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = -133.33333
 Advancing Volume Va = 519
 If AV/Va then warrant is met Yes

Right Turn Taper Warranted: YES

Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.
 The right turn lane and taper analysis is based on work conducted by Cottel in 1981.
 The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2010 Project Conditions - October Peak, 11am-Noon



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 1144
 Advancing Volume Va = 656
 If AV/Va then warrant is met No

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

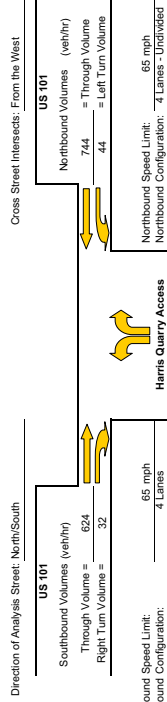
2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = -133.33333
 Advancing Volume Va = 656
 If AV/Va then warrant is met Yes

Right Turn Taper Warranted: YES

Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.
 The right turn lane and taper analysis is based on work conducted by Cottel in 1981.
 The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2010 Project Conditions - October Peak, 1:15-2:15pm



Southbound Right Turn Lane Warrants
 Left Turn Volume Threshold LTVol = 14.7 veh/hr
 Left Turn Volume Vt = 44 veh/hr
 If Vt > LTVol then warrant is met

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = -
 Advancing Volume Va = 656
 If AV < Va then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants
 (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria
Thresholds not met, continue to next step

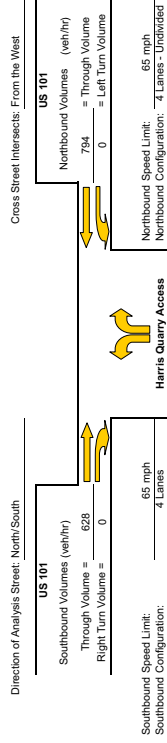
2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 266.666667
 Advancing Volume Va = 656
 If AV < Va then warrant is met
 Yes

Right Turn Taper Warranted: YES

Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.
 The right turn lane and taper analysis is based on work conducted by Cottel in 1981.
 The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2010 Project Conditions - October Peak, 4:45-5:45pm



Southbound Right Turn Lane Warrants
 Left Turn Volume Threshold LTVol = 14.6 veh/hr
 Left Turn Volume Vt = 0 veh/hr
 If Vt > LTVol then warrant is met

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = -
 Advancing Volume Va = 628
 If AV < Va then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants
 (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria
Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 1333.33333
 Advancing Volume Va = 628
 If AV < Va then warrant is met
 No

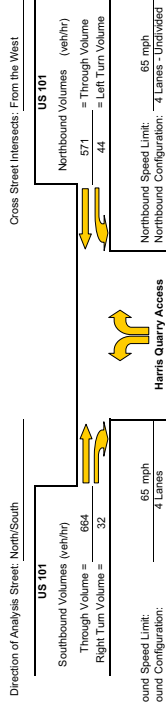
Right Turn Taper Warranted: NO

Left Turn Lane Warranted: NO

Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.
 The right turn lane and taper analysis is based on work conducted by Cottel in 1981.
 The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101 Harris Quarry Access
Study Scenario: 2014 without Willis Bypass, Project Conditions - July Peak 9-10am



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
Advancing Volume Threshold $AV = 696$
Advancing Volume $Va = 696$
If $AV < Va$ then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
Advancing Volume Threshold $AV = 266.666667$
Advancing Volume $Va = 696$
If $AV < Va$ then warrant is met

Right Turn Taper Warranted: YES

Northbound Left Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
Advancing Volume Threshold $AV = 888$
Advancing Volume $Va = 888$
If $AV < Va$ then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

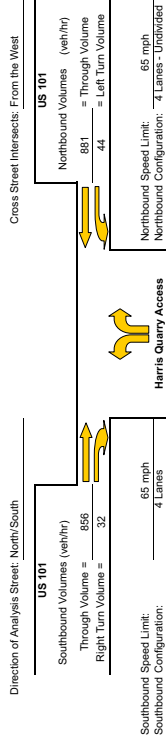
2. Check advance volume threshold criteria for taper
Advancing Volume Threshold $AV = 266.666667$
Advancing Volume $Va = 888$
If $AV < Va$ then warrant is met

Right Turn Taper Warranted: YES

Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.
The right turn lane and taper analysis is based on work conducted by Cottel in 1981.
The left turn lane analysis is based on work conducted by M.D. Hamelink in 1967, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101 Harris Quarry Access
Study Scenario: 2014 without Willis Bypass, Project Conditions - July Peak 11am-Noon



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
Advancing Volume Threshold $AV = 888$
Advancing Volume $Va = 888$
If $AV < Va$ then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
Advancing Volume Threshold $AV = 266.666667$
Advancing Volume $Va = 888$
If $AV < Va$ then warrant is met

Right Turn Taper Warranted: YES

Northbound Left Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
Advancing Volume Threshold $AV = 1088$
Advancing Volume $Va = 1088$
If $AV < Va$ then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

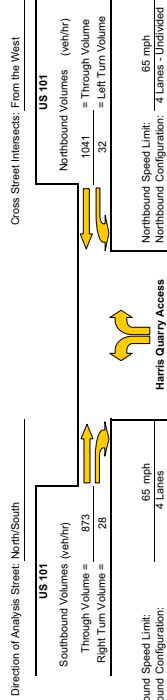
2. Check advance volume threshold criteria for taper
Advancing Volume Threshold $AV = 266.666667$
Advancing Volume $Va = 888$
If $AV < Va$ then warrant is met

Right Turn Taper Warranted: YES

Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.
The right turn lane and taper analysis is based on work conducted by Cottel in 1981.
The left turn lane analysis is based on work conducted by M.D. Hamelink in 1967, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101 Harris Quarry Access
 Study Scenario: 2014 without W/HA Bypass, Project Conditions - July Peak, 11:15-2:15pm



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 901
 Advancing Volume Va = -
 If AV < Va then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants

(evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 400
 Advancing Volume Va = 901
 If AV < Va then warrant is met

Right Turn Taper Warranted: YES

Northbound Left Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 879
 Advancing Volume Va = -
 If AV < Va then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants

(evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 1333.33333
 Advancing Volume Va = 879
 If AV < Va then warrant is met

Right Turn Taper Warranted: YES

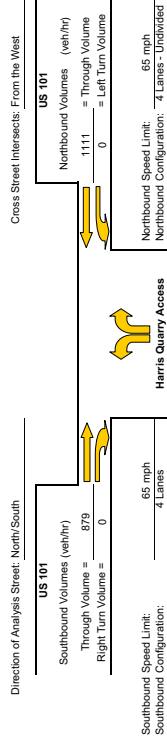
Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottel in 1981.

The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101 Harris Quarry Access
 Study Scenario: 2014 without W/HA Bypass, Project Conditions - July Peak, 4:45-5:45pm



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = -
 Advancing Volume Va = -
 If AV < Va then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants

(evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 1333.33333
 Advancing Volume Va = 879
 If AV < Va then warrant is met

Right Turn Taper Warranted: NO

Northbound Left Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 10.5
 Advancing Volume Va = 0
 If AV < Va then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants

(evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 10.5
 Advancing Volume Va = 0
 If AV < Va then warrant is met

Right Turn Taper Warranted: NO

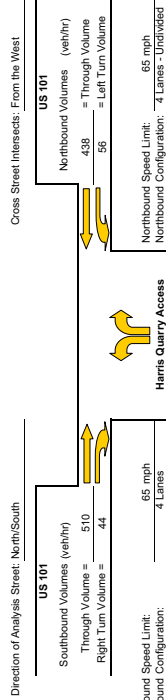
Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottel in 1981.

The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101 Harris Quarry Access
 Study Scenario: 2014 without Willis Bypass, Project Conditions - October Peak, P-Turn



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 1144
 Advancing Volume VA = 554
 If AV/VA then warrant is met No

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = -133.33333
 Advancing Volume VA = 554
 If AV/VA then warrant is met Yes

Right Turn Taper Warranted: YES

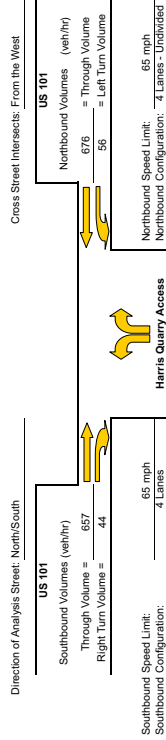
Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottell in 1981.

The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101 Harris Quarry Access
 Study Scenario: 2014 without Willis Bypass, Project Conditions - October Peak, 11am-Noon



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 1144
 Advancing Volume VA = 701
 If AV/VA then warrant is met No

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = -133.33333
 Advancing Volume VA = 701
 If AV/VA then warrant is met Yes

Right Turn Taper Warranted: YES

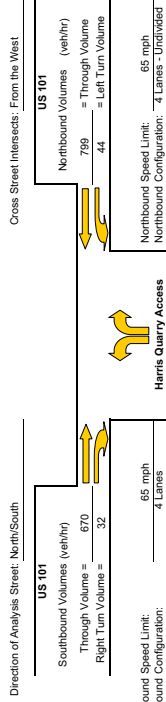
Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottell in 1981.

The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101 Harris, Quarry Access
 Study Scenario: 2014, without Willis Bypass, Project Conditions - October Peak, 1:15-2:15pm



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 702
 Advancing Volume Va = -
 If AV < Va then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants

(evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

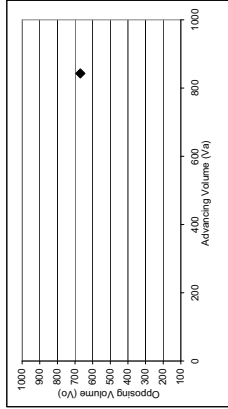
Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 266.666667
 Advancing Volume Va = 702
 If AV < Va then warrant is met Yes

Right Turn Taper Warranted: YES

Northbound Left Turn Lane Warrants

Left Turn Volume Threshold LTVol 13.8 veh/hr
 Left Turn Volume Vt 44 veh/hr
 If Vt > LTVol then warrant is met



Study Intersection

Left Turn Lane Warranted: YES

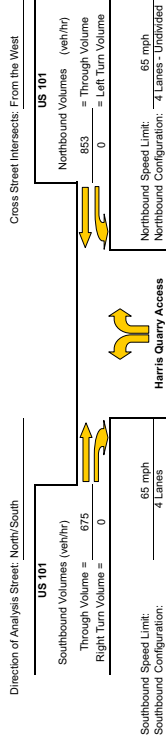
Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottel in 1981.

The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101 Harris, Quarry Access
 Study Scenario: 2014, without Willis Bypass, Project Conditions - October Peak, 4:45-5:45pm



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = -
 Advancing Volume Va = 675
 If AV < Va then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants

(evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

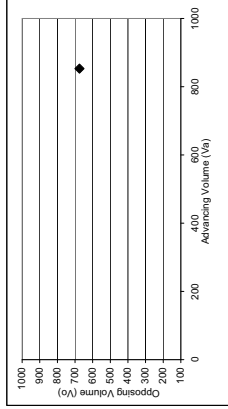
Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 1333.33333
 Advancing Volume Va = 675
 If AV < Va then warrant is met No

Right Turn Taper Warranted: NO

Northbound Left Turn Lane Warrants

Left Turn Volume Threshold LTVol 13.7 veh/hr
 Left Turn Volume Vt 0 veh/hr
 If Vt > LTVol then warrant is met



Study Intersection

Left Turn Lane Warranted: NO

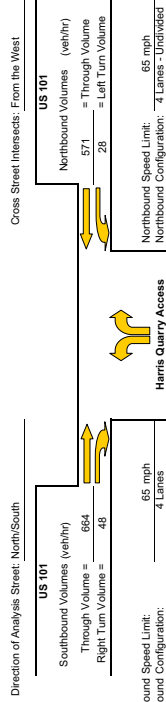
Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottel in 1981.

The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2014 with WHTA Bypass, Project Conditions - July Peak, 9:10am



Southbound Right Turn Lane Warrants
 1. Check for right turn volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 1088
 Advancing Volume Va = 712
 If AV > Va then warrant is met
 No

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants
 (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = -266.66667
 Advancing Volume Va = 712
 If AV < Va then warrant is met
 Yes

Right Turn Taper Warranted: YES

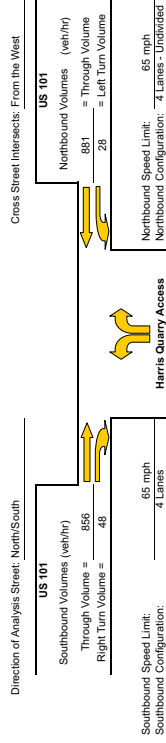
Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottell in 1981.

The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2014 with WHTA Bypass, Project Conditions - July Peak, 11am-Noon



Southbound Right Turn Lane Warrants
 1. Check for right turn volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 1088
 Advancing Volume Va = 904
 If AV > Va then warrant is met
 No

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants
 (evaluate if right turn lane is unwarranted)

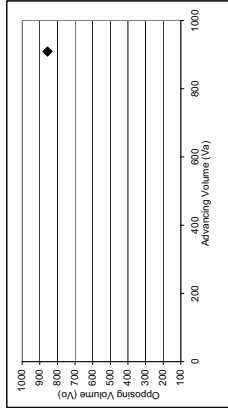
1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = -266.66667
 Advancing Volume Va = 904
 If AV < Va then warrant is met
 Yes

Right Turn Taper Warranted: YES

Northbound Left Turn Lane Warrants
 Left Turn Volume Threshold LTVol = 10.8 veh/hr
 Left Turn Volume Vt = 28 veh/hr
 If Vt > LTVol then warrant is met



◆ Study Intersection

Left Turn Lane Warranted: YES

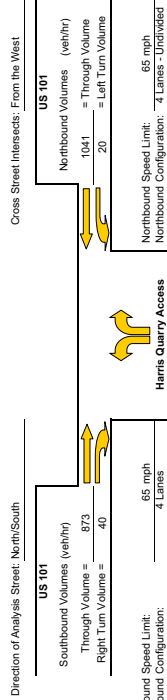
Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottell in 1981.

The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2014 with Willis Bypass, Project Conditions - July Peak, 1:15-2:15pm



Southbound Right Turn Lane Warrants
 1. Check for right turn volume criteria

Thresholds not met, continue to next step

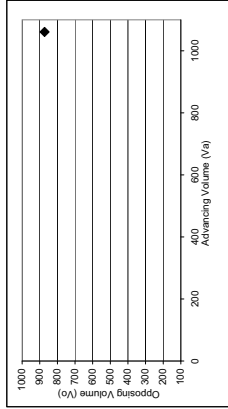
2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 1200
 Advancing Volume Va = 913
 If AV > Va then warrant is met No
 Right Turn Lane Warranted: **NO**

Southbound Right Turn Taper Warrants
 (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria
Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 0
 Advancing Volume Va = 913
 If AV > Va then warrant is met Yes
 Right Turn Taper Warranted: **YES**

Left Turn Volume Threshold LTVol 10.6 veh/hr
 Left Turn Volume Vt 20 veh/hr
 If Vt > LTVol then warrant is met



Study Intersection

Left Turn Lane Warranted: **YES**

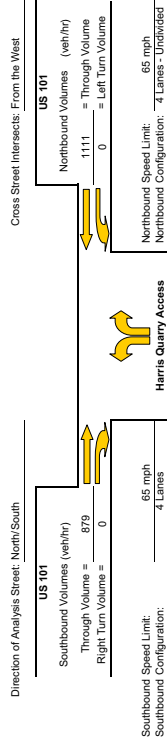
Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottel in 1981.

The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2014 with Willis Bypass, Project Conditions - July Peak, 4:45-5:45pm



Southbound Right Turn Lane Warrants
 1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

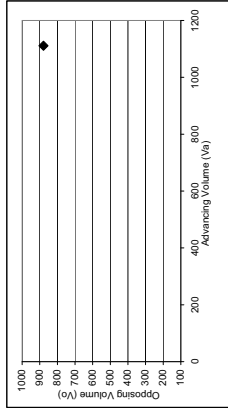
2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = -
 Advancing Volume Va = -
 If AV > Va then warrant is met
 Right Turn Lane Warranted: **NO**

Southbound Right Turn Taper Warrants
 (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria
Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 1333.33333
 Advancing Volume Va = 879
 If AV > Va then warrant is met No
 Right Turn Taper Warranted: **NO**

Left Turn Volume Threshold LTVol 10.5 veh/hr
 Left Turn Volume Vt 0 veh/hr
 If Vt > LTVol then warrant is met



Study Intersection

Left Turn Lane Warranted: **NO**

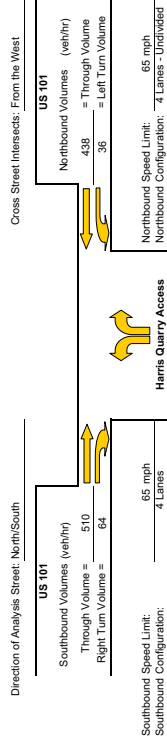
Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottel in 1981.

The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2014 with Willis Bypass, Project Conditions - October Peak, 9-10am



Southbound Right Turn Lane Warrants
 1. Check for right turn volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 863.9
 Advancing Volume VA = 574
 If AV < VA then warrant is met No

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants
 (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

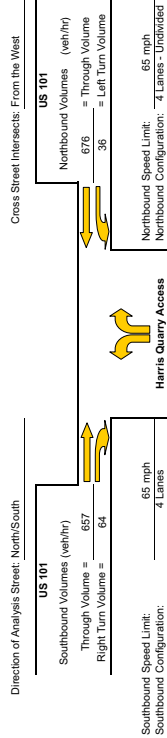
2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 800
 Advancing Volume VA = 574
 If AV < VA then warrant is met Yes

Right Turn Taper Warranted: YES

Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.
 The right turn lane and taper analysis is based on work conducted by Cottel in 1981.
 The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2014 with Willis Bypass, Project Conditions - October Peak, 11am-12pm



Southbound Right Turn Lane Warrants
 1. Check for right turn volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 863.9
 Advancing Volume VA = 721
 If AV < VA then warrant is met No

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants
 (evaluate if right turn lane is unwarranted)

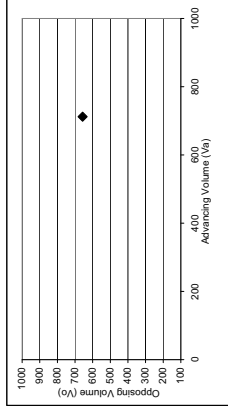
1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 800
 Advancing Volume VA = 721
 If AV < VA then warrant is met Yes

Right Turn Taper Warranted: YES

Northbound Left Turn Lane Warrants
 Left Turn Volume Threshold LTVol 14.1 veh/hr
 Left Turn Volume VL 36 veh/hr
 If VL > LTVol then warrant is met



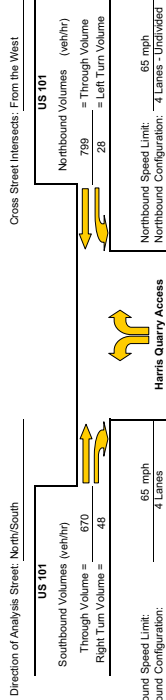
Study Intersection

Left Turn Lane Warranted: YES

Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.
 The right turn lane and taper analysis is based on work conducted by Cottel in 1981.
 The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2014 with Willis Bypass, Project Conditions - October Peak, 1:15-2:15pm



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 1088
 Advancing Volume Va = 718
 If AV/Va then warrant is met No

Right Turn Lane Warranted: **NO**

Southbound Right Turn Taper Warrants

(evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = -266.66667
 Advancing Volume Va = 718
 If AV/Va then warrant is met Yes

Right Turn Taper Warranted: **YES**

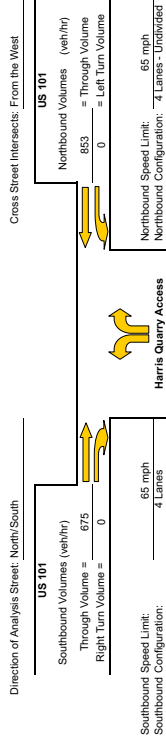
Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottel in 1981.

The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2014 with Willis Bypass, Project Conditions - October Peak, 4:45-5:45pm



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = -
 Advancing Volume Va = 675
 If AV/Va then warrant is met

Right Turn Lane Warranted: **NO**

Southbound Right Turn Taper Warrants

(evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 1333.33333
 Advancing Volume Va = 675
 If AV/Va then warrant is met No

Right Turn Taper Warranted: **NO**

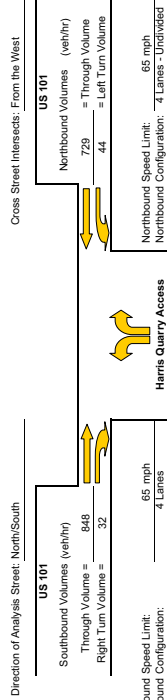
Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottel in 1981.

The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2030 Project Conditions - AM Peak 5-10am



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = -
 Advancing Volume Va = 880
 If AV<Va then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 266.666667
 Advancing Volume Va = 880
 If AV<Va then warrant is met Yes

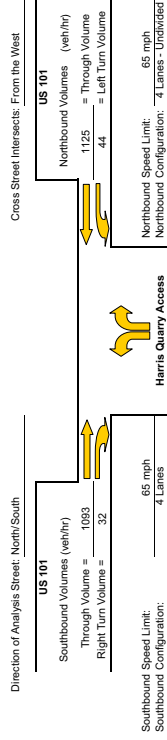
Right Turn Taper Warranted: YES

Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottel in 1981. The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2030 Project Conditions - AM Peak 11am-Noon



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = -
 Advancing Volume Va = 1125
 If AV<Va then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 266.666667
 Advancing Volume Va = 1125
 If AV<Va then warrant is met Yes

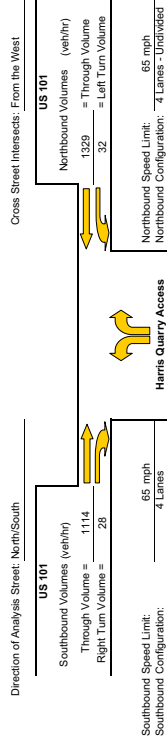
Right Turn Taper Warranted: YES

Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottel in 1981. The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2030 Project Conditions - 207 Peak, 1:15-2:15pm



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = -
 Advancing Volume Va = 1142
 If AV<Va then warrant is met

Right Turn Lane Warranted: **NO**

Southbound Right Turn Taper Warrants

(evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

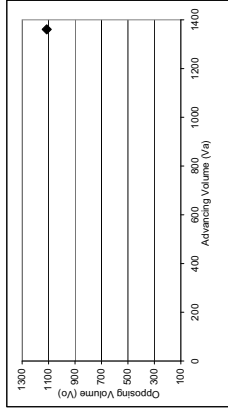
Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 400
 Advancing Volume Va = 1142
 If AV<Va then warrant is met

Right Turn Taper Warranted: **YES**

Northbound Left Turn Lane Warrants

Left Turn Volume Threshold LIVol = 7.7 veh/hr
 Left Turn Volume Vt = 32 veh/hr
 If Vt>LIVol then warrant is met



Study Intersection

Left Turn Lane Warranted: **YES**

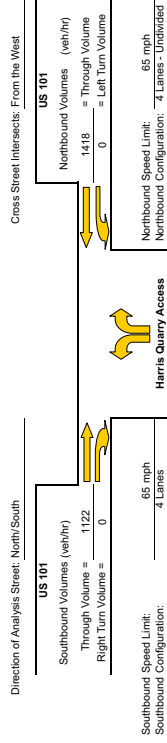
Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottel in 1981.

The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2030 Project Conditions - 207 Peak, 4:45-5:45pm



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = -
 Advancing Volume Va = 1122
 If AV<Va then warrant is met

Right Turn Lane Warranted: **NO**

Southbound Right Turn Taper Warrants

(evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

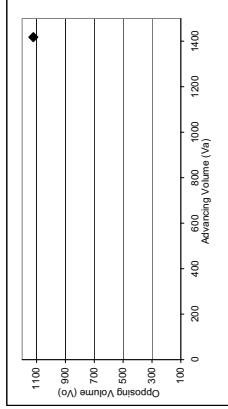
Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 1333.33333
 Advancing Volume Va = 1122
 If AV<Va then warrant is met

Right Turn Taper Warranted: **NO**

Northbound Left Turn Lane Warrants

Left Turn Volume Threshold LIVol = 7.6 veh/hr
 Left Turn Volume Vt = 0 veh/hr
 If Vt>LIVol then warrant is met



Study Intersection

Left Turn Lane Warranted: **NO**

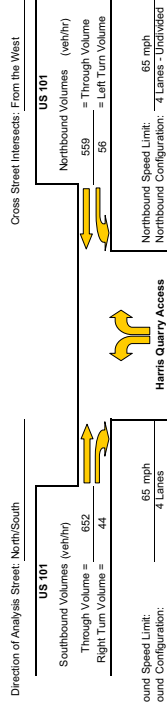
Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottel in 1981.

The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2030 Project Conditions - October Peak, 9-10am



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 1144
 Advancing Volume Va = 696
 If AV/Va then warrant is met No

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants
 (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = -133.33333
 Advancing Volume Va = 696
 If AV/Va then warrant is met Yes

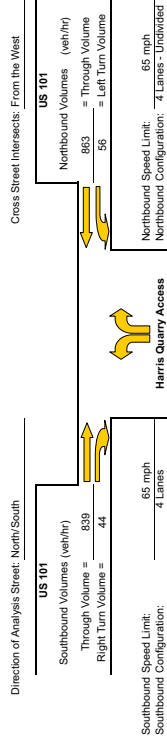
Right Turn Taper Warranted: YES

Left Turn Lane Warranted: YES

Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.
 The right turn lane and taper analysis is based on work conducted by Cottel in 1981.
 The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2030 Project Conditions - October Peak, 11am-Noon



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 1144
 Advancing Volume Va = 883
 If AV/Va then warrant is met No

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants
 (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

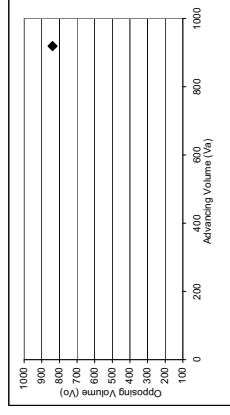
2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = -133.33333
 Advancing Volume Va = 883
 If AV/Va then warrant is met Yes

Right Turn Taper Warranted: YES

Left Turn Lane Warranted: YES

Northbound Left Turn Lane Warrants

Left Turn Volume Threshold LIVol 11.1 veh/hr
 Left Turn Volume V = 56 veh/hr
 If V>LIVol then warrant is met

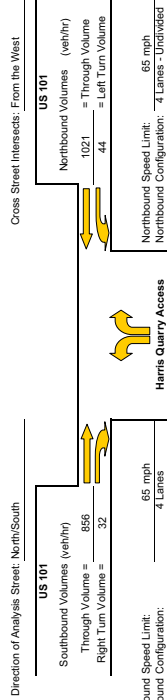


◆ Study Intersection

Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.
 The right turn lane and taper analysis is based on work conducted by Cottel in 1981.
 The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2030 Project Conditions - October Peak, 1:15-2:15pm



Southbound Right Turn Lane Warrants
 1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = -
 Advancing Volume Va = 888
 If AV/Va then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants
 (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 266.666667
 Advancing Volume Va = 888
 If AV/Va then warrant is met Yes

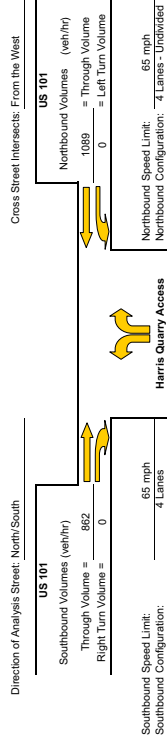
Right Turn Taper Warranted: YES

Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottel in 1981.
 The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2030 Project Conditions - October Peak, 4:45-5:45pm



Southbound Right Turn Lane Warrants
 1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = -
 Advancing Volume Va = 862
 If AV/Va then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants
 (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 1333.33333
 Advancing Volume Va = 862
 If AV/Va then warrant is met No

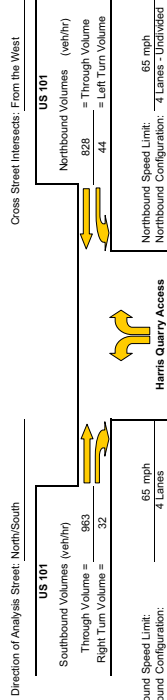
Right Turn Taper Warranted: NO

Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottel in 1981.
 The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2040 Project Conditions - 300 Peak S-10am



Southbound Right Turn Lane Warrants
 Left Turn Volume Threshold L/Vol 9.4 veh/hr
 Left Turn Volume Vt = 44 veh/hr
 If $V_t > L/Vol$ then warrant is met

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 995
 $V_a = 32$
 If $AV < V_a$ then warrant is met

Right Turn Lane Warranted: **NO**

Southbound Right Turn Taper Warrants
 (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria
Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 266.666667
 $V_a = 995$
 If $AV < V_a$ then warrant is met
 Yes

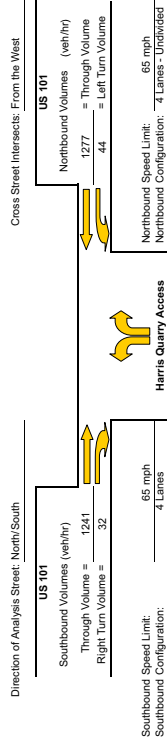
Right Turn Taper Warranted: **YES**

Left Turn Lane Warranted: **YES**

Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.
 The right turn lane and taper analysis is based on work conducted by Cottel in 1981.
 The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2040 Project Conditions - 300 Peak T-11am-Noon



Southbound Right Turn Lane Warrants
 Left Turn Volume Threshold L/Vol 6.5 veh/hr
 Left Turn Volume Vt = 44 veh/hr
 If $V_t > L/Vol$ then warrant is met

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 1273
 $V_a = 32$
 If $AV < V_a$ then warrant is met

Right Turn Lane Warranted: **NO**

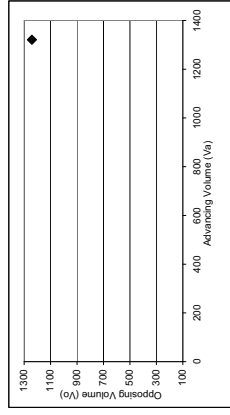
Southbound Right Turn Taper Warrants
 (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria
Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 266.666667
 $V_a = 1273$
 If $AV < V_a$ then warrant is met
 Yes

Right Turn Taper Warranted: **YES**

Left Turn Lane Warranted: **YES**

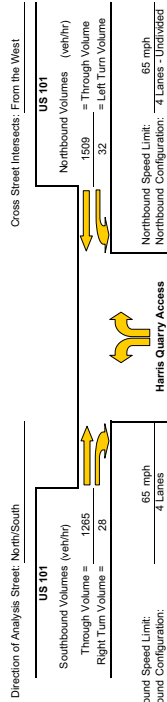


Study Intersection

Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.
 The right turn lane and taper analysis is based on work conducted by Cottel in 1981.
 The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2040 Project Conditions - 207 Peak, 1:15-2:15pm



Southbound Right Turn Lane Warrants
 1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = -
 Va = 1283
 If AV<Va then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants
 (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 400
 Va = 1283
 If AV<Va then warrant is met

Right Turn Taper Warranted: YES

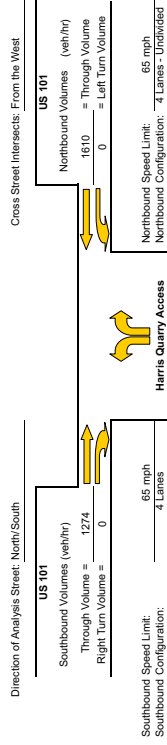
Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottel in 1981.

The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2040 Project Conditions - 207 Peak, 4:45-5:45pm



Southbound Right Turn Lane Warrants
 1. Check for right turn volume criteria

NOT WARRANTED - Less than 40 vehicles

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = -
 Va = 1274
 If AV<Va then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants
 (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 1333.33333
 Va = 1274
 If AV<Va then warrant is met

Right Turn Taper Warranted: NO

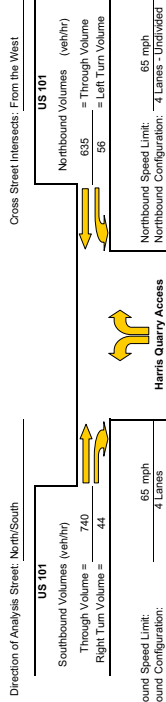
Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.

The right turn lane and taper analysis is based on work conducted by Cottel in 1981.

The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2040 Project Conditions - October Peak, 9-10am



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 1144
 Advancing Volume Va = 784
 If AV/Va then warrant is met No

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = -133.33333
 Advancing Volume Va = 784
 If AV/Va then warrant is met Yes

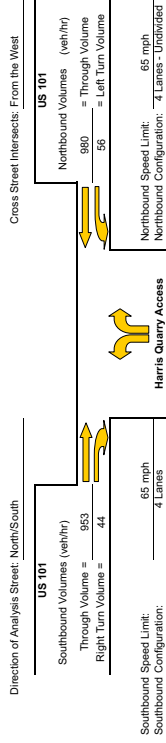
Right Turn Taper Warranted: YES

Left Turn Lane Warranted: YES

Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.
 The right turn lane and taper analysis is based on work conducted by Cottel in 1981.
 The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2040 Project Conditions - October Peak, 11am-Noon



Southbound Right Turn Lane Warrants

1. Check for right turn volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 1144
 Advancing Volume Va = 997
 If AV/Va then warrant is met No

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = -133.33333
 Advancing Volume Va = 997
 If AV/Va then warrant is met Yes

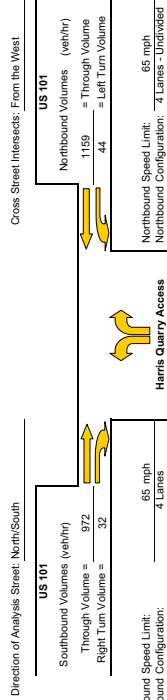
Right Turn Taper Warranted: YES

Left Turn Lane Warranted: YES

Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.
 The right turn lane and taper analysis is based on work conducted by Cottel in 1981.
 The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2040 Project Conditions - October Peak, 1:15-2:15pm



Southbound Right Turn Lane Warrants
 Left Turn Volume Threshold LTVol = 9.3 veh/hr
 Left Turn Volume Vt = 44

NOT WARRANTED - Less than 40 vehicles

1. Check for right turn volume criteria

Right Turn Lane Warranted: NO

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 1004
 Advancing Volume Va = 1004
 If AV/Va then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants
 (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 266.666667
 Advancing Volume Va = 1004
 If AV/Va then warrant is met

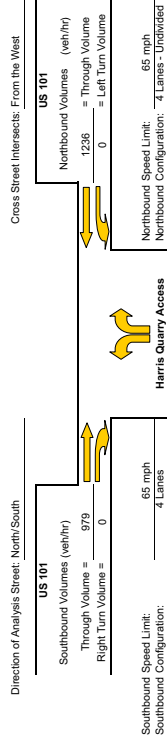
Right Turn Taper Warranted: YES

Left Turn Lane Warranted: YES

Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.
 The right turn lane and taper analysis is based on work conducted by Cottel in 1981.
 The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Turn Lane Warrant Analysis - Tee Intersections

Study Intersection: US 101/Harris Quarry Access
 Study Scenario: 2040 Project Conditions - October Peak, 4:45-5:45pm



Southbound Right Turn Lane Warrants
 Left Turn Volume Threshold LTVol = 9.2 veh/hr
 Left Turn Volume Vt = 0

NOT WARRANTED - Less than 40 vehicles

1. Check for right turn volume criteria

Right Turn Lane Warranted: NO

2. Check advance volume threshold criteria for turn lane
 Advancing Volume Threshold AV = 979
 Advancing Volume Va = 979
 If AV/Va then warrant is met

Right Turn Lane Warranted: NO

Southbound Right Turn Taper Warrants
 (evaluate if right turn lane is unwarranted)

1. Check taper volume criteria

Thresholds not met, continue to next step

2. Check advance volume threshold criteria for taper
 Advancing Volume Threshold AV = 1333.33333
 Advancing Volume Va = 979
 If AV/Va then warrant is met

Right Turn Taper Warranted: NO

Left Turn Lane Warranted: NO

Methodology based on Washington State Transportation Center Research Report Method For Prioritizing Intersection Improvements, January 1997.
 The right turn lane and taper analysis is based on work conducted by Cottel in 1981.
 The left turn lane analysis is based on work conducted by M.D. Hamelink in 1987, and modified by Kikuchi and Chakraborty in 1991.

Appendix H

Intersection Level of Service Calculations

9-10am Peak Hour - Base 2010 July Peak (Existing Lanes)
Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart

Average Delay (sec/veh): 0.5 Worst Case Level of Service: C [15.8]

Street Name: US 101
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
Rights: Include Include Include Include
Lanes: 1 0 2 0 0 0 2 0 1 0 0 1 0 0 0 0 0 0 0

Volume Module:
Base Vol: 9 539 0 0 622 7 18 0 12 0 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 9 539 0 0 622 7 18 0 12 0 0
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 9 539 0 0 622 7 18 0 12 0 0
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0
FinalVolume: 9 539 0 0 622 7 18 0 12 0 0

Critical Gap Module:
Critical Gap: 4.1 xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx
FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx

Capacity Module:
Conflict Vol: 629 xxxxx xxxxx 910 1179 311
Potent Cap.: 963 xxxxx xxxxx 278 192 691
Move Cap.: 963 xxxxx xxxxx 278 190 691
Volume/Cap: 0.01 xxxxx xxxxx 0.07 0.00 0.02

Level of Service Module:
2Way95thQ: 0.0 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Control Del: 8.8 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
LOS by Move: A * * * * *
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT
Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
SharedQueue: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Shrd Condel: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Shared LOS: * * * * * C
ApproachDel: xxxxxx 15.8
ApproachLOS: * * C

Note: Queue reported is the number of cars per lane.

9-10am Peak Hour - Base 2010 July Peak (Existing Lanes)
Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 US 101/Quarry Access

Average Delay (sec/veh): 0.5 Worst Case Level of Service: C [15.6]

Street Name: US 101
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
Rights: Include Include Include Include
Lanes: 0 1 1 0 0 0 0 1 1 0 0 0 1 0 0 0 0 0 0

Volume Module:
Base Vol: 12 531 0 0 618 16 16 0 12 0 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 12 531 0 0 618 16 16 0 12 0 0
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 12 531 0 0 618 16 16 0 12 0 0
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0
FinalVolume: 12 531 0 0 618 16 16 0 12 0 0

Critical Gap Module:
Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx
FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx

Capacity Module:
Conflict Vol: 634 xxxxx xxxxx 916 1181 317
Potent Cap.: 959 xxxxx xxxxx 276 192 685
Move Cap.: 959 xxxxx xxxxx 273 189 685
Volume/Cap: 0.01 xxxxx xxxxx 0.06 0.00 0.02

Level of Service Module:
2Way95thQ: 0.0 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Control Del: 8.8 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
LOS by Move: A * * * * *
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT
Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
SharedQueue: 0.0 xxxxx xxxxx xxxxx xxxxx xxxxx 0.2 xxxxx xxxxx
Shrd Condel: 8.8 xxxxx xxxxx xxxxx xxxxx xxxxx 15.6 xxxxx xxxxx
Shared LOS: * * * * * C
ApproachDel: xxxxxx 15.6
ApproachLOS: * * C

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart

Average Delay (sec/veh): 0.4 Worst Case Level Of Service: C [19.9]

Street Name: US 101 Black Bart

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Uncontrolled	Uncontrolled	Stop Sign	Stop Sign
Rights:	Include	Include	Include	Include
Lanes:	1 0 2 0 0	0 0 2 0 1	0 0 1 1 0	0 0 0 0 0

Volume Module:

Base Vol:	19 1019	0 800	20 11	0 18
Growth Adj:	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
Initial Bse:	19 1019	0 800	20 11	0 18
User Adj:	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
PHF Adj:	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
PHF Volume:	19 1019	0 800	20 11	0 18
Reduct Vol:	0 0 0 0	0 0 0 0	0 0 0 0	0 0 0 0
FinalVolume:	19 1019	0 800	20 11	0 18

Critical Gap Module:

Critical Gap:	4.1	xxxx	xxxx	xxxx	xxxx	6.8	6.5	6.9	xxxx	xxxx	xxxx
FollowUpTim:	2.2	xxxx	xxxx	xxxx	xxxx	3.5	4.0	3.3	xxxx	xxxx	xxxx

Capacity Module:

Conflict Vol:	820	xxxx	xxxx	xxxx	xxxx	1348	1857	400	xxxx	xxxx	xxxx
Potent Cap.:	818	xxxx	xxxx	xxxx	xxxx	145	74	605	xxxx	xxxx	xxxx
Move Cap.:	818	xxxx	xxxx	xxxx	xxxx	142	73	605	xxxx	xxxx	xxxx
Volume/Cap:	0.02	xxxx	xxxx	xxxx	xxxx	0.08	0.00	0.03	xxxx	xxxx	xxxx

Level Of Service Module:

2Way95thQ:	0.1	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx
Control Del:	9.5	xxxx	xxxx	xxxx	xxxx	xxxx	xxx	xxxx	xxxx	xxxx	xxxx
LOS by Move:	A	*	*	*	*	*	*	*	*	*	*
Movement:	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT
Shared Cap.:	xxxx	xxxx	xxxx	xxxx	xxxx	271	xxxx	xxxx	xxxx	xxxx	xxxx
SharedQueue:	xxxx	xxxx	xxxx	xxxx	xxxx	0.4	xxxx	xxxx	xxxx	xxxx	xxxx
Shrd Condel:	xxxx	xxxx	xxxx	xxxx	xxxx	19.9	xxxx	xxxx	xxxx	xxxx	xxxx
Shared LOS:	*	*	*	*	*	C	*	*	*	*	*
ApproachDel:	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	19.9	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx
ApproachLOS:	*	*	*	*	*	C	*	*	*	*	*

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 US 101/Quarry Access

Average Delay (sec/veh): 0.1 Worst Case Level Of Service: C [20.7]

Street Name: US 101 Quarry Access

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Uncontrolled	Uncontrolled	Stop Sign	Stop Sign
Rights:	Include	Include	Include	Include
Lanes:	0 1 1 0 0	0 0 1 1 0	0 0 1 1 0	0 0 0 0 0

Volume Module:

Base Vol:	0 1034	0 818	0 4	0 4
Growth Adj:	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
Initial Bse:	0 1034	0 818	0 4	0 4
User Adj:	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
PHF Adj:	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
PHF Volume:	0 1034	0 818	0 4	0 4
Reduct Vol:	0 0 0 0	0 0 0 0	0 0 0 0	0 0 0 0
FinalVolume:	0 1034	0 818	0 4	0 4

Critical Gap Module:

Critical Gap:	xxxx	xxxx	xxxx	xxxx	6.8	6.5	6.9	xxxx	xxxx	xxxx
FollowUpTim:	xxxx	xxxx	xxxx	xxxx	3.5	4.0	3.3	xxxx	xxxx	xxxx

Capacity Module:

Conflict Vol:	xxxx	xxxx	xxxx	xxxx	1335	1852	409	xxxx	xxxx	xxxx
Potent Cap.:	xxxx	xxxx	xxxx	xxxx	148	75	597	xxxx	xxxx	xxxx
Move Cap.:	xxxx	xxxx	xxxx	xxxx	148	75	597	xxxx	xxxx	xxxx
Volume/Cap:	xxxx	xxxx	xxxx	xxxx	0.03	0.00	0.01	xxxx	xxxx	xxxx

Level Of Service Module:

2Way95thQ:	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx
Control Del:	xxxx	xxxx	xxxx	xxxx	xxxx	xxx	xxxx	xxxx	xxxx	xxxx
LOS by Move:	*	*	*	*	*	*	*	*	*	*
Movement:	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT
Shared Cap.:	xxxx	xxxx	xxxx	xxxx	xxxx	237	xxxx	xxxx	xxxx	xxxx
SharedQueue:	0.0	xxxx	xxxx	xxxx	xxxx	0.1	xxxx	xxxx	xxxx	xxxx
Shrd Condel:	9.0	xxxx	xxxx	xxxx	xxxx	20.7	xxxx	xxxx	xxxx	xxxx
Shared LOS:	A	*	*	*	*	C	*	*	*	*
ApproachDel:	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	20.7	xxxxxx	xxxxxx	xxxxxx	xxxxxx
ApproachLOS:	*	*	*	*	*	C	*	*	*	*

Note: Queue reported is the number of cars per lane.

11-12pm Peak Hour - Base 2014 w/ Willits Bypass July Peak (Existing Lanes)
Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)
Intersection #1 US 101/Black Bart

Average Delay (sec/veh): 0.8 Worst Case Level of Service: D [31.8]

Table with columns: Street Name, Approach, Movement, Control, Rights, Lanes, Volume Module, Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Volume/Cap, Level of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap, SharedQueue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

11-12pm Peak Hour - Base 2014 w/ Willits Bypass July Peak (Existing Lanes)
Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)
Intersection #2 US 101/Quarry Access

Average Delay (sec/veh): 0.5 Worst Case Level of Service: D [30.5]

Table with columns: Street Name, Approach, Movement, Control, Rights, Lanes, Volume Module, Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Volume/Cap, Level of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap, SharedQueue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

1:15-2:15pm Peak Hour - Base 2014 w/o Willits Bypass October Peak (Existing Lan
Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart
Average Delay (sec/veh): 0.7 Worst Case Level of Service: C [22.8]

Table with columns: Street Name, Approach, Movement, Control, Rights, Lanes, Volume Module, Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Volume/Cap, Level of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap, SharedQueue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS.

1:15-2:15pm Peak Hour - Base 2014 w/o Willits Bypass October Peak (Existing Lan
Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart
Average Delay (sec/veh): 0.7 Worst Case Level of Service: C [22.8]

Table with columns: Street Name, Approach, Movement, Control, Rights, Lanes, Volume Module, Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Volume/Cap, Level of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap, SharedQueue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS.

1:15-2:15pm Peak Hour - Base 2014 w/o Willits Bypass October Peak (Existing Lan
Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 US 101/Quarry Access
Average Delay (sec/veh): 0.4 Worst Case Level of Service: C [18.6]

Table with columns: Street Name, Approach, Movement, Control, Rights, Lanes, Volume Module, Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Volume/Cap, Level of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap, SharedQueue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS.

4:45-5:45pm Peak Hour - Base 2040 October Peak (Existing Lanes)
 Harris Quarry EIR
 County of Mendocino

Level of Service Computation Report
 2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart
 Average Delay (sec/veh): 0.7 Worst Case Level of Service: D [29.6]

Street Name: US 101 Black Bart

Approach:	N	S	E	W
Movement:	L-T-R	L-T-R	L-T-R	L-T-R
Control:	Uncontrolled	Uncontrolled	Uncontrolled	Uncontrolled
Rights:	Uncontrolled	Uncontrolled	Uncontrolled	Uncontrolled
Lanes:	1 0 2 0 0	0 0 2 0 1	0 0 1 0 0	0 0 0 0 0

Volume Module:

Base Vol:	29 1211	0	0 952	31	17	0	27	0	0	0
Growth Adj:	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
Initial Bse:	29 1211	0	0 952	31	17	0	27	0	0	0
User Adj:	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
PHF Adj:	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
PHF Volume:	29 1211	0	0 952	31	17	0	27	0	0	0
Reduce Vol:	0	0	0	0	0	0	0	0	0	0
FinalVolume:	29 1211	0	0 952	31	17	0	27	0	0	0

Critical Gap Module:

Critical Gap:	4.1 xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	6.8	6.5	6.9	xxxxx	xxxxx
FollowUpTim:	2.2 xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	3.5	4.0	3.3	xxxxx	xxxxx

Capacity Module:

Conflict Vol:	983	xxxxx	xxxxx	xxxxx	xxxxx	1616	2221	476	xxxxx	xxxxx
Potent Cap.:	711	xxxxx	xxxxx	xxxxx	xxxxx	96	44	541	xxxxx	xxxxx
Move Cap.:	711	xxxxx	xxxxx	xxxxx	xxxxx	93	42	541	xxxxx	xxxxx
Volume/Cap:	0.04	xxxxx	xxxxx	xxxxx	xxxxx	0.18	0.00	0.05	xxxxx	xxxxx

Level of Service Module:

2Way95thQ:	0.1	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx
Control Del:	10.3	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx
LOS by Move:	B	*	*	*	*	*	*	*	*	*
Movement:	L-T-R	L-T-R	L-T-R	L-T-R	L-T-R	L-T-R	L-T-R	L-T-R	L-T-R	L-T-R
Shared Cap.:	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	190	xxxxx	xxxxx	xxxxx	xxxxx
SharedQueue:	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	0.9	xxxxx	xxxxx	xxxxx	xxxxx
Shrd ConDel:	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	29.6	xxxxx	xxxxx	xxxxx	xxxxx
Shared LOS:	A	*	*	*	*	D	*	*	*	*
ApproachDel:	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	29.6	xxxxxx	xxxxxx	xxxxxx	xxxxxx
ApproachLOS:	*	*	*	*	*	D	*	*	*	*

Note: Queue reported is the number of cars per lane.

4:45-5:45pm Peak Hour - Base 2040 October Peak (Existing Lanes)
 Harris Quarry EIR
 County of Mendocino

Level of Service Computation Report
 2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 US 101/Quarry Access
 Average Delay (sec/veh): 0.1 Worst Case Level of Service: D [27.6]

Street Name: US 101 Quarry Access

Approach:	N	S	E	W
Movement:	L-T-R	L-T-R	L-T-R	L-T-R
Control:	Uncontrolled	Uncontrolled	Uncontrolled	Uncontrolled
Rights:	Uncontrolled	Uncontrolled	Uncontrolled	Uncontrolled
Lanes:	0 1 1 0 0	0 0 1 1 0	0 0 1 1 0	0 0 0 0 0

Volume Module:

Base Vol:	0	1236	0	0	979	0	4	0	4	0
Growth Adj:	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
Initial Bse:	0	1236	0	0	979	0	4	0	4	0
User Adj:	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
PHF Adj:	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
PHF Volume:	0	1236	0	0	979	0	4	0	4	0
Reduce Vol:	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	1236	0	0	979	0	4	0	4	0

Critical Gap Module:

Critical Gap:	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	6.8	6.5	6.9	xxxxx	xxxxx
FollowUpTim:	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	3.5	4.0	3.3	xxxxx	xxxxx

Capacity Module:

Conflict Vol:	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	1597	2215	490	xxxxx	xxxxx
Potent Cap.:	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	99	44	530	xxxxx	xxxxx
Move Cap.:	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	99	44	530	xxxxx	xxxxx
Volume/Cap:	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	0.04	0.00	0.01	xxxxx	xxxxx

Level of Service Module:

2Way95thQ:	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx
Control Del:	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx
LOS by Move:	*	*	*	*	*	*	*	*	*	*
Movement:	L-T-R	L-T-R	L-T-R	L-T-R	L-T-R	L-T-R	L-T-R	L-T-R	L-T-R	L-T-R
Shared Cap.:	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	167	xxxxx	xxxxx	xxxxx	xxxxx
SharedQueue:	0.0	xxxxx	xxxxx	xxxxx	xxxxx	0.1	xxxxx	xxxxx	xxxxx	xxxxx
Shrd ConDel:	9.0	xxxxx	xxxxx	xxxxx	xxxxx	27.6	xxxxx	xxxxx	xxxxx	xxxxx
Shared LOS:	A	*	*	*	*	D	*	*	*	*
ApproachDel:	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	27.6	xxxxxx	xxxxxx	xxxxxx	xxxxxx
ApproachLOS:	*	*	*	*	*	D	*	*	*	*

Note: Queue reported is the number of cars per lane.

9-10am Peak Hour - Base 2010 July Peak (Mitigated Lanes)
Harris Quarry EIR
County of Mendocino

Level Of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart

Average Delay (sec/veh): 0.4 Worst Case Level Of Service: B [13.1]

Street Name: US 101
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
Rights: Include Include Include Include
Lanes: 1 0 2 0 0 0 2 0 1 0 0 1 0 0 0 0 0 0

Volume Module:
Base Vol: 9 538 0 0 622 7 18 0 12 0 0 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 9 538 0 0 622 7 18 0 12 0 0
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 9 538 0 0 622 7 18 0 12 0 0
Reduct Vol: 0 0 0 0 0 0 0 0 0 0
FinalVolume: 9 538 0 0 622 7 18 0 12 0 0

Critical Gap Module:
Critical Gap: 4.1 xxxx xxxxx xxxx xxxx 6.8 6.5 6.9 xxxxx xxx xxxx
FollowUpTim: 2.2 xxxx xxxxx xxxxx xxxx xxxxx 3.5 4.0 3.3 xxxxx xxx xxxx

Capacity Module:
Conflict Vol: 629 xxxxx xxxxx xxxx xxxx xxxxx 909 1178 311 xxxxx xxxxx xxxxx
Potential Cap: 963 xxxxx xxxxx xxxx xxxx xxxxx 278 192 691 xxxxx xxxxx xxxxx
Move Cap: 963 xxxxx xxxxx xxxx xxxx xxxxx 276 191 691 xxxxx xxxxx xxxxx
Total Cap: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 391 313 xxxxx 361 309 xxxxx
Volume/Cap: 0.01 xxxxx xxxxx xxxxx xxxxx xxxxx 0.05 0.00 0.02 xxxxx xxxxx xxxxx

Level Of Service Module:
2Way95thQ: 0.0 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Control Del: 8.8 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
LOS by Move: A *
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
SharedQueue: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Shrd Condel: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Shared LOS: *
ApproachDel: xxxxxx 13.1 xxxxxx
ApproachLOS: *

Note: Queue reported is the number of cars per lane.

9-10am Peak Hour - Base 2010 July Peak (Mitigated Lanes)
Harris Quarry EIR
County of Mendocino

Level Of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 US 101/Quarry Access

Average Delay (sec/veh): 0.4 Worst Case Level Of Service: B [13.0]

Street Name: US 101
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
Rights: Include Include Include Include
Lanes: 1 0 2 0 0 0 0 1 1 0 0 0 1 1 0 0 0 0 0 0

Volume Module:
Base Vol: 12 531 0 0 618 16 16 0 12 0 0 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 12 531 0 0 618 16 16 0 12 0 0
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 12 531 0 0 618 16 16 0 12 0 0
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
FinalVolume: 12 531 0 0 618 16 16 0 12 0 0

Critical Gap Module:
Critical Gap: 4.1 xxxx xxxxx xxxxx xxxx xxxxx 6.8 6.5 6.9 xxxxx xxx xxxx
FollowUpTim: 2.2 xxxx xxxxx xxxxx xxxx xxxxx 3.5 4.0 3.3 xxxxx xxx xxxx

Capacity Module:
Conflict Vol: 634 xxxxx xxxxx xxxxx xxxxx xxxxx 916 1181 317 xxxxx xxxxx xxxxx
Potential Cap: 959 xxxxx xxxxx xxxx xxxx xxxxx 276 192 685 xxxxx xxxxx xxxxx
Move Cap: 959 xxxxx xxxxx xxxx xxxx xxxxx 273 189 685 xxxxx xxxxx xxxxx
Total Cap: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 388 312 xxxxx 361 307 xxxxx
Volume/Cap: 0.01 xxxxx xxxxx xxxxx xxxxx xxxxx 0.04 0.00 0.02 xxxxx xxxxx xxxxx

Level Of Service Module:
2Way95thQ: 0.0 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Control Del: 8.8 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
LOS by Move: A *
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
SharedQueue: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Shrd Condel: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Shared LOS: *
ApproachDel: xxxxxx 13.0 xxxxxx
ApproachLOS: *

Note: Queue reported is the number of cars per lane.

11-12pm Peak Hour - Base 2010 July Peak (Mitigated Lanes)

Harris Quarry EIR

County of Mendocino

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart

Average Delay (sec/veh): 0.4 Worst Case Level Of Service: C[17.0]

Street Name: US 101

Approach: North Bound South Bound East Bound West Bound

Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Uncontrolled Uncontrolled

Rights: Include Include Include Include

Lanes: 1 0 2 0 0 0 2 0 1 0 0 1 0 0 0 0 0 0

Volume Module:

Table with columns: Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume. Rows include US 101, North Bound, South Bound, East Bound, West Bound.

Critical Gap Module:

Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx xxxxx

FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx xxxxx

Capacity Module:

Conflict Vol: 828 xxxxx xxxxx xxxxx xxxxx 1243 1654 402 xxxxx xxxxx xxxxx

Potent Cap.: 812 xxxxx xxxxx xxxxx xxxxx 169 99 604 xxxxx xxxxx xxxxx

Move Cap.: 812 xxxxx xxxxx xxxxx xxxxx 167 98 604 xxxxx xxxxx xxxxx

Total Cap: xxxxx xxxxx xxxxx xxxxx xxxxx 294 220 xxxxx 240 214 xxxxx

Volume/Cap: 0.02 xxxxx xxxxx xxxxx xxxxx 0.10 0.00 0.01 xxxxx xxxxx xxxxx

Level Of Service Module:

2Way95thQ: 0.1 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx

Control Del: 9.5 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx

LOS by Move: A *

Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT

Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx 336 xxxxx xxxxx xxxxx xxxxx

SharedQueue: xxxxx xxxxx xxxxx xxxxx xxxxx 0.4 xxxxx xxxxx xxxxx xxxxx

Shrd ConDel: xxxxx xxxxx xxxxx xxxxx xxxxx 17.0 xxxxx xxxxx xxxxx xxxxx

Shared LOS: *

ApproachDel: xxxxxx xxxxxx 17.0 xxxxxx

ApproachLOS: *

Note: Queue reported is the number of cars per lane.

11-12pm Peak Hour - Base 2010 July Peak (Mitigated Lanes)

Harris Quarry EIR

County of Mendocino

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 US 101/Quarry Access

Average Delay (sec/veh): 0.3 Worst Case Level Of Service: C[15.3]

Street Name: US 101

Approach: North Bound South Bound East Bound West Bound

Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Uncontrolled Uncontrolled

Rights: Include Include Include Include

Lanes: 1 0 2 0 0 0 2 0 1 0 0 0 1 0 0 0 0 0

Volume Module:

Table with columns: Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume. Rows include US 101, North Bound, South Bound, East Bound, West Bound.

Critical Gap Module:

Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx xxxxx

FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx xxxxx

Capacity Module:

Conflict Vol: 813 xxxxx xxxxx xxxxx xxxxx 1239 1649 407 xxxxx xxxxx xxxxx

Potent Cap.: 823 xxxxx xxxxx xxxxx xxxxx 170 100 600 xxxxx xxxxx xxxxx

Move Cap.: 823 xxxxx xxxxx xxxxx xxxxx 169 99 600 xxxxx xxxxx xxxxx

Total Cap: xxxxx xxxxx xxxxx xxxxx xxxxx 295 222 xxxxx 242 218 xxxxx

Volume/Cap: 0.01 xxxxx xxxxx xxxxx xxxxx 0.05 0.00 0.02 xxxxx xxxxx xxxxx

Level Of Service Module:

2Way95thQ: 0.0 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx

Control Del: 9.4 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx

LOS by Move: A *

Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT

Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx 377 xxxxx xxxxx xxxxx xxxxx

SharedQueue: xxxxx xxxxx xxxxx xxxxx xxxxx 0.2 xxxxx xxxxx xxxxx xxxxx

Shrd ConDel: xxxxx xxxxx xxxxx xxxxx xxxxx 15.3 xxxxx xxxxx xxxxx xxxxx

Shared LOS: *

ApproachDel: xxxxxx xxxxxx 15.3 xxxxxx

ApproachLOS: *

Note: Queue reported is the number of cars per lane.

1:15-2:15pm Peak Hour - Base 2010 July Peak (Mitigated Lanes)

Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart
Average Delay (sec/veh): 0.3 Worst Case Level of Service: B [14.4]

Table with columns: Street Name (US 101), Approach (North, South, East, West), Movement (L, T, R), and Control (Uncontrolled, Stop Sign, etc.).

Volume Module: Base Vol: 18 963, Growth Adj: 1.00, Initial Bse: 18 963, User Adj: 1.00, PHF Volume: 18 963, Reduct Vol: 0, Final Volume: 18 963

Critical Gap Module: Critical Gap: 4.1, FollowUpTim: 2.2

Capacity Module: Conflict Vol: 829, Potent Cap.: 811, Move Cap.: 811, Total Cap.: xxxxxx, Volume/Cap.: 0.02

Level of Service Module: 2Way95thQ: 0.1, Control Del: 9.5, LOS by Move: A, Movement: LT-LTR-RT, Shared Queue: xxxxxx, Shrd ConDel: xxxxxx, Shared LOS: B, ApproachDel: xxxxxx, ApproachLOS: B

Note: Queue reported is the number of cars per lane.

1:15-2:15pm Peak Hour - Base 2010 July Peak (Mitigated Lanes)

Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 US 101/Quarry Access
Average Delay (sec/veh): 0.2 Worst Case Level of Service: C [15.7]

Table with columns: Street Name (US 101), Approach (North, South, East, West), Movement (L, T, R), and Control (Uncontrolled, Stop Sign, etc.).

Volume Module: Base Vol: 8 969, Growth Adj: 1.00, Initial Bse: 8 969, User Adj: 1.00, PHF Volume: 8 969, Reduct Vol: 0, Final Volume: 8 969

Critical Gap Module: Critical Gap: 4.1, FollowUpTim: 2.2

Capacity Module: Conflict Vol: 824, Potent Cap.: 815, Move Cap.: 815, Total Cap.: xxxxxx, Volume/Cap.: 0.01

Level of Service Module: 2Way95thQ: 0.0, Control Del: 9.5, LOS by Move: A, Movement: LT-LTR-RT, Shared Queue: xxxxxx, Shrd ConDel: xxxxxx, Shared LOS: C, ApproachDel: xxxxxx, ApproachLOS: C

Note: Queue reported is the number of cars per lane.

9-10am Peak Hour - Base 2010 October Peak (Mitigated Lanes)
 Harris Quarry EIR
 County of Mendocino

Level of Service Computation Report
 2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 US 101/Quarry Access
 Worst Case Level of Service: B [12.2]

Average Delay (sec/veh): 0.6

Street Name:	US 101	Quarry Access
Approach:	North Bound	South Bound
Movement:	L - T - R	L - T - R
	L - T - R	L - T - R
Control:	Uncontrolled	Uncontrolled
Rights:	Uncontrolled	Uncontrolled
Lanes:	1 0 2 0 0	0 0 1 1 0

Volume Module:

Base Vol:	12 408	0	475	24	0	12	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	12 408	0	475	24	0	12	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	12 408	0	475	24	0	12	0
Reduct Vol:	0	0	0	0	0	0	0
FinalVolume:	12 408	0	475	24	0	12	0

Critical Gap Module:

Critical Gap:	4.1	6.9	6.5	6.9
FollowUpTim:	2.2	3.5	4.0	3.3

Capacity Module:

Conflict Vol:	499	715	919	250
Potent Cap.:	1075	370	273	757
Move Cap.:	1075	367	270	757
Total Cap.:	xxxx	467	382	442
Volume/Cap.:	0.01	0.05	0.00	0.02

Level of Service Module:

2Way95thQ:	0.0	xxxx	xxxx	xxxx
Control Del:	8.4	xxxx	xxxx	xxxx
LOS by Move:	A	*	*	*
Movement:	LT - LTR	RT - LTR	RT - LTR	RT - LTR
Shared Cap.:	xxxx	xxxx	xxxx	xxxx
SharedQueue:	xxxx	xxxx	xxxx	xxxx
Shrd ConDel:	xxxx	xxxx	xxxx	xxxx
Shared LOS:	*	*	*	*
ApproachDel:	xxxxxx	xxxxxx	12.2	xxxxxx
ApproachLOS:	*	B	B	*

Note: Queue reported is the number of cars per lane.

9-10am Peak Hour - Base 2010 October Peak (Mitigated Lanes)
 Harris Quarry EIR
 County of Mendocino

Level of Service Computation Report
 2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart
 Worst Case Level of Service: B [11.9]

Average Delay (sec/veh): 0.5

Street Name:	US 101	Black Bart
Approach:	North Bound	South Bound
Movement:	L - T - R	L - T - R
	L - T - R	L - T - R
Control:	Uncontrolled	Uncontrolled
Rights:	Uncontrolled	Uncontrolled
Lanes:	1 0 2 0 0	0 0 1 1 0

Volume Module:

Base Vol:	9 423	0	487	7	18	0	12
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	9 423	0	487	7	18	0	12
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	9 423	0	487	7	18	0	12
Reduct Vol:	0	0	0	0	0	0	0
FinalVolume:	9 423	0	487	7	18	0	12

Critical Gap Module:

Critical Gap:	4.1	6.5	6.9	6.9
FollowUpTim:	2.2	3.5	4.0	3.3

Capacity Module:

Conflict Vol:	494	717	928	244
Potent Cap.:	1080	369	270	763
Move Cap.:	1080	367	268	763
Total Cap.:	xxxx	467	380	436
Volume/Cap.:	0.01	0.04	0.00	0.02

Level of Service Module:

2Way95thQ:	0.0	xxxx	xxxx	xxxx
Control Del:	8.4	xxxx	xxxx	xxxx
LOS by Move:	A	*	*	*
Movement:	LT - LTR	RT - LTR	RT - LTR	RT - LTR
Shared Cap.:	xxxx	xxxx	xxxx	xxxx
SharedQueue:	xxxx	xxxx	xxxx	xxxx
Shrd ConDel:	xxxx	xxxx	xxxx	xxxx
Shared LOS:	*	*	*	*
ApproachDel:	xxxxxx	xxxxxx	11.9	xxxxxx
ApproachLOS:	*	B	B	*

Note: Queue reported is the number of cars per lane.

11-12pm Peak Hour - Base 2010 October Peak (Mitigated Lanes)
Harris Quarry EIR
County of Mendocino

Level Of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)
Intersection #1 US 101/Black Bart

Average Delay (sec/veh): 0.5 Worst Case Level Of Service: B [14.4]

Table with columns: Street Name (US 101), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L-T-R, L-T-R, L-T-R, L-T-R), Control (Uncontrolled, Uncontrolled, Uncontrolled, Uncontrolled), Rights (Include, Include, Include, Include), Lanes (1 0 2 0 0, 0 0 2 0 1, 0 0 1 1 0 0, 0 0 0 0 0), Volume Module, Base Vol (14 639), Growth Adj (1.00 1.00), Initial Bse (14 639), User Adj (1.00 1.00), PHF Adj (1.00 1.00), PHF Volume (14 639), Reduct Vol (0 0), Final Volume (14 639), Critical Gap (4.1), FollowUpTim (2.2), Capacity Module, Conflict Vol (651), Potent Cap (945), Move Cap (945), Total Cap (xxxx), Volume/Cap (0.01), Level Of Service Module, 2Way95thQ (0.0), Control Del (8.9), LOS by Move (A), Movement (LT-LTR-RT, LT-LTR-RT, LT-LTR-RT, LT-LTR-RT), Shared Queue (xxxx), Shrd Condel (xxxx), Shared LOS (B), ApproachDel (xxxxxx), ApproachLOS (B)

Note: Queue reported is the number of cars per lane.

11-12pm Peak Hour - Base 2010 October Peak (Mitigated Lanes)
Harris Quarry EIR
County of Mendocino

Level Of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)
Intersection #2 US 101/Quarry Access

Average Delay (sec/veh): 0.5 Worst Case Level Of Service: B [13.8]

Table with columns: Street Name (US 101), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L-T-R, L-T-R, L-T-R, L-T-R), Control (Uncontrolled, Uncontrolled, Uncontrolled, Uncontrolled), Rights (Include, Include, Include, Include), Lanes (1 0 2 0 0, 0 0 1 1 0, 0 0 1 1 0, 0 0 0 0 0), Volume Module, Base Vol (12 629), Growth Adj (1.00 1.00), Initial Bse (12 629), User Adj (1.00 1.00), PHF Adj (1.00 1.00), PHF Volume (12 629), Reduct Vol (0 0), Final Volume (12 629), Critical Gap (4.1), FollowUpTim (2.2), Capacity Module, Conflict Vol (636), Potent Cap (957), Move Cap (957), Total Cap (xxxx), Volume/Cap (0.01), Level Of Service Module, 2Way95thQ (0.0), Control Del (8.8), LOS by Move (A), Movement (LT-LTR-RT, LT-LTR-RT, LT-LTR-RT, LT-LTR-RT), Shared Queue (xxxx), Shrd Condel (xxxx), Shared LOS (B), ApproachDel (xxxxxx), ApproachLOS (B)

Note: Queue reported is the number of cars per lane.

4:45-5:45pm Peak Hour - Base 2010 October Peak (Mitigated Lanes)

Harris Quarry EIR
County of Mendocino

Level Of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart
Average Delay (sec/veh): 0.4 Worst Case Level Of Service: B [12.5]

Table with columns: Street Name, Approach, Movement, Control, Rights, Lanes, Volume Module, Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap, Level Of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Queue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

4:45-5:45pm Peak Hour - Base 2010 October Peak (Mitigated Lanes)

Harris Quarry EIR
County of Mendocino

Level Of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 US 101/Quarry Access
Average Delay (sec/veh): 0.1 Worst Case Level Of Service: B [12.7]

Table with columns: Street Name, Approach, Movement, Control, Rights, Lanes, Volume Module, Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap, Level Of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Queue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

9-10am Peak Hour - Base 2014 w/o Willits Bypass July Peak (Mitigated Lanes)
Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart
Average Delay (sec/veh): 0.4 Worst Case Level of Service: B [13.6]

Table with columns: Street Name, Approach, Movement, Control, Rights, Lanes, Volume Module, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap, Level of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Queue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

9-10am Peak Hour - Base 2014 w/o Willits Bypass July Peak (Mitigated Lanes)
Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 US 101/Quarry Access
Average Delay (sec/veh): 0.4 Worst Case Level of Service: B [13.5]

Table with columns: Street Name, Approach, Movement, Control, Rights, Lanes, Volume Module, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap, Level of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Queue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

11-12pm Peak Hour - Base 2014 w/o Willits Bypass October Peak (Mitigated Lanes)
Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)
Intersection #1 US 101/Black Bart

Average Delay (sec/veh): 0.5 Worst Case Level of Service: C [15.2]

Table with columns: Street Name (US 101), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L-T-R, L-T-R, L-T-R, L-T-R), Control (Uncontrolled, Uncontrolled, Uncontrolled, Uncontrolled), Rights (Include, Include, Include, Include), Lanes (1 0 2 0 0, 0 0 2 0 1, 0 0 1 1 0 0, 0 0 0 0 0), Volume Module (Base Vol: 15 685, Growth Adj: 1.00 1.00, Initial Bse: 15 685, User Adj: 1.00 1.00, PHF Adj: 1.00 1.00, PHF Volume: 15 685, Reduct Vol: 0 0, Final Volume: 15 685), Critical Gap Module (Critical Gap: 4.1 xxx, FollowUpTim: 2.2 xxx), Capacity Module (Conflict Vol: 698, Potent Cap.: 908, Move Cap.: 908, Total Cap.: xxx, Volume/Cap: 0.02), Level of Service Module (2Way95thQ: 0.1, Control Del: 9.0, LOS by Move: A, Movement: LT-LTR-RT, Shared Queue: xxx, Shrd ConDel: xxx, Shared LOS: *, ApproachDel: xxx, ApproachLOS: C), Note: Queue reported is the number of cars per lane.

11-12pm Peak Hour - Base 2014 w/o Willits Bypass October Peak (Mitigated Lanes)
Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)
Intersection #2 US 101/Quarry Access

Average Delay (sec/veh): 0.4 Worst Case Level of Service: B [14.4]

Table with columns: Street Name (US 101), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L-T-R, L-T-R, L-T-R, L-T-R), Control (Uncontrolled, Uncontrolled, Uncontrolled, Uncontrolled), Rights (Include, Include, Include, Include), Lanes (1 0 2 0 0, 0 0 1 1 0, 0 0 1 1 0, 0 0 0 0 0), Volume Module (Base Vol: 12 676, Growth Adj: 1.00 1.00, Initial Bse: 12 676, User Adj: 1.00 1.00, PHF Adj: 1.00 1.00, PHF Volume: 12 676, Reduct Vol: 0 0, Final Volume: 12 676), Critical Gap Module (Critical Gap: 4.1 xxx, FollowUpTim: 2.2 xxx), Capacity Module (Conflict Vol: 681, Potent Cap.: 921, Move Cap.: 921, Total Cap.: xxx, Volume/Cap: 0.01), Level of Service Module (2Way95thQ: 0.0, Control Del: 9.0, LOS by Move: A, Movement: LT-LTR-RT, Shared Queue: xxx, Shrd ConDel: xxx, Shared LOS: *, ApproachDel: xxx, ApproachLOS: B), Note: Queue reported is the number of cars per lane.

Level of Service Computation Report
 2000 HCM Unsignalized Method (Base Volume Alternative)
 Intersection #1 US 101/Black Bart
 Average Delay (sec/veh): 0.5 Worst Case Level of Service: C [15.7]
 Street Name: US 101 Black Bart
 Approach: North Bound South Bound East Bound West Bound
 Movement: L - T - R L - T - R L - T - R L - T - R
 Control: Uncontrolled Uncontrolled Uncontrolled Uncontrolled Stop Sign Stop Sign
 Rights: Include Include Include Include Include Include
 Lanes: 1 0 2 0 0 0 2 0 1 0 0 1 1 0 0 0 0 0 0 0
 Volume Module:
 Base Vol: 19 796 0 0 677 26 30 0 9 0 0 0
 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 Initial Bse: 19 796 0 0 677 26 30 0 9 0 0 0
 User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 PHF Volume: 19 796 0 0 677 26 30 0 9 0 0 0
 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
 FinalVolume: 19 796 0 0 677 26 30 0 9 0 0 0
 Critical Gap Module:
 Critical Gp: 4.1 xxxx xxxxx xxxxx xxxx xxxxx 6.8 6.5 6.9 xxxxx xxxx xxxxx
 FollowUpTim: 2.2 xxxx xxxxx xxxxx xxxx xxxxx 3.5 4.0 3.3 xxxxx xxxx xxxxx
 Capacity Module:
 Conflict Vol: 703 xxxx xxxxx xxxxx xxxxx xxxxx 1113 1511 339 xxxx xxxxx xxxxx
 Potent Cap.: 904 xxxx xxxxx xxxxx xxxxx xxxxx 206 121 663 xxxx xxxxx xxxxx
 Move Cap.: 904 xxxx xxxxx xxxxx xxxxx xxxxx 202 119 663 xxxx xxxx xxxxx
 Total Cap: xxxx xxxxx xxxxx xxxxx xxxxx xxxxx 333 242 xxxxx 253 234 xxxxx
 Volume/Cap: 0.02 xxxxx xxxxx xxxxx xxxxx xxxxx 0.09 0.00 0.01 xxxxx xxxx xxxx
 Level of Service Module:
 2Way95thQ: 0.1 xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
 Control Del: 9.1 xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
 LOS by Move: A *
 Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
 Shared Cap.: xxxx xxxxx xxxxx xxxxx xxxxx xxxxx 376 xxxxx xxxxx xxxxx
 SharedQueue: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 0.3 xxxxx xxxxx xxxxx
 Shrd ConDel: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 15.7 xxxxx xxxxx xxxxx
 Shared LOS: * * * * * * * * * * * * * * * * * * C * * * * * * * * * * * *
 ApproachDel: xxxxxx * xxxxxx 15.7 xxxxxx
 ApproachLOS: * * * * * * * * * * * * * * * * * * C * * * * * * * * * * * *
 Note: Queue reported is the number of cars per lane.

Level of Service Computation Report
 2000 HCM Unsignalized Method (Base Volume Alternative)
 Intersection #2 US 101/Quarry Access
 Average Delay (sec/veh): 0.3 Worst Case Level of Service: B [14.0]
 Street Name: US 101 Quarry Access
 Approach: North Bound South Bound East Bound West Bound
 Movement: L - T - R L - T - R L - T - R L - T - R
 Control: Uncontrolled Uncontrolled Uncontrolled Uncontrolled Stop Sign Stop Sign
 Rights: Include Include Include Include Include Include
 Lanes: 1 0 2 0 0 0 2 0 1 0 0 1 1 0 0 0 0 0 0 0
 Volume Module:
 Base Vol: 12 799 0 0 670 16 16 0 12 0 0 0
 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 Initial Bse: 12 799 0 0 670 16 16 0 12 0 0 0
 User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 PHF Volume: 12 799 0 0 670 16 16 0 12 0 0 0
 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
 FinalVolume: 12 799 0 0 670 16 16 0 12 0 0 0
 Critical Gap Module:
 Critical Gp: 4.1 xxxx xxxxx xxxxx xxxx xxxxx 6.8 6.5 6.9 xxxxx xxxx xxxxx
 FollowUpTim: 2.2 xxxx xxxxx xxxxx xxxx xxxxx 3.5 4.0 3.3 xxxxx xxxx xxxxx
 Capacity Module:
 Conflict Vol: 686 xxxx xxxxx xxxxx xxxxx xxxxx 1102 1501 343 xxxx xxxxx xxxxx
 Potent Cap.: 917 xxxx xxxxx xxxxx xxxxx xxxxx 209 123 659 xxxx xxxxx xxxxx
 Move Cap.: 917 xxxx xxxxx xxxxx xxxxx xxxxx 207 121 659 xxxx xxxx xxxxx
 Total Cap: xxxx xxxxx xxxxx xxxxx xxxxx xxxxx 337 245 xxxxx 258 242 xxxxx
 Volume/Cap: 0.01 xxxxx xxxxx xxxxx xxxxx xxxxx 0.05 0.00 0.02 xxxxx xxxx xxxx
 Level of Service Module:
 2Way95thQ: 0.0 xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
 Control Del: 9.0 xxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
 LOS by Move: A *
 Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
 Shared Cap.: xxxx xxxxx xxxxx xxxxx xxxxx xxxxx 426 xxxxx xxxxx xxxxx
 SharedQueue: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 0.2 xxxxx xxxxx xxxxx
 Shrd ConDel: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 14.0 xxxxx xxxxx xxxxx
 Shared LOS: * * * * * * * * * * * * * * * * * * B * * * * * * * * * * * *
 ApproachDel: xxxxxx * xxxxxx 14.0 xxxxxx
 ApproachLOS: * * * * * * * * * * * * * * * * * * B * * * * * * * * * * * *
 Note: Queue reported is the number of cars per lane.

4:45-5:45pm Peak Hour - Base 2014 w/o Willits Bypass October Peak (Mitigated La Harris Quarry EIR County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)
Intersection #1 US 101/Black Bart

Average Delay (sec/veh): 0.4 Worst Case Level of Service: B [13.0]

Street Name: US 101
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Uncontrolled Uncontrolled
Rights: Include Include Include Include
Lanes: 1 0 2 0 0 0 2 0 1 0 0 1 1 0 0 0 0 0 0 0

Volume Module:
Base Vol: 20 837 0 0 656 21 12 0 19 0 0 0

Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 20 837 0 0 656 21 12 0 19 0 0 0
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 20 837 0 0 656 21 12 0 19 0 0 0
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
FinalVolume: 20 837 0 0 656 21 12 0 19 0 0 0

Critical Gap Module:
Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx xxxxx
FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx xxxxx

Capacity Module:
Conflict Vol: 677 xxxxx xxxxx xxxxx xxxxx 1115 1533 328 xxxxx xxxxx xxxxx
Potent Cap.: 924 xxxxx xxxxx xxxxx xxxxx 205 118 674 xxxxx xxxxx xxxxx
Move Cap.: 924 xxxxx xxxxx xxxxx xxxxx 202 115 674 xxxxx xxxxx xxxxx

Total Cap: xxxxx xxxxx xxxxx xxxxx xxxxx 334 237 xxxxx 240 230 xxxxx
Volume/Cap: 0.02 xxxxx xxxxx xxxxx xxxxx 0.04 0.00 0.03 xxxxx xxxxx xxxxx
Level of Service Module:
2Way95thQ: 0.1 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Control Del: 9.0 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx

4:45-5:45pm Peak Hour - Base 2014 w/o Willits Bypass October Peak (Mitigated La Harris Quarry EIR County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)
Intersection #2 US 101/Quarry Access

Average Delay (sec/veh): 0.1 Worst Case Level of Service: B [13.2]

Street Name: US 101
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Uncontrolled Uncontrolled
Rights: Include Include Include Include
Lanes: 1 0 2 0 0 0 0 0 1 1 0 0 0 0 1 1 0 0 0 0 0 0

Volume Module:
Base Vol: 0 853 0 0 675 0 4 0 4 0 0 0

Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 0 853 0 0 675 0 4 0 4 0 0 0
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 0 853 0 0 675 0 4 0 4 0 0 0
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
FinalVolume: 0 853 0 0 675 0 4 0 4 0 0 0

Critical Gap Module:
Critical Gap: xxxxx xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx xxxxx
FollowUpTim: xxxxx xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx xxxxx

Capacity Module:
Conflict Vol: xxxxx xxxxx xxxxx xxxxx xxxxx 1102 1528 338 xxxxx xxxxx xxxxx
Potent Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx 209 119 664 xxxxx xxxxx xxxxx
Move Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx 209 119 664 xxxxx xxxxx xxxxx

Total Cap: xxxxx xxxxx xxxxx xxxxx xxxxx 339 242 xxxxx 253 242 xxxxx
Volume/Cap: xxxxx xxxxx xxxxx xxxxx xxxxx 0.01 0.00 0.01 xxxxx xxxxx xxxxx
Level of Service Module:
2Way95thQ: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Control Del: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx

LOS by Move: * * * * *
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 449 xxxxx xxxxx xxxxx
SharedQueue: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 0.1 xxxxx xxxxx xxxxx
Shrd ConDel: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 13.2 xxxxx xxxxx xxxxx
Shared LOS: * * * * *
ApproachDel: xxxxxx 13.2 xxxxxx
ApproachLOS: * * * * *
Note: Queue reported is the number of cars per lane.

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9-10am Peak Hour - Base 2030 July Peak (Mitigated Lanes)
Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart
Average Delay (sec/veh): 0.5 Worst Case Level of Service: C [16.0]

Table with columns: Street Name, Approach, Movement, Control, Rights, Lanes, Volume Module, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap, Level of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Queue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

9-10am Peak Hour - Base 2030 July Peak (Mitigated Lanes)
Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 US 101/Quarry Access
Average Delay (sec/veh): 0.3 Worst Case Level of Service: C [15.6]

Table with columns: Street Name, Approach, Movement, Control, Rights, Lanes, Volume Module, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap, Level of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Queue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

11-12pm Peak Hour - Base 2030 July Peak (Mitigated Lanes)

Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart
Average Delay (sec/veh): 0.6 Worst Case Level of Service: C [24.6]

Table with columns: Street Name (US 101), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L-T-R, L-T-R, L-T-R, L-T-R), Control (Uncontrolled, Uncontrolled, Uncontrolled, Uncontrolled), Rights (Include, Include, Include, Include), Lanes (1 0 2 0 0, 0 0 2 0 1, 0 0 1 1 0 0, 0 0 0 0 0), Volume Module (Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume), Critical Gap Module (Critical Gap, FollowUpTim), Capacity Module (Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap), Level of Service Module (2Way95thQ, Control Del, LOS by Move, Movement, Shared Queue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS).

Note: Queue reported is the number of cars per lane.

11-12pm Peak Hour - Base 2030 July Peak (Mitigated Lanes)

Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 US 101/Quarry Access
Average Delay (sec/veh): 0.3 Worst Case Level of Service: C [19.9]

Table with columns: Street Name (US 101), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L-T-R, L-T-R, L-T-R, L-T-R), Control (Uncontrolled, Uncontrolled, Uncontrolled, Uncontrolled), Rights (Include, Include, Include, Include), Lanes (1 0 2 0 0, 0 0 1 1 0, 0 0 1 1 0 0, 0 0 0 0 0), Volume Module (Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume), Critical Gap Module (Critical Gap, FollowUpTim), Capacity Module (Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap), Level of Service Module (2Way95thQ, Control Del, LOS by Move, Movement, Shared Queue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS).

Note: Queue reported is the number of cars per lane.

4:45-5:45pm Peak Hour - Base 2030 July Peak (Mitigated Lanes)

Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart
Average Delay (sec/veh): 0.4 Worst Case Level of Service: C [19.0]

Table with columns: Street Name, Approach, Movement, Control, Rights, Lanes, Volume Module, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap, Level of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Queue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

4:45-5:45pm Peak Hour - Base 2030 July Peak (Mitigated Lanes)

Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 US 101/Quarry Access
Average Delay (sec/veh): 0.1 Worst Case Level of Service: C [18.8]

Table with columns: Street Name, Approach, Movement, Control, Rights, Lanes, Volume Module, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap, Level of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Queue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

9-10am Peak Hour - Base 2030 October Peak (Mitigated Lanes)
Harris Quarry EIR
County of Mendocino

Level Of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)
Intersection #1 US 101/Black Bart

Average Delay (sec/veh): 0.5 Worst Case Level Of Service: B [13.7]

Street Name: US 101 Black Bart
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
Rights: Include Include Include Include
Lanes: 1 0 2 0 0 0 2 0 1 0 0 1 0 0 0 0 0 0 0 0
Volume Module:
Base Vol: 12 571 0 0 659 9 24 0 17 0 0 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 12 571 0 0 659 9 24 0 17 0 0 0
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 12 571 0 0 659 9 24 0 17 0 0 0
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
FinalVolume: 12 571 0 0 659 9 24 0 17 0 0 0
Critical Gap Module:
Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx xxxxx
FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx xxxxx

Capacity Module:
Conflict Vol: 668 xxxxx xxxxx xxxxx xxxxx 969 1254 330 xxxxx xxxxx xxxxx
Potent Cap.: 931 xxxxx xxxxx xxxxx xxxxx 255 173 672 xxxxx xxxxx xxxxx
Move Cap.: 931 xxxxx xxxxx xxxxx xxxxx 252 171 672 xxxxx xxxxx xxxxx
Total Cap: xxxxx xxxxx xxxxx xxxxx xxxxx 370 295 xxxxx 338 290 xxxxx
Volume/Cap: 0.01 xxxxx xxxxx xxxxx xxxxx 0.06 0.00 0.03 xxxxx xxxxx xxxxx

Level Of Service Module:
2Way95thQ: 0.0 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Control Del: 8.9 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
LOS by Move: A * * * * *
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx 455 xxxxx xxxxx xxxxx xxxxx
SharedQueue: xxxxx xxxxx xxxxx xxxxx xxxxx 0.3 xxxxx xxxxx xxxxx xxxxx
Shrd ConDel: xxxxx xxxxx xxxxx xxxxx xxxxx 13.7 xxxxx xxxxx xxxxx xxxxx
Shared LOS: * * * * *
ApproachDel: xxxxxx 13.7 xxxxxx
ApproachLOS: * * * * *

Note: Queue reported is the number of cars per lane.

9-10am Peak Hour - Base 2030 October Peak (Mitigated Lanes)
Harris Quarry EIR
County of Mendocino

Level Of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)
Intersection #2 US 101/Quarry Access

Average Delay (sec/veh): 0.5 Worst Case Level Of Service: B [14.0]

Street Name: US 101 Quarry Access
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
Rights: Include Include Include Include
Lanes: 1 0 2 0 0 0 2 0 1 0 0 1 0 0 0 0 0 0 0 0
Volume Module:
Base Vol: 12 559 0 0 652 24 24 0 12 0 12 0 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 12 559 0 0 652 24 24 0 12 0 12 0 0
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 12 559 0 0 652 24 24 0 12 0 12 0 0
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0
FinalVolume: 12 559 0 0 652 24 24 0 12 0 12 0 0
Critical Gap Module:
Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx xxxxx
FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx xxxxx

Capacity Module:
Conflict Vol: 676 xxxxx xxxxx xxxxx xxxxx 968 1247 338 xxxxx xxxxx xxxxx
Potent Cap.: 925 xxxxx xxxxx xxxxx xxxxx 255 175 664 xxxxx xxxxx xxxxx
Move Cap.: 925 xxxxx xxxxx xxxxx xxxxx 253 173 664 xxxxx xxxxx xxxxx
Total Cap: xxxxx xxxxx xxxxx xxxxx xxxxx 370 297 xxxxx 345 291 xxxxx
Volume/Cap: 0.01 xxxxx xxxxx xxxxx xxxxx 0.06 0.00 0.02 xxxxx xxxxx xxxxx

Level Of Service Module:
2Way95thQ: 0.0 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Control Del: 8.9 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
LOS by Move: A * * * * *
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx 434 xxxxx xxxxx xxxxx xxxxx
SharedQueue: xxxxx xxxxx xxxxx xxxxx xxxxx 0.3 xxxxx xxxxx xxxxx xxxxx
Shrd ConDel: xxxxx xxxxx xxxxx xxxxx xxxxx 14.0 xxxxx xxxxx xxxxx xxxxx
Shared LOS: * * * * *
ApproachDel: xxxxxx 14.0 xxxxxx
ApproachLOS: * * * * *

Note: Queue reported is the number of cars per lane.

1:15-2:15pm Peak Hour - Base 2030 October Peak (Mitigated Lanes)
 Harris Quarry EIR
 County of Mendocino

Level Of Service Computation Report
 2000 HCM Unsignalized Method (Base Volume Alternative)

 Intersection #1 US 101/Black Bart

Average Delay (sec/veh): 0.6 Worst Case Level Of Service: C [19.5]

Street Name: US 101
 Approach: North Bound South Bound East Bound West Bound
 Movement: L - T - R L - T - R L - T - R L - T - R
 Control: Uncontrolled Uncontrolled Uncontrolled Uncontrolled
 Rights: Include Include Include Include
 Lanes: 1 0 2 0 0 0 2 0 1 0 0 1 0 0 0 0 0 0 0 0

Volume Module:
 Base Vol: 24 1013 0 0 860 33 38 0 12 0 0
 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 Initial Bse: 24 1013 0 0 860 33 38 0 12 0 0
 User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 PHF Volume: 24 1013 0 0 860 33 38 0 12 0 0
 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0
 FinalVolume: 24 1013 0 0 860 33 38 0 12 0 0

Critical Gap Module:
 Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx xxxxx
 FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx xxxxx

Capacity Module:
 Conflict Vol: 893 xxxxx xxxxx xxxxx 1415 1921 430 xxxxx xxxxx xxxxx
 Potent Cap.: 768 xxxxx xxxxx xxxxx 131 68 579 xxxxx xxxxx xxxxx
 Move Cap.: 768 xxxxx xxxxx xxxxx 128 66 579 xxxxx xxxxx xxxxx
 Total Cap: xxxxx xxxxx xxxxx xxxxx 258 180 xxxxx 180 172 xxxxx
 Volume/Cap: 0.03 xxxxx xxxxx xxxxx 0.15 0.00 0.02 xxxxx xxxxx xxxxx

Level Of Service Module:
 2Way95thQ: 0.1 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
 Control Del: 9.8 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
 LOS by Move: A * * * * *
 Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT
 Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx 297 xxxxx xxxxx xxxxx
 SharedQueue: xxxxx xxxxx xxxxx xxxxx xxxxx 0.6 xxxxx xxxxx xxxxx
 Shrd ConDel: xxxxx xxxxx xxxxx xxxxx xxxxx 19.5 xxxxx xxxxx xxxxx
 Shared LOS: * * * * * C * *
 ApproachDel: xxxxxx 19.5 xxxxxx
 ApproachLOS: * * * * * C

Note: Queue reported is the number of cars per lane.

1:15-2:15pm Peak Hour - Base 2030 October Peak (Mitigated Lanes)
 Harris Quarry EIR
 County of Mendocino

Level Of Service Computation Report
 2000 HCM Unsignalized Method (Base Volume Alternative)

 Intersection #2 US 101/Quarry Access

Average Delay (sec/veh): 0.3 Worst Case Level Of Service: C [16.5]

Street Name: US 101
 Approach: North Bound South Bound East Bound West Bound
 Movement: L - T - R L - T - R L - T - R L - T - R
 Control: Uncontrolled Uncontrolled Uncontrolled Uncontrolled
 Rights: Include Include Include Include
 Lanes: 1 0 2 0 0 0 0 1 1 0 0 0 1 1 0 0 0 0 0 0

Volume Module:
 Base Vol: 12 1021 0 0 856 16 16 0 12 0 0
 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 Initial Bse: 12 1021 0 0 856 16 16 0 12 0 0
 User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 PHF Volume: 12 1021 0 0 856 16 16 0 12 0 0
 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0
 FinalVolume: 12 1021 0 0 856 16 16 0 12 0 0

Critical Gap Module:
 Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx xxxxx
 FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx xxxxx

Capacity Module:
 Conflict Vol: 872 xxxxx xxxxx xxxxx xxxxx 1399 1909 436 xxxxx xxxxx xxxxx
 Potent Cap.: 782 xxxxx xxxxx xxxxx xxxxx 134 69 574 xxxxx xxxxx xxxxx
 Move Cap.: 782 xxxxx xxxxx xxxxx xxxxx 132 68 574 xxxxx xxxxx xxxxx
 Total Cap: xxxxx xxxxx xxxxx xxxxx 262 184 xxxxx 187 181 xxxxx
 Volume/Cap: 0.02 xxxxx xxxxx xxxxx xxxxx 0.06 0.00 0.02 xxxxx xxxxx xxxxx

Level Of Service Module:
 2Way95thQ: 0.0 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
 Control Del: 9.7 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
 LOS by Move: A * * * * *
 Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT
 Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx 341 xxxxx xxxxx xxxxx
 SharedQueue: xxxxx xxxxx xxxxx xxxxx xxxxx 0.3 xxxxx xxxxx xxxxx
 Shrd ConDel: xxxxx xxxxx xxxxx xxxxx xxxxx 16.5 xxxxx xxxxx xxxxx
 Shared LOS: * * * * * C * *
 ApproachDel: xxxxxx 16.5 xxxxxx
 ApproachLOS: * * * * * C

Note: Queue reported is the number of cars per lane.

4:45-5:45pm Peak Hour - Base 2030 October Peak (Mitigated Lanes)

Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart
Average Delay (sec/veh): 0.4 Worst Case Level of Service: C [15.1]

Table with columns: Street Name, Approach, Movement, Control, Rights, Lanes, Volume Module, Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap, Level of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

4:45-5:45pm Peak Hour - Base 2030 October Peak (Mitigated Lanes)

Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 US 101/Quarry Access
Average Delay (sec/veh): 0.1 Worst Case Level of Service: C [15.2]

Table with columns: Street Name, Approach, Movement, Control, Rights, Lanes, Volume Module, Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap, Level of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

11-12pm Peak Hour - Base 2040 July Peak (Mitigated Lanes)
 Harris Quarry EIR
 County of Mendocino

Level Of Service Computation Report
 2000 HCM Unsignalized Method (Base Volume Alternative)
 Intersection #1 US 101/Black Bart

Average Delay (sec/veh): 0.8 Worst Case Level Of Service: D [30.6]
 Street Name: US 101 Black Bart
 Approach: North Bound South Bound East Bound West Bound
 Movement: L T R L T R L T R L T R L T R
 Control: Uncontrolled Uncontrolled Uncontrolled Uncontrolled Stop Sign Stop Sign
 Rights: Include Include Include Include Include Include
 Lanes: 1 0 2 0 0 0 2 0 1 0 0 1 0 0 0 0 0 0 0

Volume Module:
 Base Vol: 22 1259 0 0 1243 38 43 0 14 0 0 0
 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 Initial Bse: 22 1259 0 0 1243 38 43 0 14 0 0 0
 User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 PHF Volume: 22 1259 0 0 1243 38 43 0 14 0 0 0
 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
 FinalVolume: 22 1259 0 0 1243 38 43 0 14 0 0 0

Critical Gap Module:
 Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx xxxxx
 FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx xxxxx

Capacity Module:
 Conflict Vol: 1281 xxxxx xxxxx xxxxx xxxxx 1917 2546 622 xxxxx xxxxx xxxxx
 Potent Cap.: 549 xxxxx xxxxx xxxxx xxxxx 61 27 435 xxxxx xxxxx xxxxx
 Move Cap.: 549 xxxxx xxxxx xxxxx xxxxx 59 26 435 xxxxx xxxxx xxxxx
 Total Cap: xxxxx xxxxx xxxxx xxxxx xxxxx 167 119 xxxxx 122 112 xxxxx
 Volume/Cap: 0.04 xxxxx xxxxx xxxxx xxxxx 0.26 0.00 0.03 xxxxx xxxxx xxxxx

Level Of Service Module:
 2Way95thQ: 0.1 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
 Control Del: 11.8 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
 LOS by Move: B * * * * *
 Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT
 Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx 197 xxxxx xxxxx xxxxx xxxxx
 SharedQueue: xxxxx xxxxx xxxxx xxxxx xxxxx 1.1 xxxxx xxxxx xxxxx xxxxx
 Shrd ConDel: xxxxx xxxxx xxxxx xxxxx xxxxx 30.6 xxxxx xxxxx xxxxx xxxxx
 Shared LOS: * * * * *
 ApproachDel: xxxxxx 30.6 xxxxxx
 ApproachLOS: * * * * *

Note: Queue reported is the number of cars per lane.

11-12pm Peak Hour - Base 2040 July Peak (Mitigated Lanes)
 Harris Quarry EIR
 County of Mendocino

Level Of Service Computation Report
 2000 HCM Unsignalized Method (Base Volume Alternative)
 Intersection #2 US 101/Quarry Access

Average Delay (sec/veh): 0.3 Worst Case Level Of Service: C [23.1]
 Street Name: US 101 Quarry Access
 Approach: North Bound South Bound East Bound West Bound
 Movement: L T R L T R L T R L T R L T R
 Control: Uncontrolled Uncontrolled Uncontrolled Uncontrolled Stop Sign Stop Sign
 Rights: Include Include Include Include Include Include
 Lanes: 1 0 2 0 0 0 2 0 1 0 0 1 0 0 0 0 0 0 0

Volume Module:
 Base Vol: 12 1277 0 0 1241 16 16 0 12 0 12 0 0
 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 Initial Bse: 12 1277 0 0 1241 16 16 0 12 0 12 0 0
 User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 PHF Volume: 12 1277 0 0 1241 16 16 0 12 0 12 0 0
 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
 FinalVolume: 12 1277 0 0 1241 16 16 0 12 0 12 0 0

Critical Gap Module:
 Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx xxxxx
 FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx xxxxx

Capacity Module:
 Conflict Vol: 1257 xxxxx xxxxx xxxxx xxxxx 1912 2550 629 xxxxx xxxxx xxxxx
 Potent Cap.: 560 xxxxx xxxxx xxxxx xxxxx 61 27 430 xxxxx xxxxx xxxxx
 Move Cap.: 560 xxxxx xxxxx xxxxx xxxxx 60 27 430 xxxxx xxxxx xxxxx
 Total Cap: xxxxx xxxxx xxxxx xxxxx xxxxx 168 120 xxxxx 125 117 xxxxx
 Volume/Cap: 0.02 xxxxx xxxxx xxxxx xxxxx 0.10 0.00 0.03 xxxxx xxxxx xxxxx

Level Of Service Module:
 2Way95thQ: 0.1 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
 Control Del: 11.6 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
 LOS by Move: B * * * * *
 Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT
 Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx 227 xxxxx xxxxx xxxxx xxxxx
 SharedQueue: xxxxx xxxxx xxxxx xxxxx xxxxx 0.4 xxxxx xxxxx xxxxx xxxxx
 Shrd ConDel: xxxxx xxxxx xxxxx xxxxx xxxxx 23.1 xxxxx xxxxx xxxxx xxxxx
 Shared LOS: * * * * *
 ApproachDel: xxxxxx 23.1 xxxxxx
 ApproachLOS: * * * * *

Note: Queue reported is the number of cars per lane.

4:45-5:45pm Peak Hour - Base 2040 July Peak (Mitigated Lanes)

Harris Quarry EIR

County of Mendocino

Level of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart

Average Delay (sec/veh): 0.5 Worst Case Level of Service: C [22.4]

Street Name: US 101
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
Rights: Include Include Include Include
Lanes: 1 0 2 0 0 0 2 0 1 0 0 1 0 0 0 0 0 0 0

Volume Module:
Base Vol: 29 1585 0 0 1247 31 17 0 27 0 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Critical Gap Module:
Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx xxxxx
FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx xxxxx

Capacity Module:
Conflict Vol: 1278 xxxxx xxxxx xxxxx xxxxx 2098 2890 624 xxxxx xxxxx xxxxx
Potential Cap.: 550 xxxxx xxxxx xxxxx xxxxx 46 16 434 xxxxx xxxxx xxxxx

Level of Service Module:
2Way95thQ: 0.2 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Control Del: 11.9 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx

4:45-5:45pm Peak Hour - Base 2040 July Peak (Mitigated Lanes)

Harris Quarry EIR

County of Mendocino

Level of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 US 101/Quarry Access

Average Delay (sec/veh): 0.1 Worst Case Level of Service: C [21.5]

Street Name: US 101
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
Rights: Include Include Include Include
Lanes: 1 0 2 0 0 0 0 0 1 1 0 0 0 0 1 0 0 0 0 0 0

Volume Module:
Base Vol: 0 1610 0 0 1274 0 4 0 4 0 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Critical Gap Module:
Critical Gap: xxxxxx xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx xxxxx
FollowUpTim: xxxxxx xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx xxxxx

Capacity Module:
Conflict Vol: xxxxx xxxxx xxxxx xxxxx xxxxx 2079 2884 637 xxxxx xxxxx xxxxx
Potential Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx 47 16 425 xxxxx xxxxx xxxxx

Level of Service Module:
2Way95thQ: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Control Del: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx

9-10am Peak Hour - Base 2040 October Peak (Mitigated Lanes)

Harris Quarry EIR
County of Mendocino

Level Of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart

Average Delay (sec/veh): 0.6 Worst Case Level Of Service: B [14.9]

Street Name: US 101 Black Bart

Approach: North Bound South Bound East Bound West Bound

Movement: L T R L T R L T R L T R

Control: Uncontrolled Uncontrolled Uncontrolled Stop Sign

Rights: Include Include Include Include

Lanes: 1 0 2 0 0 0 2 0 1 0 0 1 0 0 0 0 0 0 0

Volume Module:

Table with 18 columns: Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap, Level Of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

9-10am Peak Hour - Base 2040 October Peak (Mitigated Lanes)

Harris Quarry EIR
County of Mendocino

Level Of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 US 101/Quarry Access

Average Delay (sec/veh): 0.5 Worst Case Level Of Service: C [15.1]

Street Name: US 101 Quarry Access

Approach: North Bound South Bound East Bound West Bound

Movement: L T R L T R L T R L T R

Control: Uncontrolled Uncontrolled Stop Sign

Rights: Include Include Include Include

Lanes: 1 0 2 0 0 0 0 1 1 0 0 0 1 1 0 0 0 0 0 0

Volume Module:

Table with 18 columns: Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap, Level Of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

1:15-2:15 Peak Hour - Base 2040 October Peak (Mitigated Lanes)

Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart
Average Delay (sec/veh): 0.7 Worst Case Level of Service: C [22.9]

Table with columns: Street Name (US 101), Approach (North, South, East, West), Movement (L, T, R), and various traffic metrics like Volume, Growth Adj, Initial Bse, User Adj, PHF, Reduct Vol, Final Volume.

Critical Gap Module:

Table with columns: Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap.

Level of Service Module:

Table with columns: 2Way95thQ, Control Del, LOS by Move, Movement, Shared Queue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

1:15-2:15 Peak Hour - Base 2040 October Peak (Mitigated Lanes)

Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 US 101/Quarry Access
Average Delay (sec/veh): 0.3 Worst Case Level of Service: C [18.3]

Table with columns: Street Name (US 101), Approach (North, South, East, West), Movement (L, T, R), and various traffic metrics like Volume, Growth Adj, Initial Bse, User Adj, PHF, Reduct Vol, Final Volume.

Critical Gap Module:

Table with columns: Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap.

Level of Service Module:

Table with columns: 2Way95thQ, Control Del, LOS by Move, Movement, Shared Queue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

9-10am Peak Hour - Project 2010 July Peak (Existing Lanes)
Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)
Intersection #1 US 101/Black Bart

Average Delay (sec/veh): 0.5 Worst Case Level of Service: C [16.2]

Table with columns: Street Name, Approach, Movement, Control, Rights, Lanes, Volume Module, Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Volume/Cap, Level of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap, SharedQueue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

9-10am Peak Hour - Project 2010 July Peak (Existing Lanes)
Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)
Intersection #2 US 101/Quarry Access

Average Delay (sec/veh): 1.3 Worst Case Level of Service: C [16.8]

Table with columns: Street Name, Approach, Movement, Control, Rights, Lanes, Volume Module, Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Volume/Cap, Level of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap, SharedQueue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

11-12pm Peak Hour - Project 2010 July Peak (Existing Lanes)
Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart

Average Delay (sec/veh): 0.7 Worst Case Level of Service: D [27.6]

Street Name: US 101

Table with 11 columns: Approach (North, South, East, West), Movement (L, T, R), Uncontrolled Include, Stop Sign Include, Stop Sign, Quarry Access

Control: Uncontrolled Include

Rights: Uncontrolled Include

Lanes: 1 0 2 0 0 0 0 2 0 1 0 0 1 1 0 0 0 0 0 0 0 0

Volume Module:

Table with 2 columns: Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume

Critical Gap Module:

Table with 2 columns: Critical Gap, FollowUpTim

Capacity Module:

Table with 2 columns: Conflict Vol, Potent Cap, Move Cap, Volume/Cap

Level of Service Module:

Table with 2 columns: 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap, Shared Queue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS

Note: Queue reported is the number of cars per lane.

11-12pm Peak Hour - Project 2010 July Peak (Existing Lanes)
Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 US 101/Quarry Access

Average Delay (sec/veh): 1.3 Worst Case Level of Service: C [24.5]

Street Name: US 101

Table with 11 columns: Approach (North, South, East, West), Movement (L, T, R), Uncontrolled Include, Stop Sign Include, Stop Sign, Quarry Access

Control: Uncontrolled Include

Rights: Uncontrolled Include

Lanes: 0 1 1 0 0 0 0 1 1 0 0 0 0 1 1 0 0 0 0 0 0 0

Volume Module:

Table with 2 columns: Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume

Critical Gap Module:

Table with 2 columns: Critical Gap, FollowUpTim

Capacity Module:

Table with 2 columns: Conflict Vol, Potent Cap, Move Cap, Volume/Cap

Level of Service Module:

Table with 2 columns: 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap, Shared Queue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
 2000 HCM Unsignalized Method (Base Volume Alternative)
 Intersection #1 US 101/Black Bart

Average Delay (sec/veh): 0.4 Worst Case Level Of Service: C [20.5]

Street Name: US 101 Black Bart

Approach:	North Bound		South Bound		East Bound		West Bound	
	L	T - R	L	T - R	L	T - R	L	T - R
Control:	Uncontrolled		Uncontrolled		Stop Sign		Stop Sign	
Rights:	Include		Include		Include		Include	
Lanes:	1	0 2 0 0	0	0 2 0 1	0	0 1 0 0	0	0 0 0 0

Volume Module:

Base Vol:	18	979	0	822	23	12	0	18	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	18	979	0	822	23	12	0	18	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	18	979	0	822	23	12	0	18	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0
Final Volume:	18	979	0	822	23	12	0	18	0	0	0

Critical Gap Module:

Critical Gap:	4.1	xxxx	xxxx	xxxx	xxxx	6.8	6.5	6.9	xxxx	xxxx	xxxx
FollowUpTim:	2.2	xxxx	xxxx	xxxx	xxxx	3.5	4.0	3.3	xxxx	xxxx	xxxx

Capacity Module:

Conflict Vol:	845	xxxx	xxxx	xxxx	xxxx	1348	1837	411	xxxx	xxxx	xxxx
Potent Cap.:	800	xxxx	xxxx	xxxx	xxxx	145	77	596	xxxx	xxxx	xxxx
Move Cap.:	800	xxxx	xxxx	xxxx	xxxx	142	75	596	xxxx	xxxx	xxxx
Volume/Cap.:	0.02	xxxx	xxxx	xxxx	xxxx	0.08	0.00	0.03	xxxx	xxxx	xxxx

Level Of Service Module:

2Way95thQ: 0.1

Control Del: 9.6

LOS by Move: A

Movement: LT - LTR - RT

Shared Cap.: 0.4

Shrd Condel: 20.5

Shared LOS: A

ApproachDel: 20.5

ApproachLOS: C

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
 2000 HCM Unsignalized Method (Base Volume Alternative)
 Intersection #2 US 101/Quarry Access

Average Delay (sec/veh): 1.0 Worst Case Level Of Service: D [26.2]

Street Name: US 101 Quarry Access

Approach:	North Bound		South Bound		East Bound		West Bound	
	L	T - R	L	T - R	L	T - R	L	T - R
Control:	Uncontrolled		Uncontrolled		Stop Sign		Stop Sign	
Rights:	Include		Include		Include		Include	
Lanes:	0	1 1 0 0	0	0 1 1 0	0	0 1 0 0	0	0 0 0 0

Volume Module:

Base Vol:	32	969	0	812	28	0	32	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	32	969	0	812	28	0	32	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	32	969	0	812	28	0	32	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0
Final Volume:	32	969	0	812	28	0	32	0	0

Critical Gap Module:

Critical Gap:	4.1	xxxx	xxxx	xxxx	xxxx	6.8	6.5	6.9	xxxx	xxxx	xxxx
FollowUpTim:	2.2	xxxx	xxxx	xxxx	xxxx	3.5	4.0	3.3	xxxx	xxxx	xxxx

Capacity Module:

Conflict Vol:	840	xxxx	xxxx	xxxx	xxxx	1375	1859	420	xxxx	xxxx	xxxx
Potent Cap.:	804	xxxx	xxxx	xxxx	xxxx	139	74	588	xxxx	xxxx	xxxx
Move Cap.:	804	xxxx	xxxx	xxxx	xxxx	135	71	588	xxxx	xxxx	xxxx
Volume/Cap.:	0.04	xxxx	xxxx	xxxx	xxxx	0.21	0.00	0.05	xxxx	xxxx	xxxx

Level Of Service Module:

2Way95thQ: 0.1

Control Del: 9.7

LOS by Move: A

Movement: LT - LTR - RT

Shared Cap.: 0.1

Shrd Condel: 26.2

Shared LOS: A

ApproachDel: 26.2

ApproachLOS: D

Note: Queue reported is the number of cars per lane.

9-10am Peak Hour - Project 2010 October Peak (Existing Lanes)
 Harris Quarry EIR
 County of Mendocino

Level Of Service Computation Report
 2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart
 Average Delay (sec/veh): 0.5 Worst Case Level Of Service: B [13.7]

Street Name: US 101 Black Bart
 Approach: North Bound South Bound East Bound West Bound
 Movement: L - T - R L - T - R L - T - R L - T - R
 Control: Uncontrolled Uncontrolled Uncontrolled Uncontrolled
 Rights: Include Include Include Include
 Lanes: 1 0 2 0 0 0 2 0 1 0 0 1 1 0 0 0 0 0 0 0 0

Volume Module:
 Base Vol: 9 443 0 0 507 7 18 0 12 0 0
 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 Initial Bse: 9 443 0 0 507 7 18 0 12 0 0
 User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 PHF Volume: 9 443 0 0 507 7 18 0 12 0 0
 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0
 FinalVolume: 9 443 0 0 507 7 18 0 12 0 0

Critical Gap Module:
 Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx xxxxx
 FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx xxxxx

Capacity Module:
 Conflict Vol: 514 xxxxx xxxxx xxxxx xxxxx 747 968 254 xxxxx xxxxx xxxxx
 Potent Cap.: 1062 xxxxx xxxxx xxxxx xxxxx 353 256 752 xxxxx xxxxx xxxxx
 Move Cap.: 1062 xxxxx xxxxx xxxxx xxxxx 351 254 752 xxxxx xxxxx xxxxx
 Volume/Cap: 0.01 xxxxx xxxxx xxxxx xxxxx 0.05 0.00 0.02 xxxxx xxxxx xxxxx

Level Of Service Module:
 2Way95thQ: 0.0 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
 Control Del: 8.4 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
 LOS by Move: A * * * * *
 Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
 Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx 446 xxxxx xxxxx xxxxx
 SharedQueue: xxxxx xxxxx xxxxx xxxxx xxxxx 0.2 xxxxx xxxxx xxxxx
 Shrd Condel: xxxxx xxxxx xxxxx xxxxx xxxxx 13.7 xxxxx xxxxx xxxxx
 Shared LOS: * * * * * B * * * * *
 ApproachDel: xxxxxx 13.7
 ApproachLOS: * * * * * B

Note: Queue reported is the number of cars per lane.

9-10am Peak Hour - Project 2010 October Peak (Existing Lanes)
 Harris Quarry EIR
 County of Mendocino

Level Of Service Computation Report
 2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 US 101/Quarry Access
 Average Delay (sec/veh): 1.8 Worst Case Level Of Service: C [15.0]

Street Name: US 101 Quarry Access
 Approach: North Bound South Bound East Bound West Bound
 Movement: L - T - R L - T - R L - T - R L - T - R
 Control: Uncontrolled Uncontrolled Uncontrolled Uncontrolled
 Rights: Include Include Include Include
 Lanes: 0 1 1 0 0 0 0 1 1 0 0 0 1 1 0 0 0 0 0 0 0

Volume Module:
 Base Vol: 56 408 0 0 475 44 44 0 56 0 0
 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 Initial Bse: 56 408 0 0 475 44 44 0 56 0 0
 User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 PHF Volume: 56 408 0 0 475 44 44 0 56 0 0
 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0
 FinalVolume: 56 408 0 0 475 44 44 0 56 0 0

Critical Gap Module:
 Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx xxxxx
 FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx xxxxx

Capacity Module:
 Conflict Vol: 519 xxxxx xxxxx xxxxx xxxxx 813 1017 260 xxxxx xxxxx xxxxx
 Potent Cap.: 1057 xxxxx xxxxx xxxxx xxxxx 320 239 745 xxxxx xxxxx xxxxx
 Move Cap.: 1057 xxxxx xxxxx xxxxx xxxxx 307 226 745 xxxxx xxxxx xxxxx
 Volume/Cap: 0.05 xxxxx xxxxx xxxxx xxxxx 0.14 0.00 0.08 xxxxx xxxxx xxxxx

Level Of Service Module:
 2Way95thQ: 0.2 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
 Control Del: 8.6 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
 LOS by Move: A * * * * *
 Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
 Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx 458 xxxxx xxxxx xxxxx
 SharedQueue: 0.2 xxxxx xxxxx xxxxx xxxxx xxxxx 0.8 xxxxx xxxxx xxxxx
 Shrd Condel: 8.6 xxxxx xxxxx xxxxx xxxxx 15.0 xxxxx xxxxx xxxxx
 Shared LOS: A * * * * * C * * * * *
 ApproachDel: xxxxxx 15.0
 ApproachLOS: * * * * * C

Note: Queue reported is the number of cars per lane.

Harris Quarry EIR
County of Mendocino

Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)
Intersection #1 US 101/Black Bart
Average Delay (sec/veh): 0.6 Worst Case Level of Service: C [19.6]

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)
Intersection #2 US 101/Quarry Access
Average Delay (sec/veh): 1.7 Worst Case Level of Service: C [19.7]

Street Name: US 101
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
Rights: Include Include Include Include
Lanes: 1 0 2 0 0 0 2 0 1 0 0 1 0 0 0 0 0 0 0

Street Name: US 101
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
Rights: Include Include Include Include
Lanes: 0 1 1 0 0 0 0 1 1 0 0 0 1 0 0 0 0 0 0

Volume Module:
Base Vol: 14 659 0 0 647 24 28 0 9 0 0 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 14 659 0 0 647 24 28 0 9 0 0
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 14 659 0 0 647 24 28 0 9 0 0
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0
FinalVolume: 14 659 0 0 647 24 28 0 9 0 0

Volume Module:
Base Vol: 56 629 0 0 612 44 44 0 56 0 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 56 629 0 0 612 44 44 0 56 0 0
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 56 629 0 0 612 44 44 0 56 0 0
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0
FinalVolume: 56 629 0 0 612 44 44 0 56 0 0

Critical Gap Module:
Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx

Critical Gap Module:
Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx

FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx

FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx

Capacity Module:
Conflict Vol: 671 xxxxx xxxxx xxxxx xxxxx 1005 1334 324 xxxxx xxxxx
Potent Cap.: 929 xxxxx xxxxx xxxxx xxxxx 242 155 678 xxxxx xxxxx
Move Cap.: 929 xxxxx xxxxx xxxxx xxxxx 239 153 678 xxxxx xxxxx
Volume/Cap: 0.02 xxxxx xxxxx xxxxx xxxxx 0.12 0.00 0.01 xxxxx xxxxx

Capacity Module:
Conflict Vol: 656 xxxxx xxxxx xxxxx xxxxx 1061 1375 328 xxxxx xxxxx
Potent Cap.: 941 xxxxx xxxxx xxxxx xxxxx 222 147 674 xxxxx xxxxx
Move Cap.: 941 xxxxx xxxxx xxxxx xxxxx 212 138 674 xxxxx xxxxx
Volume/Cap: 0.06 xxxxx xxxxx xxxxx xxxxx 0.21 0.00 0.08 xxxxx xxxxx

Level of Service Module:
2Way95thQ: 0.0 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Control Del: 8.9 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx

Level of Service Module:
2Way95thQ: 0.2 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Control Del: 9.1 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx

LOS by Move: A * * * * *
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT

LOS by Move: A * * * * *
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT

Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx 284 xxxxx xxxxx xxxxx
SharedQueue: xxxxx xxxxx xxxxx xxxxx xxxxx 0.4 xxxxx xxxxx xxxxx
Shrd Condel: xxxxx xxxxx xxxxx xxxxx xxxxx 19.6 xxxxx xxxxx xxxxx

Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx 344 xxxxx xxxxx xxxxx
SharedQueue: xxxxx xxxxx xxxxx xxxxx xxxxx 1.2 xxxxx xxxxx xxxxx
Shrd Condel: xxxxx xxxxx xxxxx xxxxx xxxxx 19.7 xxxxx xxxxx xxxxx

Shared LOS: * * * * * C
ApproachDel: xxxxxx 19.6
ApproachLOS: * * * * * C

Shared LOS: * * * * * C
ApproachDel: xxxxxx 19.7
ApproachLOS: * * * * * C

Note: Queue reported is the number of cars per lane.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

 Intersection #2 US 101/Quarry Access

Average Delay (sec/veh): 1.6 Worst Case Level Of Service: E [36.9]

Street Name: US 101 Quarry Access
 Approach: North Bound South Bound East Bound West Bound
 Movement: L T R L T R L T R L T R L T R L T R
 Control: Uncontrolled Uncontrolled Uncontrolled Uncontrolled
 Rights: Include Include Include Include
 Lanes: 0 1 1 0 0 0 0 1 1 0 0 0 1 1 0 0 0 0 0 0 0

Volume Module:
 Base Vol: 28 881 0 0 856 48 48 0 28 0 0 28 0 0
 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 Initial Bse: 28 881 0 0 856 48 48 0 28 0 0 28 0 0
 User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 PHF Volume: 28 881 0 0 856 48 48 0 28 0 0 28 0 0
 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0 0
 FinalVolume: 28 881 0 0 856 48 48 0 28 0 0 28 0 0

Critical Gap Module:

Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx xxxxx
 FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx xxxxx

Capacity Module:
 Conflict Vol: 904 xxxxx xxxxx xxxxx xxxxx 1377 1817 452 xxxxx xxxxx xxxxx
 Potent Cap: 761 xxxxx xxxxx xxxxx xxxxx 139 79 560 xxxxx xxxxx xxxxx
 Move Cap: 761 xxxxx xxxxx xxxxx xxxxx 135 76 560 xxxxx xxxxx xxxxx
 Volume/Cap: 0.04 xxxxx xxxxx xxxxx xxxxx 0.36 0.00 0.05 xxxxx xxxxx xxxxx

Level Of Service Module:

2Way5thQ: 0.1 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
 Control Del: 9.9 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
 LOS by Move: A * * * * *
 Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
 Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 187 xxxxx xxxxx xxxxx
 SharedQueue: 0.1 xxxxx xxxxx xxxxx xxxxx xxxxx 1.8 xxxxx xxxxx xxxxx xxxxx
 Shrd Condel: 9.9 xxxxx xxxxx xxxxx xxxxx xxxxx 36.9 xxxxx xxxxx xxxxx xxxxx
 Shared LOS: A * * * * * E * * * * *
 ApproachDel: xxxxxxx xxxxxx 36.9
 ApproachLOS: * * * * *

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

 Intersection #1 US 101/Black Bart

Average Delay (sec/veh): 0.8 Worst Case Level Of Service: D [33.6]

Street Name: US 101 Black Bart
 Approach: North Bound South Bound East Bound West Bound
 Movement: L T R L T R L T R L T R L T R L T R
 Control: Uncontrolled Uncontrolled Uncontrolled Uncontrolled
 Rights: Include Include Include Include
 Lanes: 1 0 2 0 0 0 0 2 0 1 0 0 1 1 0 0 0 0 0 0 0

Volume Module:
 Base Vol: 15 914 0 0 895 26 30 0 9 0 0 0 0 0
 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 Initial Bse: 15 914 0 0 895 26 30 0 9 0 0 0 0 0
 User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 PHF Volume: 15 914 0 0 895 26 30 0 9 0 0 0 0 0
 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0
 FinalVolume: 15 914 0 0 895 26 30 0 9 0 0 0 0 0

Critical Gap Module:

Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx xxxxx
 FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx xxxxx

Capacity Module:
 Conflict Vol: 921 xxxxx xxxxx xxxxx xxxxx 1382 1839 448 xxxxx xxxxx xxxxx
 Potent Cap: 750 xxxxx xxxxx xxxxx xxxxx 137 76 564 xxxxx xxxxx xxxxx
 Move Cap: 750 xxxxx xxxxx xxxxx xxxxx 135 75 564 xxxxx xxxxx xxxxx
 Volume/Cap: 0.02 xxxxx xxxxx xxxxx xxxxx 0.22 0.00 0.02 xxxxx xxxxx xxxxx

Level Of Service Module:

2Way5thQ: 0.1 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
 Control Del: 9.9 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
 LOS by Move: A * * * * *
 Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
 Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx 164 xxxxx xxxxx xxxxx xxxxx
 SharedQueue: xxxxx xxxxx xxxxx xxxxx xxxxx 0.9 xxxxx xxxxx xxxxx xxxxx
 Shrd Condel: xxxxx xxxxx xxxxx xxxxx xxxxx 33.6 xxxxx xxxxx xxxxx xxxxx
 Shared LOS: * * * * * D * * * * *
 ApproachDel: xxxxxx xxxxxx 33.6
 ApproachLOS: * * * * *

Note: Queue reported is the number of cars per lane.

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart

Average Delay (sec/veh): 0.5 Worst Case Level of Service: C [23.7]

Street Name: US 101 Black Bart

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Uncontrolled Uncontrolled Uncontrolled Uncontrolled
Rights: Include Include Include Include
Lanes: 1 0 2 0 0 0 2 0 1 0 0 1 1 0 0 0 0 0 0 0 0

Volume Module:
Base Vol: 19 1062 0 0 894 25 13 0 19 0 0 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 19 1062 0 0 894 25 13 0 19 0 0 0
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 19 1062 0 0 894 25 13 0 19 0 0 0
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0
FinalVolume: 19 1062 0 0 894 25 13 0 19 0 0 0

Critical Gap Module:

Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx
FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx

Capacity Module:

Conflict Vol: 919 xxxxx xxxxx xxxxx xxxxx 1463 1994 447 xxxxx xxxxx
Potent Cap.: 751 xxxxx xxxxx xxxxx xxxxx 122 61 564 xxxxx xxxxx
Move Cap.: 751 xxxxx xxxxx xxxxx xxxxx 119 60 564 xxxxx xxxxx

Volume/Cap: 0.03 xxxxx xxxxx xxxxx xxxxx 0.11 0.00 0.03 xxxxx xxxxx

Level of Service Module:

2Way95thQ: 0.1 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Control Del: 9.9 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
LOS by Move: A *
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx 224 xxxxx xxxxx xxxxx xxxxx
SharedQueue: xxxxx xxxxx xxxxx xxxxx xxxxx 0.5 xxxxx xxxxx xxxxx xxxxx
Shrd ConDel: xxxxx xxxxx xxxxx xxxxx xxxxx 23.7 xxxxx xxxxx xxxxx xxxxx
Shared LOS: *
ApproachDel: xxxxxx 23.7
ApproachLOS: *

Note: Queue reported is the number of cars per lane.

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 US 101/Quarry Access

Average Delay (sec/veh): 1.3 Worst Case Level of Service: E [39.3]

Street Name: US 101 Quarry Access

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Uncontrolled Uncontrolled Uncontrolled Uncontrolled
Rights: Include Include Include Include
Lanes: 0 1 1 0 0 0 0 1 1 0 0 0 1 1 0 0 0 0 0 0 0

Volume Module:
Base Vol: 20 1041 0 0 873 40 40 0 20 0 0 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 20 1041 0 0 873 40 40 0 20 0 0 0
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 20 1041 0 0 873 40 40 0 20 0 0 0
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
FinalVolume: 20 1041 0 0 873 40 40 0 20 0 0 0

Critical Gap Module:

Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx
FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx

Capacity Module:

Conflict Vol: 913 xxxxx xxxxx xxxxx xxxxx 1454 1974 457 xxxxx xxxxx
Potent Cap.: 755 xxxxx xxxxx xxxxx xxxxx 123 63 557 xxxxx xxxxx
Move Cap.: 755 xxxxx xxxxx xxxxx xxxxx 121 61 557 xxxxx xxxxx

Volume/Cap: 0.03 xxxxx xxxxx xxxxx xxxxx 0.33 0.00 0.04 xxxxx xxxxx

Level of Service Module:

2Way95thQ: 0.1 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Control Del: 9.9 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
LOS by Move: A *
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx 164 xxxxx xxxxx xxxxx
SharedQueue: 0.1 xxxxx xxxxx xxxxx xxxxx xxxxx 1.6 xxxxx xxxxx xxxxx
Shrd ConDel: 9.9 xxxxx xxxxx xxxxx xxxxx xxxxx 39.3 xxxxx xxxxx xxxxx
Shared LOS: *
ApproachDel: xxxxxx 39.3
ApproachLOS: *

Note: Queue reported is the number of cars per lane.

4:45-5:45pm Peak Hour - Project 2014 w/ Willits Bypass July Peak (Existing Lane) Harris Quarry EIR County of Mendocino

4:45-5:45pm Peak Hour - Project 2014 w/ Willits Bypass July Peak (Existing Lane) Harris Quarry EIR County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)
Intersection #2 US 101/Quarry Access

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)
Intersection #1 US 101/Black Bart

Average Delay (sec/veh): 0.1 Worst Case Level of Service: C [23.4]
Street Name: US 101 Quarry Access
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Average Delay (sec/veh): 0.4 Worst Case Level of Service: C [22.5]
Street Name: US 101 Black Bart
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Volume Module:
Base Vol: 0 1111 0 0 879 0 6 0 6 0 0 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Volume Module:
Base Vol: 20 1097 0 0 860 21 12 0 19 0 0 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Critical Gap Module:
Critical Gap: 6.8 6.5 6.9 xxxxx xxxx xxxx 6.8 6.5 6.9 xxxxx xxxx xxxxx
FollowUpTim: 3.5 4.0 3.3 xxxxx xxxx xxxxx 3.5 4.0 3.3 xxxxx xxxx xxxxx

Critical Gap Module:
Critical Gap: 6.8 6.5 6.9 xxxxx xxxx xxxxx 6.8 6.5 6.9 xxxxx xxxx xxxxx
FollowUpTim: 3.5 4.0 3.3 xxxxx xxxx xxxxx 3.5 4.0 3.3 xxxxx xxxx xxxxx

Level Of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)
Intersection #1 US 101/Black Bart

Average Delay (sec/veh): 0.5 Worst Case Level Of Service: B [14.6]
Street Name: US 101 Black Bart
Approach: North Bound South Bound East Bound West Bound
Movement: L-T-R L-T-R L-T-R L-T-R

Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
Rights: Include Include Include Include
Lanes: 1 0 2 0 0 0 2 0 1 0 0 1 1 0 0 0 0 0 0 0

Volume Module:
Base Vol: 9 493 0 0 561 7 19 0 13 0 0 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Critical Gap Module:
Critical Gap: 4.1 xxxxxx xxxxxx xxxxxx xxxxxx 6.8 6.5 6.9 xxxxxx xxxxxx xxxxxx
FollowUpTim: 2.2 xxxxxx xxxxxx xxxxxx xxxxxx 3.5 4.0 3.3 xxxxxx xxxxxx xxxxxx

Level Of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)
Intersection #2 US 101/Quarry Access

Average Delay (sec/veh): 1.8 Worst Case Level Of Service: C [17.7]
Street Name: US 101 Quarry Access
Approach: North Bound South Bound East Bound West Bound
Movement: L-T-R L-T-R L-T-R L-T-R

Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
Rights: Include Include Include Include
Lanes: 0 1 1 0 0 0 0 1 1 0 0 0 1 1 0 0 0 0 0 0

Volume Module:
Base Vol: 36 438 0 0 510 64 64 0 36 0 0 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Critical Gap Module:
Critical Gap: 4.1 xxxxxx xxxxxx xxxxxx xxxxxx 6.8 6.5 6.9 xxxxxx xxxxxx xxxxxx
FollowUpTim: 2.2 xxxxxx xxxxxx xxxxxx xxxxxx 3.5 4.0 3.3 xxxxxx xxxxxx xxxxxx

Capacity Module:
Conflict Vol: 574 xxxxx xxxxxx xxxxx xxxxx xxxxxx 833 1052 287 xxxxx xxxxx xxxxxx
Potent Cap.: 1009 xxxxx xxxxxx xxxxx xxxxx xxxxxx 311 228 716 xxxxx xxxxx xxxxxx

11-12pm Peak Hour - Project 2014 w/ Willits Bypass October Peak (Existing Lanes
Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)
Intersection #1 US 101/Black Bart

Average Delay (sec/veh): 0.7 Worst Case Level of Service: C [22.5]

Street Name: US 101 Black Bart
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
Rights: Include Include Include Include
Lanes: 1 0 2 0 0 0 2 0 1 0 0 1 0 0 0 0 0 0 0

Volume Module:

Table with traffic volume and capacity data for various approaches and movements.

Critical Gap Module:

Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx

Capacity Module:

Conflict Vol: 738 xxxxx xxxxx xxxxx xxxxx 1105 1467 356 xxxxx xxxxx xxxxx
Potential Cap.: 877 xxxxx xxxxx xxxxx xxxxx 208 129 646 xxxxx xxxxx xxxxx
Move Cap.: 877 xxxxx xxxxx xxxxx xxxxx 206 127 646 xxxxx xxxxx xxxxx
Volume/Cap: 0.02 xxxxx xxxxx xxxxx xxxxx 0.15 0.00 0.01 xxxxx xxxxx xxxxx

11-12pm Peak Hour - Project 2014 w/ Willits Bypass October Peak (Existing Lanes
Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)
Intersection #2 US 101/Quarry Access

Average Delay (sec/veh): 1.9 Worst Case Level of Service: D [26.0]

Street Name: US 101 Quarry Access
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
Rights: Include Include Include Include
Lanes: 0 1 1 0 0 0 0 1 1 0 0 0 0 1 0 0 0 0 0 0

Volume Module:

Table with traffic volume and capacity data for various approaches and movements.

Critical Gap Module:

Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx

Capacity Module:

Conflict Vol: 721 xxxxx xxxxx xxxxx xxxxx 1099 1437 361 xxxxx xxxxx xxxxx
Potential Cap.: 890 xxxxx xxxxx xxxxx xxxxx 210 135 642 xxxxx xxxxx xxxxx
Move Cap.: 890 xxxxx xxxxx xxxxx xxxxx 203 129 642 xxxxx xxxxx xxxxx
Volume/Cap: 0.04 xxxxx xxxxx xxxxx xxxxx 0.31 0.00 0.06 xxxxx xxxxx xxxxx

Traffic 7.9.0415 (c) 2007 Dowling Assoc. Licensed to W-TRANS, Santa Rosa, CA

1:15-2:15pm Peak Hour - Project 2014 w/o Willits Bypass July Peak (Existing Lane) Harris Quarry EIR County of Mendocino

Level Of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart
Average Delay (sec/veh): 0.5 Worst Case Level Of Service: C [23.2]

Table with columns: Street Name, Approach, Movement, Control, Rights, Lanes, Volume Module, Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Volume/Cap, Level of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap, SharedQueue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS.

1:15-2:15pm Peak Hour - Project 2014 w/o Willits Bypass July Peak (Existing Lane) Harris Quarry EIR County of Mendocino

Level Of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart
Average Delay (sec/veh): 0.5 Worst Case Level Of Service: C [23.2]

Table with columns: Street Name, Approach, Movement, Control, Rights, Lanes, Volume Module, Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Volume/Cap, Level of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap, SharedQueue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS.

1:15-2:15pm Peak Hour - Project 2014 w/o Willits Bypass July Peak (Existing Lane) Harris Quarry EIR County of Mendocino

Level Of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 US 101/Quarry Access
Average Delay (sec/veh): 1.1 Worst Case Level Of Service: D [30.3]

Table with columns: Street Name, Approach, Movement, Control, Rights, Lanes, Volume Module, Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Volume/Cap, Level of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap, SharedQueue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS.

11-12pm Peak Hour - Project 2014 w/o Willits Bypass October Peak (Existing Lane Harris Quarry EIR County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)
Intersection #1 US 101/Black Bart

Average Delay (sec/veh): 0.7 Worst Case Level of Service: C [21.7]

Table with columns: Street Name, Approach, Movement, Control, Rights, Lanes, Volume Module, Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Volume/Cap, Level of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap, SharedQueue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

11-12pm Peak Hour - Project 2014 w/o Willits Bypass October Peak (Existing Lane Harris Quarry EIR County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)
Intersection #2 US 101/Quarry Access

Average Delay (sec/veh): 1.7 Worst Case Level of Service: C [21.6]

Table with columns: Street Name, Approach, Movement, Control, Rights, Lanes, Volume Module, Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Volume/Cap, Level of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap, SharedQueue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

1:15-2:15pm Peak Hour - Project 2014 w/o Willits Bypass October Peak (Existing)
Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart
Average Delay (sec/veh): 0.7 Worst Case Level of Service: C [20.6]

Table with columns: Street Name, Approach, Movement, Control, Rights, Lanes, Volume Module, Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Volume/Cap, Level of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap, SharedQueue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

1:15-2:15pm Peak Hour - Project 2014 w/o Willits Bypass October Peak (Existing)
Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 US 101/Quarry Access
Average Delay (sec/veh): 1.2 Worst Case Level of Service: C [20.3]

Table with columns: Street Name, Approach, Movement, Control, Rights, Lanes, Volume Module, Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Volume/Cap, Level of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap, SharedQueue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)
Intersection #1 US 101/Black Bart

Average Delay (sec/veh): 0.4 Worst Case Level Of Service: C [16.2]

Street Name: US 101 Black Bart

Approach:	North Bound	South Bound	East Bound	West Bound											
Movement:	L - T - R	L - T - R	L - T - R	L - T - R	L	T	R	L	T	R					
Control:	Uncontrolled														
Rights:	Uncontrolled Include					Stop Sign Include									
Lanes:	1	0	2	0	0	0	2	0	1	0	0	0	0	0	0

Volume Module:

Base Vol:	20	839	0	0	656	21	12	0	19	0	0	0	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	20	839	0	0	656	21	12	0	19	0	0	0	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	20	839	0	0	656	21	12	0	19	0	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	20	839	0	0	656	21	12	0	19	0	0	0	0	0	0

Critical Gap Module:

Critical Gap:	4.1	xxxx	xxxx	xxxx	xxxx	xxxx	6.8	6.5	6.9	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx
FollowUpTim:	2.2	xxxx	xxxx	xxxx	xxxx	xxxx	3.5	4.0	3.3	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx

Capacity Module:

Conflict Vol:	677	xxxx	xxxx	xxxx	xxxx	xxxx	1116	1535	328	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx
Potent Cap.:	924	xxxx	xxxx	xxxx	xxxx	xxxx	205	117	674	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx
Move Cap.:	924	xxxx	xxxx	xxxx	xxxx	xxxx	202	115	674	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx
Volume/Cap:	0.02	xxxx	xxxx	xxxx	xxxx	xxxx	0.06	0.00	0.03	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx

Level Of Service Module:

2Way95thQ:	0.1	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx
Control Del:	9.0	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx
LOS by Move:	A	*	*	*	*	*	*	*	*	*	*	*	*	*	*
Movement:	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT
Shared Cap.:	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	353	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx
SharedQueue:	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	0.3	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx
Shrd ConDel:	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	16.2	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx
Shared LOS:	A	*	*	*	*	*	C	*	*	*	*	*	*	*	*
ApproachDel:	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	16.2	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx
ApproachLOS:	*	*	*	*	*	*	C	*	*	*	*	*	*	*	*

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)
Intersection #2 US 101/Quarry Access

Average Delay (sec/veh): 0.1 Worst Case Level Of Service: C [16.8]

Street Name: US 101 Quarry Access

Approach:	North Bound	South Bound	East Bound	West Bound														
Movement:	L - T - R	L - T - R	L - T - R	L - T - R	L	T	R	L	T	R								
Control:	Uncontrolled																	
Rights:	Uncontrolled Include					Stop Sign Include												
Lanes:	0	1	1	0	0	0	1	1	0	0	0	1	1	0	0	0	0	0

Volume Module:

Base Vol:	0	853	0	0	675	0	6	0	6	0	0	6	0	0	0	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	853	0	0	675	0	6	0	6	0	0	6	0	0	0	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	853	0	0	675	0	6	0	6	0	0	6	0	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	853	0	0	675	0	6	0	6	0	0	6	0	0	0	0	0	0

Critical Gap Module:

Critical Gap:	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	6.8	6.5	6.9	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx
FollowUpTim:	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	3.5	4.0	3.3	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx

Capacity Module:

Conflict Vol:	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	1102	1528	338	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx
Potent Cap.:	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	209	119	664	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx
Move Cap.:	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	209	119	664	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx
Volume/Cap:	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	0.03	0.00	0.01	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx

Level Of Service Module:

2Way95thQ:	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx
Control Del:	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx
LOS by Move:	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*
Movement:	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT
Shared Cap.:	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	318	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx
SharedQueue:	0.0	xxxx	xxxx	xxxx	xxxx	xxxx	0.1	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx
Shrd ConDel:	9.0	xxxx	xxxx	xxxx	xxxx	xxxx	16.8	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx
Shared LOS:	A	*	*	*	*	*	C	*	*	*	*	*	*	*	*
ApproachDel:	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	16.8	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx
ApproachLOS:	*	*	*	*	*	*	C	*	*	*	*	*	*	*	*

Note: Queue reported is the number of cars per lane.

11-12pm Peak Hour - Project 2030 July Peak (Existing Lanes)
 Harris Quarry EIR
 County of Mendocino

Level of Service Computation Report
 2000 HCM Unsignalized Method (Base Volume Alternative)
 Intersection #1 US 101/Black Bart

Average Delay (sec/veh): 1.6 Worst Case Level of Service: F [72.6]
 Street Name: US 101 Black Bart
 Approach: North Bound South Bound East Bound West Bound
 Movement: L - T - R L - T - R L - T - R L - T - R

Control:	Uncontrolled	Uncontrolled	Uncontrolled	Uncontrolled	Stop Sign	Stop Sign	Stop Sign	Stop Sign
Rights:	Include	Include	Include	Include	Include	Include	Include	Include
Lanes:	1 0 2 0 0	0 0 2 0 1	0 0 1 1 0	0 0 1 1 0	0 0 0 0 0	0 0 0 0 0	0 0 0 0 0	0 0 0 0 0

Volume Module:

Base Vol:	20 1137	0	0 1113	33	38	0	12	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	20 1137	0	0 1113	33	38	0	12	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	20 1137	0	0 1113	33	38	0	12	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0
Final Volume:	20 1137	0	0 1113	33	38	0	12	0	0

Critical Gap Module:

Critical Gap:	4.1	xxxx	xxxx	xxxx	xxxx	6.8	6.5	6.9	xxxx	xxxx	xxxx
FollowUpTim:	2.2	xxxx	xxxx	xxxx	xxxx	3.5	4.0	3.3	xxxx	xxxx	xxxx

Capacity Module:
 Conflict Vol: 1146
 Potent Cap.: 617
 Move Cap.: 0.03
 Volume/Cap: 0.03

Level of Service Module:
 2Way95thQ: 0.1
 Control Del: 11.0
 LOS by Move: B * * * * *
 Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
 Shared Cap.: xxxx
 SharedQueue: xxxx
 Shrd ConDel: xxxx
 Shared LOS: * * * * *
 ApproachDel: xxxxxx
 ApproachLOS: * * * * *

Note: Queue reported is the number of cars per lane.

11-12pm Peak Hour - Project 2030 July Peak (Existing Lanes)
 Harris Quarry EIR
 County of Mendocino

Level of Service Computation Report
 2000 HCM Unsignalized Method (Base Volume Alternative)
 Intersection #2 US 101/Quarry Access

Average Delay (sec/veh): 2.0 Worst Case Level of Service: F [55.4]
 Street Name: US 101 Quarry Access
 Approach: North Bound South Bound East Bound West Bound
 Movement: L - T - R L - T - R L - T - R L - T - R

Control:	Uncontrolled	Uncontrolled	Uncontrolled	Uncontrolled	Stop Sign	Stop Sign	Stop Sign	Stop Sign
Rights:	Include	Include	Include	Include	Include	Include	Include	Include
Lanes:	0 1 1 0 0	0 0 1 1 0	0 0 1 1 0	0 0 1 1 0	0 0 0 0 0	0 0 0 0 0	0 0 0 0 0	0 0 0 0 0

Volume Module:

Base Vol:	44 1125	0	0 1093	32	32	0	44	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	44 1125	0	0 1093	32	32	0	44	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	44 1125	0	0 1093	32	32	0	44	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0
Final Volume:	44 1125	0	0 1093	32	32	0	44	0	0

Critical Gap Module:

Critical Gap:	4.1	xxxx	xxxx	xxxx	xxxx	6.8	6.5	6.9	xxxx	xxxx	xxxx
FollowUpTim:	2.2	xxxx	xxxx	xxxx	xxxx	3.5	4.0	3.3	xxxx	xxxx	xxxx

Capacity Module:
 Conflict Vol: 1125
 Potent Cap.: 628
 Move Cap.: 0.07
 Volume/Cap: 0.07

Level of Service Module:
 2Way95thQ: 0.2
 Control Del: 11.2
 LOS by Move: B * * * * *
 Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
 Shared Cap.: xxxx
 SharedQueue: 0.2
 Shrd ConDel: 11.2
 Shared LOS: * * * * *
 ApproachDel: xxxxxx
 ApproachLOS: * * * * *

Note: Queue reported is the number of cars per lane.

1:15-2:15pm Peak Hour - Project 2030 July Peak (Existing Lanes)
Harris Quarry EIR
County of Mendocino

Level Of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart

Average Delay (sec/veh): 0.8 Worst Case Level of Service: E[42.9]

Street Name: US 101 Black Bart
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Uncontrolled Uncontrolled Uncontrolled Uncontrolled
Rights: Include Include Include Include
Lanes: 1 0 2 0 0 0 2 0 1 0 0 1 0 0 0 0 0 0 0 0
Volume Module:
Base Vol: 24 1333 0 0 1118 32 17 0 24 0 0 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 24 1333 0 0 1118 32 17 0 24 0 0 0
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 24 1333 0 0 1118 32 17 0 24 0 0 0
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
FinalVolume: 24 1333 0 0 1118 32 17 0 24 0 0 0

Critical Gap Module:
Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx xxxxx

FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx xxxxx

Capacity Module:
Conflict Vol: 1150 xxxxx xxxxx xxxxx xxxxx 1833 2499 559 xxxxx xxxxx xxxxx
Potent Cap.: 615 xxxxx xxxxx xxxxx xxxxx 69 29 478 xxxxx xxxxx xxxxx
Move Cap.: 615 xxxxx xxxxx xxxxx xxxxx 67 28 478 xxxxx xxxxx xxxxx
Volume/Cap: 0.04 xxxxx xxxxx xxxxx xxxxx 0.25 0.00 0.05 xxxxx xxxxx xxxxx

Level Of Service Module:
2Way95thQ: 0.1 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Control Del: 11.1 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
LOS by Move: B * * * * *
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
SharedQueue: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 1.2 xxxxx xxxxx xxxxx
Shrd Condel: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 42.9 xxxxx xxxxx xxxxx
Shared LOS: * * * * * E * * * * *
ApproachDel: xxxxxx 42.9 xxxxxxx
ApproachLOS: * * * * *

Note: Queue reported is the number of cars per lane.

1:15-2:15pm Peak Hour - Project 2030 July Peak (Existing Lanes)
Harris Quarry EIR
County of Mendocino

Level Of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 US 101/Quarry Access

Average Delay (sec/veh): 1.6 Worst Case Level of Service: F[63.0]

Street Name: US 101 Quarry Access
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Uncontrolled Uncontrolled Uncontrolled Uncontrolled
Rights: Include Include Include Include
Lanes: 0 1 1 0 0 0 0 1 1 0 0 0 1 0 0 0 0 0 0 0 0
Volume Module:
Base Vol: 32 1329 0 0 1114 28 28 0 32 0 0 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 32 1329 0 0 1114 28 28 0 32 0 0 0
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 32 1329 0 0 1114 28 28 0 32 0 0 0
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
FinalVolume: 32 1329 0 0 1114 28 28 0 32 0 0 0

Critical Gap Module:
Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx xxxxx

FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx xxxxx

Capacity Module:
Conflict Vol: 1142 xxxxx xxxxx xxxxx xxxxx 1857 2521 571 xxxxx xxxxx xxxxx
Potent Cap.: 619 xxxxx xxxxx xxxxx xxxxx 67 28 469 xxxxx xxxxx xxxxx
Move Cap.: 619 xxxxx xxxxx xxxxx xxxxx 64 27 469 xxxxx xxxxx xxxxx
Volume/Cap: 0.05 xxxxx xxxxx xxxxx xxxxx 0.44 0.00 0.07 xxxxx xxxxx xxxxx

Level Of Service Module:
2Way95thQ: 0.2 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Control Del: 11.1 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
LOS by Move: B * * * * *
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 119 xxxxx xxxxx xxxxx
SharedQueue: 0.2 xxxxx xxxxx xxxxx xxxxx xxxxx 2.3 xxxxx xxxxx xxxxx
Shrd Condel: 11.1 xxxxx xxxxx xxxxx xxxxx xxxxx 63.0 xxxxx xxxxx xxxxx
Shared LOS: B * * * * * F * * * * *
ApproachDel: xxxxxxx 63.0 xxxxxxx
ApproachLOS: * * * * *

Note: Queue reported is the number of cars per lane.

4:45-5:45pm Peak Hour - Project 2030 July Peak (Existing Lanes)

Harris Quarry EIR

County of Mendocino

Level Of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)
Intersection #1 US 101/Black Bart

Average Delay (sec/veh): 0.7 Worst Case Level Of Service: E[40.5]

Street Name: US 101 Black Bart

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
Rights: Include Include Include Include
Lanes: 1 0 2 0 0 0 2 0 1 0 0 1 0 0 0 0 0 0 0

Volume Module:
Base Vol: 26 1398 0 0 1098 27 15 0 24 0 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 26 1398 0 0 1098 27 15 0 24 0 0
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 26 1398 0 0 1098 27 15 0 24 0 0
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0
FinalVolume: 26 1398 0 0 1098 27 15 0 24 0 0

Critical Gap Module:
Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx xxxxx

FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx xxxxx

Capacity Module:
Conflict Vol: 1125 xxxxx xxxxx xxxxx xxxxx 1849 2548 549 xxxxx xxxxx xxxxx
Potent Cap.: 628 xxxxx xxxxx xxxxx xxxxx 67 27 485 xxxxx xxxxx xxxxx
Move Cap.: 628 xxxxx xxxxx xxxxx xxxxx 65 26 485 xxxxx xxxxx xxxxx
Volume/Cap: 0.04 xxxxx xxxxx xxxxx xxxxx 0.23 0.00 0.05 xxxxx xxxxx xxxxx

Level Of Service Module:
2Way95thQ: 0.1 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx

Control Del: 11.0 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx

LOS by Move: B *

Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT

Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 140 xxxxx xxxxx xxxxx xxxxx

SharedQueue: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 1.1 xxxxx xxxxx xxxxx xxxxx

Shrd Condel: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 40.5 xxxxx xxxxx xxxxx xxxxx

Shared LOS: *

ApproachDel: xxxxxx 40.5 xxxxxx

ApproachLOS: *

Note: Queue reported is the number of cars per lane.

4:45-5:45pm Peak Hour - Project 2030 July Peak (Existing Lanes)

Harris Quarry EIR

County of Mendocino

Level Of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)
Intersection #2 US 101/Quarry Access

Average Delay (sec/veh): 0.2 Worst Case Level Of Service: E[38.0]

Street Name: US 101 Quarry Access

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
Rights: Include Include Include Include
Lanes: 0 1 1 0 0 0 0 1 1 0 0 0 1 1 0 0 0 0 0 0 0

Volume Module:
Base Vol: 0 1418 0 0 1122 0 6 0 0 0 0 0 6 0 0 0 0 0 0 0 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 0 1418 0 0 1122 0 6 0 0 0 0 0 6 0 0 0 0 0 0 0 0
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 0 1418 0 0 1122 0 6 0 0 0 0 0 6 0 0 0 0 0 0 0 0
Reduct Vol: 0
FinalVolume: 0 1418 0 0 1122 0 6 0 0 0 0 0 6 0 0 0 0 0 0 0 0

Critical Gap Module:
Critical Gap: xxxxx xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx xxxxx

FollowUpTim: xxxxx xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx xxxxx

Capacity Module:
Conflict Vol: xxxxx xxxxx xxxxx xxxxx xxxxx 1831 2540 561 xxxxx xxxxx xxxxx
Potent Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx 69 28 476 xxxxx xxxxx xxxxx
Move Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx 69 28 476 xxxxx xxxxx xxxxx
Volume/Cap: xxxxx xxxxx xxxxx xxxxx xxxxx 0.09 0.00 0.01 xxxxx xxxxx xxxxx

Level Of Service Module:
2Way95thQ: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx

Control Del: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx

LOS by Move: *

Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT

Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx 121 xxxxx xxxxx xxxxx xxxxx

SharedQueue: 0.0 xxxxx xxxxx xxxxx xxxxx xxxxx 0.3 xxxxx xxxxx xxxxx xxxxx

Shrd Condel: 9.0 xxxxx xxxxx xxxxx xxxxx xxxxx 38.0 xxxxx xxxxx xxxxx xxxxx

Shared LOS: A *

ApproachDel: xxxxxx 38.0 xxxxxx

ApproachLOS: *

Note: Queue reported is the number of cars per lane.

11-12pm Peak Hour - Project 2030 October Peak (Existing Lanes)

Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
 2000 HCM Unsignalized Method (Base Volume Alternative)
 Intersection #1 US 101/Black Bart

Average Delay (sec/veh): 1.0 Worst Case Level of Service: D[34.4]

Street Name: US 101 Black Bart

Approach: North Bound South Bound East Bound West Bound
 Movement: L T R L T R L T R L T R

Control: Uncontrolled Uncontrolled Uncontrolled Uncontrolled
 Rights: Include Include Include Include
 Lanes: 1 0 2 0 0 0 2 0 1 0 0 1 0 0 0 0 0 0 0 0

Volume Module:
 Base Vol: 20 887 0 0 871 33 38 0 12 0 0
 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 Initial Bse: 20 887 0 0 871 33 38 0 12 0 0
 User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 PHF Volume: 20 887 0 0 871 33 38 0 12 0 0
 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0
 FinalVolume: 20 887 0 0 871 33 38 0 12 0 0

Critical Gap Module:

Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx xxxxx
 FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx xxxxx

Capacity Module:
 Conflict Vol: 904 xxxxx xxxxx xxxxx xxxxx 1355 1798 436 xxxxx xxxxx xxxxx
 Potent Cap.: 761 xxxxx xxxxx xxxxx xxxxx 143 81 574 xxxxx xxxxx xxxxx
 Move Cap.: 761 xxxxx xxxxx xxxxx xxxxx 140 79 574 xxxxx xxxxx xxxxx
 Volume/Cap: 0.03 xxxxx xxxxx xxxxx xxxxx 0.27 0.00 0.02 xxxxx xxxxx xxxxx

Level of Service Module:

2Way95thQ: 0.1 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
 Control Del: 9.9 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx

LOS by Move: A * * * * *
 Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
 Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx 171 xxxxx xxxxx xxxxx xxxxx
 SharedQueue: xxxxx xxxxx xxxxx xxxxx xxxxx 1.1 xxxxx xxxxx xxxxx xxxxx
 Shrd ConDel: xxxxx xxxxx xxxxx xxxxx xxxxx 34.4 xxxxx xxxxx xxxxx xxxxx
 Shared LOS: * * * * * D * * * * *
 ApproachDel: xxxxxx xxxxxx 34.4 xxxxxxx
 ApproachLOS: * * * * *

Note: Queue reported is the number of cars per lane.

11-12pm Peak Hour - Project 2030 October Peak (Existing Lanes)

Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
 2000 HCM Unsignalized Method (Base Volume Alternative)
 Intersection #2 US 101/Quarry Access

Average Delay (sec/veh): 2.1 Worst Case Level of Service: D[33.8]

Street Name: US 101 Quarry Access

Approach: North Bound South Bound East Bound West Bound
 Movement: L T R L T R L T R L T R

Control: Uncontrolled Uncontrolled Uncontrolled Uncontrolled
 Rights: Include Include Include Include
 Lanes: 0 1 1 0 0 0 0 1 1 0 0 0 1 0 0 0 0 0 0 0

Volume Module:
 Base Vol: 56 863 0 0 839 44 44 0 56 0 0
 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 Initial Bse: 56 863 0 0 839 44 44 0 56 0 0
 User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 PHF Volume: 56 863 0 0 839 44 44 0 56 0 0
 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0
 FinalVolume: 56 863 0 0 839 44 44 0 56 0 0

Critical Gap Module:

Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx xxxxx
 FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx xxxxx

Capacity Module:
 Conflict Vol: 883 xxxxx xxxxx xxxxx xxxxx 1405 1836 442 xxxxx xxxxx xxxxx
 Potent Cap.: 775 xxxxx xxxxx xxxxx xxxxx 133 77 569 xxxxx xxxxx xxxxx
 Move Cap.: 775 xxxxx xxxxx xxxxx xxxxx 125 71 569 xxxxx xxxxx xxxxx
 Volume/Cap: 0.07 xxxxx xxxxx xxxxx xxxxx 0.35 0.00 0.10 xxxxx xxxxx xxxxx

Level of Service Module:

2Way95thQ: 0.2 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
 Control Del: 10.0 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx

LOS by Move: B * * * * *
 Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
 Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx 222 xxxxx xxxxx xxxxx xxxxx
 SharedQueue: 0.2 xxxxx xxxxx xxxxx xxxxx xxxxx 2.1 xxxxx xxxxx xxxxx xxxxx
 Shrd ConDel: 10.0 xxxxx xxxxx xxxxx xxxxx xxxxx 33.8 xxxxx xxxxx xxxxx xxxxx
 Shared LOS: B * * * * * D * * * * *
 ApproachDel: xxxxxxx xxxxxx 33.8 xxxxxxx
 ApproachLOS: * * * * *

Note: Queue reported is the number of cars per lane.

1:15-2:15pm Peak Hour - Project 2030 October Peak (Existing Lanes)
Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)
Intersection #1 US 101/Black Bart

Average Delay (sec/veh): 1.1 Worst Case Level of Service: E [40.0]

Street Name: US 101 Black Bart
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Uncontrolled Uncontrolled Uncontrolled Stop Sign
Rights: Include Include Include Include
Lanes: 1 0 2 0 0 0 2 0 1 0 0 1 0 0 0 0 0 0 0 0

Volume Module:
Base Vol: 24 1029 0 0 876 33 38 0 12 0 0 0 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Critical Gap Module:
Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx xxxxx
FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx xxxxx

Capacity Module:
Conflict Vol: 909 xxxxx xxxxx xxxxx xxxxx 1439 1953 438 xxxxx xxxxx xxxxx
Potential Cap.: 757 xxxxx xxxxx xxxxx xxxxx 126 65 572 xxxxx xxxxx xxxxx
Move Cap.: 757 xxxxx xxxxx xxxxx xxxxx 123 63 572 xxxxx xxxxx xxxxx
Volume/Cap: 0.03 xxxxx xxxxx xxxxx xxxxx 0.31 0.00 0.02 xxxxx xxxxx xxxxx

Level of Service Module:
2Way95thQ: 0.1 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Control Del: 9.9 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
LOS by Move: A *

Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx 152 xxxxx xxxxx xxxxx xxxxx
SharedQueue: xxxxx xxxxx xxxxx xxxxx xxxxx 1.3 xxxxx xxxxx xxxxx xxxxx
Shrd Condel: xxxxx xxxxx xxxxx xxxxx xxxxx 40.0 xxxxx xxxxx xxxxx xxxxx
Shared LOS: *

1:15-2:15pm Peak Hour - Project 2030 October Peak (Existing Lanes)
Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)
Intersection #2 US 101/Quarry Access

Average Delay (sec/veh): 1.4 Worst Case Level of Service: D [30.9]

Street Name: US 101 Quarry Access
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Uncontrolled Uncontrolled Uncontrolled Stop Sign
Rights: Include Include Include Include
Lanes: 0 1 1 0 0 0 0 1 1 0 0 0 1 1 0 0 0 0 0 0 0

Volume Module:
Base Vol: 44 1021 0 0 856 32 32 0 44 0 0 0 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Critical Gap Module:
Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx xxxxx
FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx xxxxx

Capacity Module:
Conflict Vol: 888 xxxxx xxxxx xxxxx xxxxx 1471 1981 444 xxxxx xxxxx xxxxx
Potential Cap.: 771 xxxxx xxxxx xxxxx xxxxx 120 62 567 xxxxx xxxxx xxxxx
Move Cap.: 771 xxxxx xxxxx xxxxx xxxxx 115 59 567 xxxxx xxxxx xxxxx
Volume/Cap: 0.06 xxxxx xxxxx xxxxx xxxxx 0.28 0.00 0.08 xxxxx xxxxx xxxxx

Level of Service Module:
2Way95thQ: 0.2 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Control Del: 10.0 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
LOS by Move: A *

Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx 213 xxxxx xxxxx xxxxx xxxxx
SharedQueue: 0.2 xxxxx xxxxx xxxxx xxxxx xxxxx 1.5 xxxxx xxxxx xxxxx xxxxx
Shrd Condel: 10.0 xxxxx xxxxx xxxxx xxxxx 30.9 xxxxx xxxxx xxxxx xxxxx
Shared LOS: A *

4:45-5:45pm Peak Hour - Project 2030 October Peak (Existing Lanes)

Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
 2000 HCM Unsignalized Method (Base Volume Alternative)
 Intersection #1 US 101/Black Bart

Average Delay (sec/veh): 0.6 Worst Case Level of Service: C [22.6]
 Street Name: US 101 Black Bart

Approach: North Bound South Bound East Bound West Bound
 Movement: L - T - R L - T - R L - T - R L - T - R
 Control: Uncontrolled Uncontrolled Uncontrolled Uncontrolled
 Rights: Include Include Include Include
 Lanes: 1 0 2 0 0 0 2 0 1 0 0 1 0 0 0 0 0 0 0

Volume Module:
 Base Vol: 26 1069 0 0 838 27 15 0 24 0 0
 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 Initial Bse: 26 1069 0 0 838 27 15 0 24 0 0
 User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 PHF Volume: 26 1069 0 0 838 27 15 0 24 0 0
 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0
 FinalVolume: 26 1069 0 0 838 27 15 0 24 0 0

Critical Gap Module:

Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx
 FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx

Capacity Module:

Conflict Vol: 865 xxxxx xxxxx xxxxx xxxxx 1425 1959 419 xxxxx xxxxx
 Potent Cap.: 787 xxxxx xxxxx xxxxx xxxxx 129 64 589 xxxxx xxxxx
 Move Cap.: 787 xxxxx xxxxx xxxxx xxxxx 126 62 589 xxxxx xxxxx
 Volume/Cap: 0.03 xxxxx xxxxx xxxxx xxxxx 0.112 0.00 0.04 xxxxx xxxxx

Level of Service Module:

2Way95thQ: 0.1 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
 Control Del: 9.7 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
 LOS by Move: A * * * * *
 Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
 Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx 243 xxxxx xxxxx xxxxx
 SharedQueue: xxxxx xxxxx xxxxx xxxxx xxxxx 0.6 xxxxx xxxxx xxxxx
 Shrd ConDel: xxxxx xxxxx xxxxx xxxxx xxxxx 22.6 xxxxx xxxxx xxxxx
 Shared LOS: * * * * * C
 ApproachDel: xxxxxx 22.6
 ApproachLOS: * * xxxxxxx

Note: Queue reported is the number of cars per lane.

4:45-5:45pm Peak Hour - Project 2030 October Peak (Existing Lanes)

Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
 2000 HCM Unsignalized Method (Base Volume Alternative)
 Intersection #2 US 101/Quarry Access

Average Delay (sec/veh): 0.1 Worst Case Level of Service: C [22.7]
 Street Name: US 101 Quarry Access

Approach: North Bound South Bound East Bound West Bound
 Movement: L - T - R L - T - R L - T - R L - T - R
 Control: Uncontrolled Uncontrolled Uncontrolled Uncontrolled
 Rights: Include Include Include Include
 Lanes: 0 1 1 0 0 0 0 1 1 0 0 0 1 1 0 0 0 0 0 0

Volume Module:
 Base Vol: 0 1089 0 0 862 0 6 0 6 0 0 0
 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 Initial Bse: 0 1089 0 0 862 0 6 0 6 0 0
 User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 PHF Volume: 0 1089 0 0 862 0 6 0 6 0 0
 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0
 FinalVolume: 0 1089 0 0 862 0 6 0 6 0 0

Critical Gap Module:

Critical Gap: xxxxx xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx
 FollowUpTim: xxxxx xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx

Capacity Module:

Conflict Vol: xxxxx xxxxx xxxxx xxxxx xxxxx 1407 1951 431 xxxxx xxxxx
 Potent Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx 132 65 578 xxxxx xxxxx
 Move Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx 132 65 578 xxxxx xxxxx
 Volume/Cap: xxxxx xxxxx xxxxx xxxxx xxxxx 0.05 0.00 0.01 xxxxx xxxxx

Level of Service Module:

2Way95thQ: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
 Control Del: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
 LOS by Move: * * * * *
 Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
 Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx 216 xxxxx 0-2 xxxxx xxxxx
 SharedQueue: 0.0 xxxxx xxxxx xxxxx xxxxx xxxxx 0.2 xxxxx xxxxx
 Shrd ConDel: 9.0 xxxxx xxxxx xxxxx xxxxx 22.7 xxxxx xxxxx xxxxx
 Shared LOS: A * * * * * C
 ApproachDel: xxxxxxx 22.7
 ApproachLOS: * * xxxxxxx

Note: Queue reported is the number of cars per lane.

11-12pm Peak Hour - Project 2040 July Peak (Existing Lanes)
Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)
Intersection #1 US 101/Black Bart

Average Delay (sec/veh): 3.3 Worst Case Level of Service: F[151.9]

Table with columns for Street Name (US 101), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L-T-R, L-T-R, L-T-R), Control (Uncontrolled, Uncontrolled, Uncontrolled), Rights (Include, Include, Include), Lanes (1 0 2 0 0, 0 0 2 0 1, 0 0 1 1 0), Volume Module (Base Vol: 22 1287, Growth Adj: 1.00, Initial Bse: 22 1287, User Adj: 1.00, PHF Adj: 1.00, PHF Volume: 22 1287, Reduct Vol: 0, Final Volume: 22 1287), Critical Gap Module (Critical Gap: 4.1, FollowUpTim: 2.2), Capacity Module (Conflict Vol: 1297, Potent Cap.: 541, Move Cap.: 541, Volume/Cap: 0.04), Level of Service Module (2Way5thQ: 0.1, Control Del: 11.9, LOS by Move: B, Movement: LT-LTR-RT, Shared Cap.: xxxxx, Shrd ConDel: xxxxx, Shared LOS: B, ApproachDel: xxxxxx, ApproachLOS: B), Note: Queue reported is the number of cars per lane.

11-12pm Peak Hour - Project 2040 July Peak (Existing Lanes)
Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)
Intersection #2 US 101/Quarry Access

Average Delay (sec/veh): 3.1 Worst Case Level of Service: F[100.6]

Table with columns for Street Name (US 101), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L-T-R, L-T-R, L-T-R), Control (Uncontrolled, Uncontrolled, Uncontrolled), Rights (Include, Include, Include), Lanes (0 1 1 0 0, 0 0 1 1 0, 0 0 1 1 0), Volume Module (Base Vol: 44 1277, Growth Adj: 1.00, Initial Bse: 44 1277, User Adj: 1.00, PHF Adj: 1.00, PHF Volume: 44 1277, Reduct Vol: 0, Final Volume: 44 1277), Critical Gap Module (Critical Gap: 4.1, FollowUpTim: 2.2), Capacity Module (Conflict Vol: 1273, Potent Cap.: 552, Move Cap.: 552, Volume/Cap: 0.08), Level of Service Module (2Way5thQ: 0.3, Control Del: 12.1, LOS by Move: B, Movement: LT-LTR-RT, Shared Cap.: xxxxx, Shrd ConDel: 12.1, Shared LOS: B, ApproachDel: xxxxxx, ApproachLOS: B), Note: Queue reported is the number of cars per lane.

9-10am Peak Hour - Project 2040 October Peak (Existing Lanes)
Harris Quarry EIR
County of Mendocino

Level Of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)
Intersection #1 US 101/Black Bart

Average Delay (sec/veh): 0.7 Worst Case Level Of Service: C [20.6]

Street Name: US 101 Black Bart
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Uncontrolled Uncontrolled Uncontrolled Uncontrolled
Rights: Include Include Include Include
Lanes: 1 0 2 0 0 0 2 0 1 0 0 1 0 0 0 0 0 0 0 0

Volume Module:
Base Vol: 14 665 0 0 765 10 27 0 19 0 0 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Critical Gap Module:
Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx xxxxx

Capacity Module:
Conflict Vol: 775 xxxxx xxxxx xxxxx xxxxx 1126 1458 383 xxxxx xxxxx xxxxx

Level Of Service Module:
2Way95thQ: 0.1 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx

Control Del: 9.3 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx

LOS by Move: A * * * * *
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT

Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx

Shrd Condel: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx

Shared LOS: * * * * *
ApproachDel: xxxxxx 20.6 xxxxxx

ApproachLOS: * * * * *

Note: Queue reported is the number of cars per lane.

9-10am Peak Hour - Project 2040 October Peak (Existing Lanes)
Harris Quarry EIR
County of Mendocino

Level Of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)
Intersection #2 US 101/Quarry Access

Average Delay (sec/veh): 1.9 Worst Case Level Of Service: C [23.8]

Street Name: US 101 Quarry Access
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Uncontrolled Uncontrolled Uncontrolled Uncontrolled
Rights: Include Include Include Include
Lanes: 0 1 1 0 0 0 0 1 1 0 0 0 1 0 0 0 0 0 0 0

Volume Module:
Base Vol: 56 635 0 0 740 44 44 0 56 0 0 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Critical Gap Module:
Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx xxxxx

Capacity Module:
Conflict Vol: 784 xxxxx xxxxx xxxxx xxxxx 1192 1509 392 xxxxx xxxxx xxxxx

Level Of Service Module:
2Way95thQ: 0.2 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx

Control Del: 9.6 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx

LOS by Move: A * * * * *
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT

Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx

Shrd Condel: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx

Shared LOS: * * * * *
ApproachDel: xxxxxx 23.8 xxxxxx

ApproachLOS: * * * * *

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
 2000 HCM Unsignalized Method (Base Volume Alternative)

 Intersection #1 US 101/Black Bart

 Average Delay (sec/veh): 1.5 Worst Case Level Of Service: F [50.6]

Street Name: US 101 Black Bart

Approach:	North Bound			South Bound			East Bound			West Bound			
	L	T	R	L	T	R	L	T	R	L	T	R	
Control:	Uncontrolled			Uncontrolled			Uncontrolled			Uncontrolled			
Rights:	Uncontrolled			Uncontrolled			Uncontrolled			Uncontrolled			
Lanes:	1	0	2	0	0	0	2	0	1	0	0	0	0

Volume Module:

Base Vol:	22	1002	0	983	38	43	0	14	0	14	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	22	1002	0	983	38	43	0	14	0	14	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	22	1002	0	983	38	43	0	14	0	14	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	22	1002	0	983	38	43	0	14	0	14	0	0	0

Critical Gap Module:

Critical Gap: 4.1 xxxx xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxx xxxxx
 FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxx xxxxx

Capacity Module:

Conflict Vol: 1021 xxxxx xxxxx xxxxx xxxxx xxxxx 1528 2029 492 xxxxx xxxxx xxxxx
 Potent Cap.: 688 xxxxx xxxxx xxxxx xxxxx xxxxx 110 58 528 xxxxx xxxxx xxxxx
 Move Cap.: 688 xxxxx xxxxx xxxxx xxxxx xxxxx 107 56 528 xxxxx xxxxx xxxxx
 Volume/Cap: 0.03 xxxxx xxxxx xxxxx xxxxx xxxxx 0.40 0.00 0.03 xxxxx xxxxx xxxxx

Level Of Service Module:

2Way95thQ: 0.1 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
 Control Del: 10.4 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
 LOS by Move: B * * * * *
 Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
 Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 134 xxxxx xxxxx xxxxx xxxxx
 SharedQueue: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 1-9 xxxxx xxxxx xxxxx xxxxx
 Shrd Condel: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 50.6 xxxxx xxxxx xxxxx xxxxx
 Shared LOS: * * * * * F * * * * *
 ApproachDel: xxxxxx 50.6 xxxxxx
 ApproachLOS: * * * * *

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
 2000 HCM Unsignalized Method (Base Volume Alternative)

 Intersection #2 US 101/Quarry Access

 Average Delay (sec/veh): 2.6 Worst Case Level Of Service: E [49.1]

Street Name: US 101 Quarry Access

Approach:	North Bound			South Bound			East Bound			West Bound			
	L	T	R	L	T	R	L	T	R	L	T	R	
Control:	Uncontrolled			Uncontrolled			Uncontrolled			Uncontrolled			
Rights:	Uncontrolled			Uncontrolled			Uncontrolled			Uncontrolled			
Lanes:	0	1	1	0	0	0	1	0	0	0	1	0	0

Volume Module:

Base Vol:	56	980	0	953	44	44	0	56	0	56	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	56	980	0	953	44	44	0	56	0	56	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	56	980	0	953	44	44	0	56	0	56	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	56	980	0	953	44	44	0	56	0	56	0	0	0

Critical Gap Module:

Critical Gap: 4.1 xxxx xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxx xxxxx
 FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxx xxxxx

Capacity Module:

Conflict Vol: 997 xxxxx xxxxx xxxxx xxxxx xxxxx 1577 2067 499 xxxxx xxxxx xxxxx
 Potent Cap.: 702 xxxxx xxxxx xxxxx xxxxx xxxxx 102 55 523 xxxxx xxxxx xxxxx
 Move Cap.: 702 xxxxx xxxxx xxxxx xxxxx xxxxx 96 50 523 xxxxx xxxxx xxxxx
 Volume/Cap: 0.08 xxxxx xxxxx xxxxx xxxxx xxxxx 0.46 0.00 0.11 xxxxx xxxxx xxxxx

Level Of Service Module:

2Way95thQ: 0.3 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
 Control Del: 10.6 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
 LOS by Move: B * * * * *
 Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
 Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 177 xxxxx xxxxx xxxxx xxxxx
 SharedQueue: 0.3 xxxxx xxxxx xxxxx xxxxx xxxxx 3.0 xxxxx xxxxx xxxxx xxxxx
 Shrd Condel: 10.6 xxxxx xxxxx xxxxx xxxxx xxxxx 49.1 xxxxx xxxxx xxxxx xxxxx
 Shared LOS: B * * * * * E * * * * *
 ApproachDel: xxxxxx 49.1 xxxxxx
 ApproachLOS: * * * * *

Note: Queue reported is the number of cars per lane.

Level of Service Computation Report

Level of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 US 101/Quarry Access

Intersection #1 US 101/Black Bart

Average Delay (sec/veh): 1.6 Worst Case Level of Service: E [43.5]

Average Delay (sec/veh): 1.7 Worst Case Level of Service: F [63.4]

Street Name: US 101

Street Name: Black Bart

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Uncontrolled Uncontrolled
Rights: Include Include Include Include
Lanes: 0 1 1 0 0 0 0 1 1 0 0 0 1 1 0 0 0 0 0 0

Control: Uncontrolled Uncontrolled Uncontrolled Uncontrolled
Rights: Include Include Include Include
Lanes: 1 0 2 0 0 0 0 2 0 1 0 0 1 1 0 0 0 0 0 0

Volume Module:

Volume Module:

Base Vol: 44 1159 0 0 972 32 32 0 44 0 0 0 0

Base Vol: 27 1164 0 0 990 38 43 0 14 0 0 0 0

Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Initial Bse: 44 1159 0 0 972 32 32 0 44 0 0 0 0

Initial Bse: 27 1164 0 0 990 38 43 0 14 0 0 0 0

User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

PHF Volume: 44 1159 0 0 972 32 32 0 44 0 0 0 0

PHF Volume: 27 1164 0 0 990 38 43 0 14 0 0 0 0

Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0 0

Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0 0

FinalVolume: 44 1159 0 0 972 32 32 0 44 0 0 0 0

FinalVolume: 27 1164 0 0 990 38 43 0 14 0 0 0 0

Critical Gap Module:

Critical Gap Module:

Critical Gap: 4.1 xxxxxx xxxxxx xxxxxx xxxxxx 6.8 6.5 6.9 xxxxxx xxxxxx

Critical Gap: 4.1 xxxxxx xxxxxx xxxxxx xxxxxx 6.8 6.5 6.9 xxxxxx xxxxxx

FollowUpTim: 2.2 xxxxxx xxxxxx xxxxxx xxxxxx 3.5 4.0 3.3 xxxxxx xxxxxx

FollowUpTim: 2.2 xxxxxx xxxxxx xxxxxx xxxxxx 3.5 4.0 3.3 xxxxxx xxxxxx

Capacity Module:

Capacity Module:

Conflict Vol: 1004 xxxxxx xxxxxx xxxxxx 1656 2235 502 xxxxxx xxxxxx

Conflict Vol: 1028 xxxxxx xxxxxx xxxxxx 1626 2208 495 xxxxxx xxxxxx

Potent Cap: 698 xxxxxx xxxxxx xxxxxx 91 43 520 xxxxxx xxxxxx

Potent Cap: 683 xxxxxx xxxxxx xxxxxx 95 45 525 xxxxxx xxxxxx

Move Cap: 698 xxxxxx xxxxxx xxxxxx 86 40 520 xxxxxx xxxxxx

Move Cap: 683 xxxxxx xxxxxx xxxxxx 92 43 525 xxxxxx xxxxxx

Volume/Cap: 0.06 xxxxxx xxxxxx xxxxxx 0.37 0.00 0.08 xxxxxx xxxxxx

Volume/Cap: 0.04 xxxxxx xxxxxx xxxxxx 0.47 0.00 0.03 xxxxxx xxxxxx

Level of Service Module:

Level of Service Module:

2Way95thQ: 0.2 xxxxxx xxxxxx xxxxxx xxxxxx xxxxxx xxxxxx xxxxxx

2Way95thQ: 0.1 xxxxxx xxxxxx xxxxxx xxxxxx xxxxxx xxxxxx xxxxxx

Control Del: 10.5 xxxxxx xxxxxx xxxxxx xxxxxx xxxxxx xxxxxx xxxxxx

Control Del: 10.5 xxxxxx xxxxxx xxxxxx xxxxxx xxxxxx xxxxxx xxxxxx

LOS by Move: B *

LOS by Move: B *

Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT

Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT

Shared Cap: xxxxxx xxxxxx xxxxxx xxxxxx xxxxxx 167 xxxxxx xxxxxx

Shared Cap: xxxxxx xxxxxx xxxxxx xxxxxx xxxxxx 115 xxxxxx xxxxxx

SharedQueue: 0.2 xxxxxx xxxxxx xxxxxx xxxxxx 2.1 xxxxxx xxxxxx

SharedQueue: xxxxxx xxxxxx xxxxxx xxxxxx xxxxxx 2.2 xxxxxx xxxxxx

Shrd Condel: 10.5 xxxxxx xxxxxx xxxxxx xxxxxx 43.5 xxxxxx xxxxxx

Shrd Condel: xxxxxx xxxxxx xxxxxx xxxxxx xxxxxx 63.4 xxxxxx xxxxxx

Shared LOS: B *

Shared LOS: *

ApproachDel: xxxxxx 43.5

ApproachDel: xxxxxx 63.4

ApproachLOS: * * * * *

ApproachLOS: * * * * *

Note: Queue reported is the number of cars per lane.

Note: Queue reported is the number of cars per lane.

4:45-5:45pm Peak Hour - Project 2040 October Peak (Existing Lanes)

Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart
Average Delay (sec/veh): 0.7 Worst Case Level of Service: D [29.6]

Table with columns: Street Name, Approach, Movement, Control, Rights, Lanes, Volume Module, Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Volume/Cap, Level of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap, Shared Queue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

4:45-5:45pm Peak Hour - Project 2040 October Peak (Existing Lanes)

Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 US 101/Quarry Access
Average Delay (sec/veh): 0.2 Worst Case Level of Service: D [28.2]

Table with columns: Street Name, Approach, Movement, Control, Rights, Lanes, Volume Module, Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Volume/Cap, Level of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap, Shared Queue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

9-10am Peak Hour - Project 2010 July Peak (Mitigated Lanes)
Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart
Average Delay (sec/veh): 0.4 Worst Case Level of Service: B [13.3]

Table with columns: Street Name, Approach, Movement, Control, Rights, Lanes, Volume Module, Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap, Level of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Queue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

9-10am Peak Hour - Project 2010 July Peak (Mitigated Lanes)
Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 US 101/Quarry Access
Average Delay (sec/veh): 1.1 Worst Case Level of Service: B [13.6]

Table with columns: Street Name, Approach, Movement, Control, Rights, Lanes, Volume Module, Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap, Level of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Queue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

1:15-2:15pm Peak Hour - Project 2010 July Peak (Mitigated Lanes)
Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart

Average Delay (sec/veh): 0.3 Worst Case Level of Service: B [14.6]

Street Name: US 101 Black Bart
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Uncontrolled Uncontrolled
Rights: Include Include Include Include
Lanes: 1 0 2 0 0 0 2 0 1 0 0 1 0 0 0 0 0 0 0

Volume Module:
Base Vol: 18 979 0 0 822 23 12 0 18 0 0 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Initial Bse: 18 979 0 0 822 23 12 0 18 0 0 0
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

PHF Volume: 18 979 0 0 822 23 12 0 18 0 0 0
Reduce Vol: 0 0 0 0 0 0 0 0 0 0 0 0
FinalVolume: 18 979 0 0 822 23 12 0 18 0 0 0

Critical Gap Module:
Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx xxxxx
FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx xxxxx

Capacity Module:
Conflict Vol: 845 xxxxx xxxxx xxxxx xxxxx 1348 1837 411 xxxxx xxxxx xxxxx
Potent Cap.: 800 xxxxx xxxxx xxxxx xxxxx 145 77 596 xxxxx xxxxx xxxxx

Level of Service Module:
2Way95thQ: 0.1 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Control Del: 9.6 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx

LOS by Move: A * * * * *
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT
Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx 405 xxxxx xxxxx xxxxx xxxxx

1:15-2:15pm Peak Hour - Project 2010 July Peak (Mitigated Lanes)
Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 US 101/Quarry Access

Average Delay (sec/veh): 0.7 Worst Case Level of Service: C [16.4]

Street Name: US 101 Quarry Access
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Uncontrolled Uncontrolled
Rights: Include Include Include Include
Lanes: 1 0 2 0 0 0 0 1 1 0 0 0 1 0 0 0 0 0 0

Volume Module:
Base Vol: 32 969 0 0 812 28 28 0 32 0 0 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Initial Bse: 32 969 0 0 812 28 28 0 32 0 0 0
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

PHF Volume: 32 969 0 0 812 28 28 0 32 0 0 0
Reduce Vol: 0 0 0 0 0 0 0 0 0 0 0 0
FinalVolume: 32 969 0 0 812 28 28 0 32 0 0 0

Critical Gap Module:
Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx xxxxx
FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx xxxxx

Capacity Module:
Conflict Vol: 840 xxxxx xxxxx xxxxx xxxxx 1375 1859 420 xxxxx xxxxx xxxxx
Potent Cap.: 804 xxxxx xxxxx xxxxx xxxxx 139 74 588 xxxxx xxxxx xxxxx

Level of Service Module:
2Way95thQ: 0.1 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Control Del: 9.7 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx

LOS by Move: A * * * * *
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT
Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx 376 xxxxx xxxxx xxxxx xxxxx

1:15-2:15pm Peak Hour - Project 2010 July Peak (Mitigated Lanes)
Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart

Average Delay (sec/veh): 0.3 Worst Case Level of Service: B [14.6]

Street Name: US 101 Black Bart
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Uncontrolled Uncontrolled
Rights: Include Include Include Include
Lanes: 1 0 2 0 0 0 2 0 1 0 0 1 0 0 0 0 0 0 0

Volume Module:
Base Vol: 18 979 0 0 822 23 12 0 18 0 0 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Initial Bse: 18 979 0 0 822 23 12 0 18 0 0 0
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

PHF Volume: 18 979 0 0 822 23 12 0 18 0 0 0
Reduce Vol: 0 0 0 0 0 0 0 0 0 0 0 0
FinalVolume: 18 979 0 0 822 23 12 0 18 0 0 0

Critical Gap Module:
Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx xxxxx
FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx xxxxx

Capacity Module:
Conflict Vol: 845 xxxxx xxxxx xxxxx xxxxx 1348 1837 411 xxxxx xxxxx xxxxx
Potent Cap.: 800 xxxxx xxxxx xxxxx xxxxx 145 77 596 xxxxx xxxxx xxxxx

Level of Service Module:
2Way95thQ: 0.1 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Control Del: 9.6 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx

LOS by Move: A * * * * *
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT
Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx 405 xxxxx xxxxx xxxxx xxxxx

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart
Average Delay (sec/veh): 0.3 Worst Case Level of Service: B [14.3]

Street Name: US 101 Black Bart

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Uncontrolled	Uncontrolled	Stop Sign	Stop Sign
Rights:	Include	Include	Include	Include
Lanes:	1 0 2 0 0	0 0 2 0 1	0 0 1 0 0	0 0 0 0 0

Volume Module:

Base Vol:	17 1023	0 800	20 11	0 18	0 0
Growth Adj:	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
Initial Bse:	17 1023	0 800	20 11	0 18	0 0
User Adj:	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
PHF Adj:	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
PHF Volume:	17 1023	0 800	20 11	0 18	0 0
Reduct Vol:	0 0	0 0	0 0	0 0	0 0
FinalVolume:	17 1023	0 800	20 11	0 18	0 0

Critical Gap Module:

Critical Gap:	4.1 xxxxx	xxxxx	xxxxx	xxxxx	6.8 6.5	6.9 xxxxx	xxxxx	xxxxx
FollowUpTim:	2.2 xxxxx	xxxxx	xxxxx	xxxxx	3.5 4.0	3.3 xxxxx	xxxxx	xxxxx

Capacity Module:

Conflict Vol:	820 xxxxx	xxxxx	xxxxx	xxxxx	1346 1857	400 xxxxx	xxxxx	xxxxx
Potent Cap.:	818 xxxxx	xxxxx	xxxxx	xxxxx	145 74	605 xxxxx	xxxxx	xxxxx
Move Cap.:	818 xxxxx	xxxxx	xxxxx	xxxxx	143 73	605 xxxxx	xxxxx	xxxxx
Total Cap.:	xxxxx	xxxxx	xxxxx	xxxxx	275 188	xxxxx	185 183	xxxxx
Volume/Cap.:	0.02 xxxxx	xxxxx	xxxxx	xxxxx	0.04 0.00	0.03 xxxxx	xxxxx	xxxxx

Level of Service Module:

2Way95thQ:	0.1 xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx
Control Del:	9.5 xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx
LOS by Move:	A * * *	* * *	* * *	* * *	* * *	* * *	* * *	* * *
Movement:	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT
Shared Cap.:	xxxxx	xxxxx	xxxxx	xxxxx	416	xxxxx	xxxxx	xxxxx
SharedQueue:	xxxxx	xxxxx	xxxxx	xxxxx	0.2	xxxxx	xxxxx	xxxxx
Shrd Condel:	xxxxx	xxxxx	xxxxx	xxxxx	14.3	xxxxx	xxxxx	xxxxx
Shared LOS:	* * *	* * *	* * *	* * *	B	* * *	* * *	* * *
ApproachDel:	xxxxxx	xxxxxx	xxxxxx	xxxxxx	14.3	xxxxxx	xxxxxx	xxxxxx
ApproachLOS:	*	*	*	*	B	*	*	*

Note: Queue reported is the number of cars per lane.

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 US 101/Quarry Access
Average Delay (sec/veh): 0.1 Worst Case Level of Service: B [14.8]

Street Name: US 101 Quarry Access

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Uncontrolled	Uncontrolled	Stop Sign	Stop Sign
Rights:	Include	Include	Include	Include
Lanes:	1 0 2 0 0	0 0 1 1 0	0 0 1 1 0	0 0 0 0 0

Volume Module:

Base Vol:	0 1034	0 818	0 6	0 6	0 0
Growth Adj:	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
Initial Bse:	0 1034	0 818	0 6	0 6	0 0
User Adj:	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
PHF Adj:	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
PHF Volume:	0 1034	0 818	0 6	0 6	0 0
Reduct Vol:	0 0	0 0	0 0	0 0	0 0
FinalVolume:	0 1034	0 818	0 6	0 6	0 0

Critical Gap Module:

Critical Gap:	xxxxx	xxxxx	xxxxx	xxxxx	6.8 6.5	6.9 xxxxx	xxxxx	xxxxx
FollowUpTim:	xxxxx	xxxxx	xxxxx	xxxxx	3.5 4.0	3.3 xxxxx	xxxxx	xxxxx

Capacity Module:

Conflict Vol:	xxxxx	xxxxx	xxxxx	xxxxx	1335 1852	409 xxxxx	xxxxx	xxxxx
Potent Cap.:	xxxxx	xxxxx	xxxxx	xxxxx	148 75	597 xxxxx	xxxxx	xxxxx
Move Cap.:	xxxxx	xxxxx	xxxxx	xxxxx	148 75	597 xxxxx	xxxxx	xxxxx
Total Cap.:	xxxxx	xxxxx	xxxxx	xxxxx	278 192	xxxxx	195 192	xxxxx
Volume/Cap.:	xxxxx	xxxxx	xxxxx	xxxxx	0.02 0.00	0.01 xxxxx	xxxxx	xxxxx

Level of Service Module:

2Way95thQ:	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx
Control Del:	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx	xxxxx
LOS by Move:	* * *	* * *	* * *	* * *	* * *	* * *	* * *	* * *
Movement:	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT
Shared Cap.:	xxxxx	xxxxx	xxxxx	xxxxx	380	xxxxx	xxxxx	xxxxx
SharedQueue:	xxxxx	xxxxx	xxxxx	xxxxx	0.1	xxxxx	xxxxx	xxxxx
Shrd Condel:	xxxxx	xxxxx	xxxxx	xxxxx	14.8	xxxxx	xxxxx	xxxxx
Shared LOS:	* * *	* * *	* * *	* * *	B	* * *	* * *	* * *
ApproachDel:	xxxxxx	xxxxxx	xxxxxx	xxxxxx	14.8	xxxxxx	xxxxxx	xxxxxx
ApproachLOS:	*	*	*	*	B	*	*	*

Note: Queue reported is the number of cars per lane.

11-12pm Peak Hour - Project 2010 October Peak (Mitigated Lanes)

Harris Quarry EIR
County of Mendocino

Level Of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart

Average Delay (sec/veh): 0.5 Worst Case Level Of Service: B [14.7]

Street Name: US 101

Approach: North Bound South Bound East Bound West Bound

Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Uncontrolled Uncontrolled

Rights: Include Include Include Include

Lanes: 1 0 2 0 0 0 2 0 1 0 0 1 0 0 0 0 0 0 0

Volume Module:

Table with 17 columns: Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume. Values range from 0 to 647.

Critical Gap Module:

Table with 4 columns: Critical Gap, FollowUpTim, Capacity Module. Values include 4.1, 2.2, 6.8, 6.5, 6.9, 3.5, 4.0, 3.3.

Capacity Module:

Table with 4 columns: Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap. Values include 671, 929, 929, 363, 0.08, 1334, 324, 678, 678, 306, 270.

Level Of Service Module:

Table with 4 columns: 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap, SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS. Values include 0.0, 8.9, A, LT-LTR-RT, 409, 0.3, 14.7, B, 14.7.

Note: Queue reported is the number of cars per lane.

11-12pm Peak Hour - Project 2010 October Peak (Mitigated Lanes)

Harris Quarry EIR
County of Mendocino

Level Of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 US 101/Quarry Access

Average Delay (sec/veh): 1.4 Worst Case Level Of Service: B [14.7]

Street Name: US 101

Approach: North Bound South Bound East Bound West Bound

Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Uncontrolled Uncontrolled

Rights: Include Include Include Include

Lanes: 1 0 2 0 0 0 0 1 1 0 0 0 1 1 0 0 0 0 0 0

Volume Module:

Table with 17 columns: Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume. Values range from 0 to 629.

Critical Gap Module:

Table with 4 columns: Critical Gap, FollowUpTim, Capacity Module. Values include 4.1, 2.2, 6.8, 6.5, 6.9, 3.5, 4.0, 3.3.

Capacity Module:

Table with 4 columns: Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap. Values include 656, 941, 941, 342, 0.13, 1375, 328, 674, 674, 268, 246.

Level Of Service Module:

Table with 4 columns: 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap, SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS. Values include 0.2, 9.1, A, LT-LTR-RT, 473, 0.8, 14.7, B, 14.7.

Note: Queue reported is the number of cars per lane.

4:45-5:45pm Peak Hour - Project 2010 October Peak (Mitigated Lanes)

Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart
Average Delay (sec/veh): 0.4 Worst Case Level of Service: B [12.5]

Table with columns: Street Name, Approach, Movement, Control, Rights, Lanes, Volume Module, Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap, Level of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

4:45-5:45pm Peak Hour - Project 2010 October Peak (Mitigated Lanes)

Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 US 101/Quarry Access
Average Delay (sec/veh): 0.1 Worst Case Level of Service: B [12.8]

Table with columns: Street Name, Approach, Movement, Control, Rights, Lanes, Volume Module, Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap, Level of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

9-10am Peak Hour - Project 2014 w/ Willits Bypass July Peak (Mitigated Lanes)
Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart

Average Delay (sec/veh): 0.4 Worst Case Level of Service: B [14.0]

Street Name: US 101 Black Bart
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
Rights: Include Include Include Include
Lanes: 1 0 2 0 0 0 2 0 1 0 0 1 1 0 0 0 0 0 0 0

Volume Module:
Base Vol: 9 610 0 0 699 7 19 0 13 0 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 9 610 0 0 699 7 19 0 13 0 0
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 9 610 0 0 699 7 19 0 13 0 0
Reduct Vol: 0 0 0 0 0 0 0 0 0 0
FinalVolume: 9 610 0 0 699 7 19 0 13 0 0

Critical Gap Module:
Critical Gap: 4.1 xxxxxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxxx xxxxx xxxxxx
FollowUpTim: 2.2 xxxxx xxxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxxx xxxxx xxxxxx

Capacity Module:
Conflict Vol: 706 xxxxx xxxxxx xxxxx xxxxx 1022 1327 350 xxxxx xxxxx xxxxxx
Potent Cap.: 902 xxxxx xxxxxx xxxxx xxxxx 235 157 652 xxxxx xxxxx xxxxxx
Move Cap.: 902 xxxxx xxxxxx xxxxx xxxxx 234 155 652 xxxxx xxxxx xxxxxx
Total Cap: xxxxx xxxxxx xxxxx xxxxx xxxxx 353 280 xxxxxx 323 276 xxxxxx
Volume/Cap: 0.01 xxxxx xxxxx xxxxx xxxxx 0.05 0.00 0.02 xxxxx xxxxx xxxxx
Level of Service Module:
2Way95thQ: 0.0 xxxxx xxxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxxx
Control Del: 9.0 xxxxx xxxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxxx
LOS by Move: A * * * * *
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
Shared Cap.: xxxxx xxxxxx xxxxx xxxxx xxxxx 434 xxxxxx xxxxx xxxxx xxxxxx
SharedQueue: xxxxx xxxxx xxxxx xxxxx xxxxx 0.2 xxxxxx xxxxx xxxxx xxxxxx
Shrd ConDel: xxxxx xxxxx xxxxx xxxxx xxxxx 14.0 xxxxxx xxxxx xxxxx xxxxxx
Shared LOS: * * * * *
ApproachDel: xxxxxx xxxxxx 14.0 xxxxxx
ApproachLOS: * * * * *

Note: Queue reported is the number of cars per lane.

9-10am Peak Hour - Project 2014 w/ Willits Bypass July Peak (Mitigated Lanes)
Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 US 101/Quarry Access

Average Delay (sec/veh): 1.0 Worst Case Level of Service: C [15.4]

Street Name: US 101 Quarry Access
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
Rights: Include Include Include Include
Lanes: 1 0 2 0 0 0 0 1 1 0 0 0 1 1 0 0 0 0 0 0

Volume Module:
Base Vol: 28 571 0 0 664 48 48 0 28 0 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 28 571 0 0 664 48 48 0 28 0 0
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 28 571 0 0 664 48 48 0 28 0 0
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0
FinalVolume: 28 571 0 0 664 48 48 0 28 0 0

Critical Gap Module:
Critical Gap: 4.1 xxxxx xxxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxxx xxxxx xxxxxx
FollowUpTim: 2.2 xxxxx xxxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxxx xxxxx xxxxxx

Capacity Module:
Conflict Vol: 712 xxxxx xxxxxx xxxxx xxxxx 1030 1315 356 xxxxx xxxxx xxxxxx
Potent Cap.: 897 xxxxx xxxxxx xxxxx xxxxx 233 159 646 xxxxx xxxxx xxxxxx
Move Cap.: 897 xxxxx xxxxxx xxxxx xxxxx 227 154 646 xxxxx xxxxx xxxxxx
Total Cap: xxxxx xxxxxx xxxxx xxxxx xxxxx 349 279 xxxxxx 316 267 xxxxxx
Volume/Cap: 0.03 xxxxx xxxxx xxxxx xxxxx 0.14 0.00 0.04 xxxxx xxxxx xxxxx
Level of Service Module:
2Way95thQ: 0.1 xxxxx xxxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxxx
Control Del: 9.1 xxxxx xxxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxxx
LOS by Move: A * * * * *
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
Shared Cap.: xxxxx xxxxxx xxxxx xxxxx xxxxx 421 xxxxxx xxxxx xxxxx xxxxxx
SharedQueue: xxxxx xxxxx xxxxx xxxxx xxxxx 0.7 xxxxxx xxxxx xxxxx xxxxxx
Shrd ConDel: xxxxx xxxxx xxxxx xxxxx xxxxx 15.4 xxxxxx xxxxx xxxxx xxxxxx
Shared LOS: * * * * *
ApproachDel: xxxxxx xxxxxx 15.4 xxxxxx
ApproachLOS: * * * * *

Note: Queue reported is the number of cars per lane.

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart
Average Delay (sec/veh): 0.5 Worst Case Level of Service: C [18.9]
Street Name: US 101
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
Rights: Include Include Include Include
Lanes: 1 0 2 0 0 0 2 0 1 0 0 1 1 0 0 0 0 0 0 0
Volume Module:
Base Vol: 15 914 0 0 895 26 30 0 0 9 0 0 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 15 914 0 0 895 26 30 0 0 9 0 0
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 15 914 0 0 895 26 30 0 0 9 0 0
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
FinalVolume: 15 914 0 0 895 26 30 0 0 9 0 0
Critical Gap Module:
Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxx xxxxx
FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxx xxxxx
Capacity Module:
Conflict Vol: 921 xxxxx xxxxx xxxxx xxxxx 1382 1839 448 xxxxx xxxxx xxxxx
Potent Cap.: 750 xxxxx xxxxx xxxxx xxxxx 137 76 564 xxxxx xxxxx xxxxx
Move Cap.: 750 xxxxx xxxxx xxxxx xxxxx 135 75 564 xxxxx xxxxx xxxxx
Total Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx 261 193 xxxxx 208 187 xxxxx
Volume/Cap: 0.02 xxxxx xxxxx xxxxx xxxxx 0.11 0.00 0.02 xxxxx xxxxx xxxxx

Level of Service Module:
2Way95thQ: 0.1 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Control Del: 9.9 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
LOS by Move: A *
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
SharedCap.: xxxxx xxxxx xxxxx xxxxx xxxxx 298 xxxxx xxxxx xxxxx xxxxx
ShareQueue: xxxxx xxxxx xxxxx xxxxx xxxxx 0.4 xxxxx xxxxx xxxxx xxxxx
Shrd Condel: xxxxx xxxxx xxxxx xxxxx xxxxx 18.9 xxxxx xxxxx xxxxx xxxxx
Shared LOS: * * * * * * * * * * * C *
ApproachDel: xxxxxx * xxxxxx 18.9 xxxxxx *
ApproachLOS: *

Note: Queue reported is the number of cars per lane.

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 US 101/Quarry Access
Average Delay (sec/veh): 0.9 Worst Case Level of Service: C [19.4]
Street Name: US 101
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
Rights: Include Include Include Include
Lanes: 1 0 2 0 0 0 0 1 1 0 0 0 1 1 0 0 0 0 0 0 0
Volume Module:
Base Vol: 28 881 0 0 856 48 48 0 0 28 0 0 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 28 881 0 0 856 48 48 0 0 28 0 0
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 28 881 0 0 856 48 48 0 0 28 0 0
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
FinalVolume: 28 881 0 0 856 48 48 0 0 28 0 0
Critical Gap Module:
Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxx xxxxx
FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxx xxxxx
Capacity Module:
Conflict Vol: 904 xxxxx xxxxx xxxxx xxxxx 1377 1817 452 xxxxx xxxxx xxxxx
Potent Cap.: 761 xxxxx xxxxx xxxxx xxxxx 139 79 560 xxxxx xxxxx xxxxx
Move Cap.: 761 xxxxx xxxxx xxxxx xxxxx 135 76 560 xxxxx xxxxx xxxxx
Total Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx 262 194 xxxxx 206 185 xxxxx
Volume/Cap: 0.04 xxxxx xxxxx xxxxx xxxxx 0.18 0.00 0.05 xxxxx xxxxx xxxxx

Level of Service Module:
2Way95thQ: 0.1 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Control Del: 9.9 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
LOS by Move: A *
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
SharedCap.: xxxxx xxxxx xxxxx xxxxx xxxxx 326 xxxxx xxxxx xxxxx xxxxx
ShareQueue: xxxxx xxxxx xxxxx xxxxx xxxxx 0.9 xxxxx xxxxx xxxxx xxxxx
Shrd Condel: xxxxx xxxxx xxxxx xxxxx xxxxx 19.4 xxxxx xxxxx xxxxx xxxxx
Shared LOS: * * * * * * * * * * * C *
ApproachDel: xxxxxx * xxxxxx 19.4 xxxxxx *
ApproachLOS: *

Note: Queue reported is the number of cars per lane.

9-10am Peak Hour - Project 2014 w/ Willlits Bypass October Peak (Mitigated Lanes Harris Quarry EIR County of Mendocino

Level of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) Intersection #1 US 101/Black Bart

Average Delay (sec/veh): 0.4 Worst Case Level of Service: B [12.6] Street Name: US 101 Black Bart Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R L - T - R Control: Uncontrolled Uncontrolled Stop Sign Stop Sign Rights: Include Include Include Include Lanes: 1 0 2 0 0 0 2 0 1 0 0 1 0 0 0 0 0 0 0

Volume Module: Base Vol: 9 493 0 0 561 7 19 0 13 0 0 0 0 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Initial Bse: 9 493 0 0 561 7 19 0 13 0 0 0 0 User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 PHF Volume: 9 493 0 0 561 7 19 0 13 0 0 0 0 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0 FinalVolume: 9 493 0 0 561 7 19 0 13 0 0 0 0

Critical Gap Module: Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx xxxxx FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx xxxxx Capacity Module: Conflict Vol: 568 xxxxx xxxxx xxxxx xxxxx 826 1072 281 xxxxx xxxxx xxxxx Potent Cap.: 1014 xxxxx xxxxx xxxxx xxxxx 315 222 723 xxxxx xxxxx xxxxx Move Cap.: 1014 xxxxx xxxxx xxxxx xxxxx 312 220 723 xxxxx xxxxx xxxxx Total Cap: xxxxx xxxxx xxxxx xxxxx xxxxx 423 340 xxxxx 390 336 xxxxx Volume/Cap: 0.01 xxxxx xxxxx xxxxx xxxxx 0.04 0.00 0.02 xxxxx xxxxx xxxxx

Level of Service Module: 2Way95thQ: 0.0 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx Control Del: 8.6 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx LOS by Move: A * * * * * * * * * * * * * * * * Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx 508 xxxxx xxxxx xxxxx xxxxx SharedQueue: xxxxx xxxxx xxxxx xxxxx xxxxx 0.2 xxxxx xxxxx xxxxx xxxxx Shrd Condel: xxxxx xxxxx xxxxx xxxxx xxxxx 12.6 xxxxx xxxxx xxxxx Shared LOS: * * * * * * * * * * * * * * * * ApproachDel: xxxxxx 12.6 xxxxxx ApproachLOS: * * * * * * * * * * * * * * * * Note: Queue reported is the number of cars per lane.

9-10am Peak Hour - Project 2014 w/ Willlits Bypass October Peak (Mitigated Lanes Harris Quarry EIR County of Mendocino

Level of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) Intersection #2 US 101/Quarry Access

Average Delay (sec/veh): 1.5 Worst Case Level of Service: B [14.2] Street Name: US 101 Quarry Access Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R L - T - R Control: Uncontrolled Uncontrolled Stop Sign Stop Sign Rights: Include Include Include Include Lanes: 1 0 2 0 0 0 0 1 1 0 0 0 1 0 0 0 0 0 0 0

Volume Module: Base Vol: 36 438 0 0 510 64 64 0 36 0 0 0 0 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Initial Bse: 36 438 0 0 510 64 64 0 36 0 0 0 0 User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 PHF Volume: 36 438 0 0 510 64 64 0 36 0 0 0 0 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0 FinalVolume: 36 438 0 0 510 64 64 0 36 0 0 0 0

Critical Gap Module: Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx xxxxx FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx xxxxx Capacity Module: Conflict Vol: 574 xxxxx xxxxx xxxxx xxxxx 833 1052 287 xxxxx xxxxx xxxxx Potent Cap.: 1009 xxxxx xxxxx xxxxx xxxxx 311 228 716 xxxxx xxxxx xxxxx Move Cap.: 1009 xxxxx xxxxx xxxxx xxxxx 303 220 716 xxxxx xxxxx xxxxx Total Cap: xxxxx xxxxx xxxxx xxxxx xxxxx 418 339 xxxxx 380 322 xxxxx Volume/Cap: 0.04 xxxxx xxxxx xxxxx xxxxx 0.15 0.00 0.05 xxxxx xxxxx xxxxx

Level of Service Module: 2Way95thQ: 0.1 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx Control Del: 8.7 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx LOS by Move: A * * * * * * * * * * * * * * * * Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx 492 xxxxx xxxxx xxxxx xxxxx SharedQueue: xxxxx xxxxx xxxxx xxxxx xxxxx 0.8 xxxxx xxxxx xxxxx xxxxx Shrd Condel: xxxxx xxxxx xxxxx xxxxx xxxxx 14.2 xxxxx xxxxx xxxxx Shared LOS: * * * * * * * * * * * * * * * * ApproachDel: xxxxxx 14.2 xxxxxx ApproachLOS: * * * * * * * * * * * * * * * * Note: Queue reported is the number of cars per lane.

11-12pm Peak Hour - Project 2014 w/ Willits Bypass October Peak (Mitigated Lane Harris Quarry EIR County of Mendocino

Level of Service Computation Report
 2000 HCM Unsignalized Method (Base Volume Alternative)
 Intersection #1 US 101/Black Bart

Average Delay (sec/veh): 0.5 Worst Case Level of Service: C[15.7]
 Street Name: US 101 Black Bart
 Approach: North Bound South Bound East Bound West Bound
 Movement: L - T - R L - T - R L - T - R L - T - R
 Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
 Rights: Include Include Include Include
 Lanes: 1 0 2 0 0 0 2 0 1 0 0 1 0 0 0 0 0 0 0 0

Volume Module:
 Base Vol: 15 725 0 0 712 26 30 0 9 0 0 0
 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 Initial Bse: 15 725 0 712 26 30 0 9 0 0
 User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 PHF Volume: 15 725 0 712 26 30 0 9 0 0
 Reduct Vol: 0 0 0 0 0 0 0 0 0 0
 FinalVolume: 15 725 0 0 712 26 30 0 9 0 0

Critical Gap Module:

Critical Gap: 4.1 xxx xxxxxx 6.8 6.5 6.9 xxxxxx
 FollowUpTim: 2.2 xxx xxxxxx 3.5 4.0 3.3 xxxxxx

Capacity Module:

Conflict Vol: 738 xxxxxx 1105 1467 356 xxxxxx
 Potent Cap.: 877 xxxxxx 208 129 646 xxxxxx
 Move Cap.: 877 xxxxxx 206 127 646 xxxxxx
 Total Cap: xxxxxx 332 252 276 244
 Volume/Cap: 0.02 xxxxxx 0.09 0.00 0.01 xxxxxx

Level of Service Module:

2Way95thQ: 0.1 xxxxxx 3.3 3.3 3.3 xxxxxx
 Control Del: 9.2 xxxxxx 15.7 15.7 15.7 xxxxxx
 LOS by Move: A * * * * *
 Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT
 Shared Cap.: xxxxxx 374 xxxxxx 0.3 xxxxxx
 SharedQueue: xxxxxx 0.3 xxxxxx 15.7 xxxxxx
 Shrd ConDel: xxxxxx 15.7 xxxxxx C * * * * *
 Shared LOS: * * * * *
 ApproachDel: xxxxxx 15.7 C * * * * *
 ApproachLOS: * * * * *
 Note: Queue reported is the number of cars per lane.

11-12pm Peak Hour - Project 2014 w/ Willits Bypass October Peak (Mitigated Lane Harris Quarry EIR County of Mendocino

Level of Service Computation Report
 2000 HCM Unsignalized Method (Base Volume Alternative)
 Intersection #2 US 101/Quarry Access

Average Delay (sec/veh): 1.3 Worst Case Level of Service: C[16.9]
 Street Name: US 101 Quarry Access
 Approach: North Bound South Bound East Bound West Bound
 Movement: L - T - R L - T - R L - T - R L - T - R
 Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
 Rights: Include Include Include Include
 Lanes: 1 0 2 0 0 0 0 1 1 0 0 0 1 0 0 0 0 0 0 0

Volume Module:
 Base Vol: 36 676 0 0 657 64 64 0 36 0 0 0
 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 Initial Bse: 36 676 0 657 64 64 0 36 0 0
 User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
 PHF Volume: 36 676 0 657 64 64 0 36 0 0
 Reduct Vol: 0 0 0 0 0 0 0 0 0 0
 FinalVolume: 36 676 0 0 657 64 64 0 36 0 0

Critical Gap Module:

Critical Gap: 4.1 xxx xxxxxx 6.8 6.5 6.9 xxxxxx
 FollowUpTim: 2.2 xxx xxxxxx 3.5 4.0 3.3 xxxxxx

Capacity Module:

Conflict Vol: 721 xxxxxx 1099 1437 361 xxxxxx
 Potent Cap.: 890 xxxxxx 210 135 642 xxxxxx
 Move Cap.: 890 xxxxxx 204 129 642 xxxxxx
 Total Cap: xxxxxx 332 253 271 240
 Volume/Cap: 0.04 xxxxxx 0.19 0.00 0.06 xxxxxx

Level of Service Module:

2Way95thQ: 0.1 xxxxxx 3.3 3.3 3.3 xxxxxx
 Control Del: 9.2 xxxxxx 16.9 16.9 16.9 xxxxxx
 LOS by Move: A * * * * *
 Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT
 Shared Cap.: xxxxxx 402 xxxxxx 1.0 xxxxxx
 SharedQueue: xxxxxx 1.0 xxxxxx 16.9 xxxxxx
 Shrd ConDel: xxxxxx 16.9 xxxxxx C * * * * *
 Shared LOS: * * * * *
 ApproachDel: xxxxxx 16.9 C * * * * *
 ApproachLOS: * * * * *
 Note: Queue reported is the number of cars per lane.

1:15-2:15pm Peak Hour - Project 2014 w/ Willits Bypass October Peak (Mitigated) Harris Quarry EIR County of Mendocino

Level of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) Intersection #1 US 101/Black Bart

Average Delay (sec/veh): 0.5 Worst Case Level of Service: C [16.2] Street Name: US 101 Black Bart Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R L - T - R Control: Uncontrolled Uncontrolled Stop Sign Stop Sign Rights: Include Include Include Include Lanes: 1 0 2 0 0 0 2 0 1 0 0 1 0 0 0 0 0 0 0 0

Volume Module: Base Vol: 19 828 0 0 709 26 30 0 9 0 0 0 0 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Initial Bse: 19 828 0 0 709 26 30 0 9 0 0 0 0 User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 PHF Volume: 19 828 0 0 709 26 30 0 9 0 0 0 0 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0 FinalVolume: 19 828 0 0 709 26 30 0 9 0 0 0 0

Critical Gap Module: Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxx xxxxx FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxx xxxxx Capacity Module: Conflict Vol: 735 xxxxx xxxxx xxxxx xxxxx 1161 1575 355 xxxxx xxxxx xxxxx Potent Cap.: 879 xxxxx xxxxx xxxxx xxxxx 191 111 648 xxxxx xxxxx xxxxx Move Cap.: 879 xxxxx xxxxx xxxxx xxxxx 188 109 648 xxxxx xxxxx xxxxx Total Cap: xxxxx xxxxx xxxxx xxxxx xxxxx 319 231 xxxxx 241 224 xxxxx Volume/Cap: 0.02 xxxxx xxxxx xxxxx xxxxx 0.09 0.00 0.01 xxxxx xxxxx xxxxx

Level of Service Module: 2Way95thQ: 0.1 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx Control Del: 9.2 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx LOS by Move: A * * * * * Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx 362 xxxxx xxxxx xxxxx xxxxx SharedQueue: xxxxx xxxxx xxxxx xxxxx xxxxx 0.4 xxxxx xxxxx xxxxx xxxxx Shrd ConDel: xxxxx xxxxx xxxxx xxxxx xxxxx 16.2 xxxxx xxxxx xxxxx xxxxx Shared LOS: * * * * * C * * * * * ApproachDel: xxxxxx 16.2 xxxxxx ApproachLOS: * * * * * C * * * * * Note: Queue reported is the number of cars per lane.

1:15-2:15pm Peak Hour - Project 2014 w/ Willits Bypass October Peak (Mitigated) Harris Quarry EIR County of Mendocino

Level of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) Intersection #2 US 101/Quarry Access

Average Delay (sec/veh): 0.9 Worst Case Level of Service: C [16.3] Street Name: US 101 Quarry Access Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R L - T - R Control: Uncontrolled Uncontrolled Stop Sign Stop Sign Rights: Include Include Include Include Lanes: 1 0 2 0 0 0 0 1 1 0 0 0 1 0 0 0 0 0 0 0 0

Volume Module: Base Vol: 28 799 0 0 670 48 48 0 28 0 0 0 0 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Initial Bse: 28 799 0 0 670 48 48 0 28 0 0 0 0 User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 PHF Volume: 28 799 0 0 670 48 48 0 28 0 0 0 0 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0 FinalVolume: 28 799 0 0 670 48 48 0 28 0 0 0 0

Critical Gap Module: Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxx xxxxx FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxx xxxxx Capacity Module: Conflict Vol: 718 xxxxx xxxxx xxxxx xxxxx 1150 1549 359 xxxxx xxxxx xxxxx Potent Cap.: 892 xxxxx xxxxx xxxxx xxxxx 195 115 643 xxxxx xxxxx xxxxx Move Cap.: 892 xxxxx xxxxx xxxxx xxxxx 190 111 643 xxxxx xxxxx xxxxx Total Cap: xxxxx xxxxx xxxxx xxxxx xxxxx 322 234 xxxxx 241 225 xxxxx Volume/Cap: 0.03 xxxxx xxxxx xxxxx xxxxx 0.15 0.00 0.04 xxxxx xxxxx xxxxx

Level of Service Module: 2Way95thQ: 0.1 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx Control Del: 9.2 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx LOS by Move: A * * * * * Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx 394 xxxxx xxxxx xxxxx xxxxx SharedQueue: xxxxx xxxxx xxxxx xxxxx xxxxx 0.7 xxxxx xxxxx xxxxx xxxxx Shrd ConDel: xxxxx xxxxx xxxxx xxxxx xxxxx 16.3 xxxxx xxxxx xxxxx xxxxx Shared LOS: * * * * * C * * * * * ApproachDel: xxxxxx 16.3 xxxxxx ApproachLOS: * * * * * C * * * * * Note: Queue reported is the number of cars per lane.

4:45-5:45pm Peak Hour - Project 2014 w/ Willits Bypass October Peak (Mitigated)
Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart
Average Delay (sec/veh): 0.4 Worst Case Level of Service: B [13.0]

Table with columns: Street Name, Approach, Movement, Control, Rights, Lanes, Volume Module, Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap, Level of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

4:45-5:45pm Peak Hour - Project 2014 w/ Willits Bypass October Peak (Mitigated)
Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 US 101/Quarry Access
Average Delay (sec/veh): 0.1 Worst Case Level of Service: B [13.2]

Table with columns: Street Name, Approach, Movement, Control, Rights, Lanes, Volume Module, Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap, Level of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

9-10am Peak Hour - Project 2014 w/o Willits Bypass July Peak (Mitigated Lanes)

Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart
Average Delay (sec/veh): 0.4 Worst Case Level of Service: B [13.9]

Street Name: US 101
Approach: North Bound South Bound East Bound West Bound

Table with columns for L, T, R, L, T, R, L, T, R and rows for Uncontrolled, Include, Stop Sign, and Lanes.

Volume Module: Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume

Critical Gap Module:

Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx xxxxx

Capacity Module:

Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap

Level of Service Module:

2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap, Shared Queue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS

Note: Queue reported is the number of cars per lane.

9-10am Peak Hour - Project 2014 w/o Willits Bypass July Peak (Mitigated Lanes)

Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 US 101/Quarry Access
Average Delay (sec/veh): 1.1 Worst Case Level of Service: B [14.1]

Street Name: US 101
Approach: North Bound South Bound East Bound West Bound

Table with columns for L, T, R, L, T, R, L, T, R and rows for Uncontrolled, Include, Stop Sign, and Lanes.

Volume Module: Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume

Critical Gap Module:

Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx xxxxx

Capacity Module:

Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap

Level of Service Module:

2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap, Shared Queue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS

Note: Queue reported is the number of cars per lane.

11-12pm Peak Hour - Project 2014 w/o Willis Bypass July Peak (Mitigated Lanes)
Harris Quarry EIR
County of Mendocino

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart

Average Delay (sec/veh): 0.5 Worst Case Level Of Service: C[18.6]

Street Name: US 101

Approach: North Bound South Bound East Bound West Bound

Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Uncontrolled Uncontrolled

Rights: Include Include Include Include

Lanes: 1 0 2 0 0 0 2 0 1 0 0 1 1 0 0 0 0 0 0

Volume Module:

Base Vol: 15 898 0 0 879 26 30 0 9 0 0 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 15 898 0 0 879 26 30 0 9 0 0
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 15 898 0 0 879 26 30 0 9 0 0
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0
FinalVolume: 15 898 0 0 879 26 30 0 9 0 0

Critical Gap Module:

Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx xxxxx
FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx xxxxx

Capacity Module:

Conflict Vol: 905 xxxxx xxxxx xxxxx xxxxx 1358 1807 440 xxxxx xxxxx xxxxx
Potent Cap.: 760 xxxxx xxxxx xxxxx xxxxx 143 80 571 xxxxx xxxxx xxxxx
Move Cap.: 760 xxxxx xxxxx xxxxx xxxxx 140 78 571 xxxxx xxxxx xxxxx
Total Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx 267 198 xxxxx 213 191 xxxxx
Volume/Cap: 0.02 xxxxx xxxxx xxxxx xxxxx 0.11 0.00 0.02 xxxxx xxxxx xxxxx

Level Of Service Module:

2Way95thQ: 0.1 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Control Del: 9.8 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
LOS by Move: A *
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
SharedQueue: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Shrd ConDel: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Shared LOS: *
ApproachDel: xxxxxx xxxxxx 18.6 C C
ApproachLOS: *

Note: Queue reported is the number of cars per lane.

11-12pm Peak Hour - Project 2014 w/o Willis Bypass July Peak (Mitigated Lanes)
Harris Quarry EIR
County of Mendocino

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 US 101/Quarry Access

Average Delay (sec/veh): 0.9 Worst Case Level Of Service: C[17.0]

Street Name: US 101

Approach: North Bound South Bound East Bound West Bound

Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Uncontrolled Uncontrolled

Rights: Include Include Include Include

Lanes: 1 0 2 0 0 0 2 0 1 0 0 1 1 0 0 0 0 0 0

Volume Module:

Base Vol: 44 881 0 0 856 32 32 0 44 0 0 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 44 881 0 0 856 32 32 0 44 0 0
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 44 881 0 0 856 32 32 0 44 0 0
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0
FinalVolume: 44 881 0 0 856 32 32 0 44 0 0

Critical Gap Module:

Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx xxxxx
FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx xxxxx

Capacity Module:

Conflict Vol: 888 xxxxx xxxxx xxxxx xxxxx 1401 1841 444 xxxxx xxxxx xxxxx
Potent Cap.: 771 xxxxx xxxxx xxxxx xxxxx 134 76 567 xxxxx xxxxx xxxxx
Move Cap.: 771 xxxxx xxxxx xxxxx xxxxx 128 72 567 xxxxx xxxxx xxxxx
Total Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx 256 188 xxxxx 191 177 xxxxx
Volume/Cap: 0.06 xxxxx xxxxx xxxxx xxxxx 0.12 0.00 0.08 xxxxx xxxxx xxxxx

Level Of Service Module:

2Way95thQ: 0.2 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Control Del: 10.0 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
LOS by Move: A *
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
SharedQueue: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Shrd ConDel: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Shared LOS: *
ApproachDel: xxxxxx xxxxxx 17.0 C C
ApproachLOS: *

Note: Queue reported is the number of cars per lane.

4:45-5:45pm Peak Hour - Project 2014 w/o Willits Bypass July Peak (Mitigated La Harris Quarry EIR County of Mendocino

Level Of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart

Average Delay (sec/veh): 0.3 Worst Case Level Of Service: C[15.1]
Street Name: US 101 Black Bart

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
Rights: Include Include Include Include
Lanes: 1 0 2 0 0 0 2 0 1 0 0 1 0 0 0 0 0 0 0 0

Volume Module:
Base Vol: 20 1097 0 0 860 21 12 0 19 0 0 0 0

Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 20 1097 0 0 860 21 12 0 19 0 0 0 0

User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

PHF Volume: 20 1097 0 0 860 21 12 0 19 0 0 0 0
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0

Final Volume: 20 1097 0 0 860 21 12 0 19 0 0 0 0

Critical Gap Module:
Critical Gp: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx xxxxx xxxxx

FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx xxxxx xxxxx

Capacity Module:
Conflict Vol: 881 xxxxx xxxxx xxxxx xxxxx 1449 1997 430 xxxxx xxxxx xxxxx xxxxx

Potent Cap: 776 xxxxx xxxxx xxxxx xxxxx 124 61 579 xxxxx xxxxx xxxxx xxxxx

Move Cap: 776 xxxxx xxxxx xxxxx xxxxx 122 59 579 xxxxx xxxxx xxxxx xxxxx

Total Cap: xxxxx xxxxx xxxxx xxxxx xxxxx 253 170 xxxxx 165 165 xxxxx xxxxx

Volume/Cap: 0.03 xxxxx xxxxx xxxxx xxxxx 0.05 0.00 0.03 xxxxx xxxxx xxxxx xxxxx

Level Of Service Module:
2Way95thQ: 0.1 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx

Control Del: 9.8 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx

LOS by Move: A *

Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT

Shared Cap: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 386 xxxxx xxxxx xxxxx xxxxx

SharedQueue: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 0.3 xxxxx xxxxx xxxxx xxxxx

Shrd ConDel: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 15.1 xxxxx xxxxx xxxxx xxxxx

Shared LOS: *

4:45-5:45pm Peak Hour - Project 2014 w/o Willits Bypass July Peak (Mitigated La Harris Quarry EIR County of Mendocino

Level Of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 US 101/Quarry Access

Average Delay (sec/veh): 0.1 Worst Case Level Of Service: C[15.5]
Street Name: US 101 Quarry Access

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
Rights: Include Include Include Include
Lanes: 1 0 2 0 0 0 2 0 1 0 0 1 0 0 0 0 0 0 0 0

Volume Module:
Base Vol: 0 1111 0 0 879 0 6 0 0 0 0 0 0 0 0

Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 0 1111 0 0 879 0 6 0 0 0 0 0 0 0 0

User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

PHF Volume: 0 1111 0 0 879 0 6 0 0 0 0 0 0 0 0
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0 0

Final Volume: 0 1111 0 0 879 0 6 0 0 0 0 0 0 0 0

Critical Gap Module:
Critical Gp: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx xxxxx xxxxx

FollowUpTim: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx xxxxx xxxxx

Capacity Module:
Conflict Vol: xxxxx xxxxx xxxxx xxxxx xxxxx 1435 1990 440 xxxxx xxxxx xxxxx xxxxx

Potent Cap: xxxxx xxxxx xxxxx xxxxx xxxxx 127 61 571 xxxxx xxxxx xxxxx xxxxx

Move Cap: xxxxx xxxxx xxxxx xxxxx xxxxx 127 61 571 xxxxx xxxxx xxxxx xxxxx

Total Cap: xxxxx xxxxx xxxxx xxxxx xxxxx 256 175 xxxxx 175 175 xxxxx xxxxx

Volume/Cap: xxxxx xxxxx xxxxx xxxxx xxxxx 0.02 0.00 0.01 xxxxx xxxxx xxxxx xxxxx

Level Of Service Module:
2Way95thQ: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx

Control Del: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx

LOS by Move: *

Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT

Shared Cap: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 354 xxxxx xxxxx xxxxx xxxxx

SharedQueue: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 0.1 xxxxx xxxxx xxxxx xxxxx

Shrd ConDel: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 15.5 xxxxx xxxxx xxxxx xxxxx

Shared LOS: *

Note: Queue reported is the number of cars per lane.

9-10am Peak Hour - Project 2014 w/o Willits Bypass October Peak (Mitigated Lane Harris Quarry EIR County of Mendocino

Level of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) Intersection #1 US 101/Black Bart

Average Delay (sec/veh): 0.4 Worst Case Level of Service: B [12.4]

Table with columns: Street Name, Approach, Movement, Control, Rights, Lanes, Volume Module, Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap, Level of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

9-10am Peak Hour - Project 2014 w/o Willits Bypass October Peak (Mitigated Lane Harris Quarry EIR County of Mendocino

Level of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) Intersection #2 US 101/Quarry Access

Average Delay (sec/veh): 1.6 Worst Case Level of Service: B [13.2]

Table with columns: Street Name, Approach, Movement, Control, Rights, Lanes, Volume Module, Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap, Level of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

4:45-5:45pm Peak Hour - Project 2014 w/o Willis Bypass October Peak (Mitigated) Harris Quarry EIR County of Mendocino

4:45-5:45pm Peak Hour - Project 2014 w/o Willis Bypass October Peak (Mitigated) Harris Quarry EIR County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)
Intersection #2 US 101/Quarry Access
Average Delay (sec/veh): 0.1 Worst Case Level of Service: B [13.2]

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)
Intersection #1 US 101/Black Bart
Average Delay (sec/veh): 0.4 Worst Case Level of Service: B [13.0]

Street Name: US 101 Quarry Access
Approach: North Bound South Bound East Bound West Bound
Movement: L T R L T R L T R L T R L T R
Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
Rights: Include Include Include Include
Lanes: 1 0 2 0 0 0 2 0 1 0 0 1 0 0 0 0 0 0 0

Street Name: US 101 Black Bart
Approach: North Bound South Bound East Bound West Bound
Movement: L T R L T R L T R L T R
Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
Rights: Include Include Include Include
Lanes: 1 0 2 0 0 0 2 0 1 0 0 1 0 0 0 0 0 0 0

Volume Module:
Base Vol: 0 853 0 0 675 0 6 0 6 0 0 0 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 0 853 0 0 675 0 6 0 6 0 0 0 0
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 0 853 0 0 675 0 6 0 6 0 0 0 0
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0
FinalVolume: 0 853 0 0 675 0 6 0 6 0 0 0 0
Critical Gap Module:
Critical Gap: xxxxxx xxxxxx xxxxxx xxxxxx xxxxxx 6.8 6.5 6.9 xxxxxx xxxxxx xxxxxx
FollowUpTim: xxxxxx xxxxxx xxxxxx xxxxxx xxxxxx 3.5 4.0 3.3 xxxxxx xxxxxx xxxxxx

Volume Module:
Base Vol: 20 839 0 0 656 21 12 0 0 19 0 0 0 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 20 839 0 0 656 21 12 0 0 19 0 0 0 0
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 20 839 0 0 656 21 12 0 0 19 0 0 0 0
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0
FinalVolume: 20 839 0 0 656 21 12 0 0 19 0 0 0 0
Critical Gap Module:
Critical Gap: 4.1 xxxx xxxxxx xxxxxx xxxxxx xxxxxx 6.8 6.5 6.9 xxxxxx xxxxxx xxxxxx
FollowUpTim: 2.2 xxxx xxxxxx xxxxxx xxxxxx xxxxxx 3.5 4.0 3.3 xxxxxx xxxxxx xxxxxx

Capacity Module:
Conflict Vol: xxxxx xxxxx xxxxxx xxxxxx xxxxxx 1102 1528 338 xxxxx xxxxx xxxxxx
Potential Cap.: xxxxx xxxxx xxxxxx xxxxxx xxxxxx 209 119 664 xxxxx xxxxx xxxxxx
Move Cap.: xxxxx xxxxx xxxxxx xxxxxx xxxxxx 209 119 664 xxxxx xxxxx xxxxxx
Total Cap.: xxxxx xxxxx xxxxxx xxxxxx xxxxxx 339 242 xxxxxx 253 242 xxxxxx
Volume/Cap.: xxxxx xxxxx xxxxxx xxxxxx xxxxxx 0.02 0.00 0.01 xxxxx xxxxx xxxxxx
Level of Service Module:
2Way95thQ: xxxxx xxxxx xxxxxx xxxxxx xxxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxxx
Control Del: xxxxx xxxxx xxxxxx xxxxxx xxxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxxx
LOS by Move: *
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
Shared Cap.: xxxxx xxxxx xxxxxx xxxxxx xxxxxx 449 xxxxx xxxxx xxxxxx xxxxxx
SharedQueue: xxxxx xxxxx xxxxxx xxxxxx xxxxxx 0.1 xxxxxx xxxxx xxxxx xxxxxx
Shrd Condel: xxxxx xxxxx xxxxxx xxxxxx xxxxxx 13.2 xxxxxx xxxxx xxxxx xxxxxx
Shared LOS: *
ApproachDel: xxxxxx xxxxxx 13.2 xxxxxx
ApproachLOS: *

Capacity Module:
Conflict Vol: 677 xxxxx xxxxxx xxxxxx xxxxxx 1116 1535 328 xxxxx xxxxx xxxxxx
Potential Cap.: 924 xxxxx xxxxxx xxxxxx xxxxxx 205 117 674 xxxxx xxxxx xxxxxx
Move Cap.: 924 xxxxx xxxxxx xxxxxx xxxxxx 202 115 674 xxxxx xxxxx xxxxxx
Total Cap.: xxxxx xxxxx xxxxxx xxxxxx xxxxxx 334 237 xxxxxx 239 230 xxxxxx
Volume/Cap.: 0.02 xxxxx xxxxxx xxxxxx xxxxxx 0.04 0.00 0.03 xxxxx xxxxx xxxxxx
Level of Service Module:
2Way95thQ: 0.1 xxxxx xxxxxx xxxxxx xxxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxxx
Control Del: 9.0 xxxxx xxxxxx xxxxxx xxxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxxx
LOS by Move: A *
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
Shared Cap.: xxxxx xxxxx xxxxxx xxxxxx xxxxxx 483 xxxxx xxxxx xxxxxx xxxxxx
SharedQueue: xxxxx xxxxx xxxxxx xxxxxx xxxxxx 0.2 xxxxxx xxxxx xxxxx xxxxxx
Shrd Condel: xxxxx xxxxx xxxxxx xxxxxx xxxxxx 13.0 xxxxxx xxxxx xxxxx xxxxxx
Shared LOS: *
ApproachDel: xxxxxx xxxxxx 13.0 xxxxxx
ApproachLOS: *

Note: Queue reported is the number of cars per lane.
Traffic 7.9.0415 (c) 2007 Dowling Assoc. Licensed to W-TRANS, Santa Rosa, CA

9-10am Peak Hour - Project 2030 July Peak (Mitigated Lanes)

Harris Quarry EIR
County of Mendocino

Level Of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart

Average Delay (sec/veh): 0.5 Worst Case Level Of Service: C [16.3]

Street Name: US 101

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Uncontrolled Uncontrolled
Rights: Include Include Include Include
Lanes: 1 0 2 0 0 0 2 0 1 0 0 1 0 0 0 0 0 0 0

Volume Module:

Table with 18 columns: Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap, Level Of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Queue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS

Critical Gap Module:

Table with 2 columns: Critical Gap, FollowUpTim

Capacity Module:

Table with 2 columns: Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap

Level Of Service Module:

Table with 2 columns: 2Way95thQ, Control Del, LOS by Move, Movement, Shared Queue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS

Note: Queue reported is the number of cars per lane.

9-10am Peak Hour - Project 2030 July Peak (Mitigated Lanes)

Harris Quarry EIR
County of Mendocino

Level Of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 US 101/Quarry Access

Average Delay (sec/veh): 1.0 Worst Case Level Of Service: C [16.5]

Street Name: US 101

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Uncontrolled Uncontrolled
Rights: Include Include Include Include
Lanes: 1 0 2 0 0 0 0 1 1 0 0 0 1 1 0 0 0 0 0 0

Volume Module:

Table with 18 columns: Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap, Level Of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Queue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS

Critical Gap Module:

Table with 2 columns: Critical Gap, FollowUpTim

Capacity Module:

Table with 2 columns: Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap

Level Of Service Module:

Table with 2 columns: 2Way95thQ, Control Del, LOS by Move, Movement, Shared Queue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS

Note: Queue reported is the number of cars per lane.

11-12pm Peak Hour - Project 2030 July Peak (Mitigated Lanes)

Harris Quarry EIR
County of Mendocino

Level Of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart
Average Delay (sec/veh): 0.6 Worst Case Level Of Service: D [25.1]

Table with columns: Street Name, Approach, Movement, Control, Rights, Lanes, Volume Module, Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap, Level Of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Queue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

11-12pm Peak Hour - Project 2030 July Peak (Mitigated Lanes)

Harris Quarry EIR
County of Mendocino

Level Of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 US 101/Quarry Access
Average Delay (sec/veh): 0.9 Worst Case Level Of Service: C [21.7]

Table with columns: Street Name, Approach, Movement, Control, Rights, Lanes, Volume Module, Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap, Level Of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Queue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

4:45-5:45pm Peak Hour - Project 2030 July Peak (Mitigated Lanes)

Harris Quarry EIR

County of Mendocino

Level of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

***** Intersection #1 US 101/Black Bart *****

Average Delay (sec/veh): 0.4 Worst Case Level of Service: C [19.6]

Table with columns: Street Name, Approach, Movement, Control, Rights, Lanes, Volume Module, Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap, Level of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Queue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS

Table with columns: Street Name, Approach, Movement, Control, Rights, Lanes, Volume Module, Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap, Level of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Queue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS

Note: Queue reported is the number of cars per lane.

4:45-5:45pm Peak Hour - Project 2030 July Peak (Mitigated Lanes)

Harris Quarry EIR

County of Mendocino

Level of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

***** Intersection #2 US 101/Quarry Access *****

Average Delay (sec/veh): 0.1 Worst Case Level of Service: C [19.1]

Table with columns: Street Name, Approach, Movement, Control, Rights, Lanes, Volume Module, Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap, Level of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Queue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS

Table with columns: Street Name, Approach, Movement, Control, Rights, Lanes, Volume Module, Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap, Level of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Queue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS

Note: Queue reported is the number of cars per lane.

9-10am Peak Hour - Project 2030 October Peak (Mitigated Lanes)

Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart
Average Delay (sec/veh): 0.5 Worst Case Level of Service: B [13.9]

Table with columns: Street Name, Approach, Movement, Control, Rights, Lanes, Volume Module, Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap, Level of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Queue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

9-10am Peak Hour - Project 2030 October Peak (Mitigated Lanes)

Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 US 101/Quarry Access
Average Delay (sec/veh): 1.4 Worst Case Level of Service: B [14.9]

Table with columns: Street Name, Approach, Movement, Control, Rights, Lanes, Volume Module, Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap, Level of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Queue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

11-12pm Peak Hour - Project 2030 October Peak (Mitigated Lanes)
 Harris Quarry EIR
 County of Mendocino

Level of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

 Intersection #1 US 101/Black Bart

Average Delay (sec/veh): 0.6 Worst Case Level of Service: C [19.0]

Street Name: US 101

Approach: North Bound South Bound East Bound West Bound

Movement: L T R L T R L T R L T R L T R

Control: Uncontrolled Uncontrolled Uncontrolled Uncontrolled Stop Sign Stop Sign

Rights: Include Include Include Include Include Include

Lanes: 1 0 2 0 0 0 2 0 1 0 0 1 1 0 0 0 0 0 0 0

Volume Module:

Base Vol:	20	887	0	0	871	33	38	0	12	0	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	20	887	0	0	871	33	38	0	12	0	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	20	887	0	0	871	33	38	0	12	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0	0
Final Volume:	20	887	0	0	871	33	38	0	12	0	0	0	0

Critical Gap Module:

Critical Gap:	4.1	xxxx	xxxx	xxxx	xxxx	xxxx	6.8	6.5	6.9	xxxx	xxxx	xxxx	xxxx
FollowUpTim:	2.2	xxxx	xxxx	xxxx	xxxx	xxxx	3.5	4.0	3.3	xxxx	xxxx	xxxx	xxxx

Capacity Module:

Conflict Vol:	904	xxxx	xxxx	xxxx	xxxx	1355	1798	436	xxxx	xxxx	xxxx	xxxx	xxxx
Potent Cap.:	761	xxxx	xxxx	xxxx	xxxx	143	81	574	xxxx	xxxx	xxxx	xxxx	xxxx
Move Cap.:	761	xxxx	xxxx	xxxx	xxxx	140	79	574	xxxx	xxxx	xxxx	xxxx	xxxx
Total Cap.:	xxxx	xxxx	xxxx	xxxx	xxxx	267	198	xxxx	212	189	xxxx	xxxx	xxxx
Volume/Cap.:	0.03	xxxx	xxxx	xxxx	xxxx	0.14	0.00	0.02	xxxx	xxxx	xxxx	xxxx	xxxx

Level of Service Module:

2Way95thQ:	0.1	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx
Control Del:	9.9	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx
LOS by Move:	A	*	*	*	*	*	*	*	*	*	*	*	*
Movement:	LT - LTR	RT	LT - LTR	RT	LT - LTR	RT	LT - LTR	RT	LT - LTR	RT	LT - LTR	RT	RT
Shared Cap.:	xxxx	xxxx	xxxx	xxxx	xxxx	307	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx
SharedQueue:	xxxx	xxxx	xxxx	xxxx	xxxx	0.6	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx
Shrd ConDel:	xxxx	xxxx	xxxx	xxxx	xxxx	19.0	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx
Shared LOS:	*	*	*	*	*	C	*	*	*	*	*	*	*
ApproachDel:	xxxxxx	*	xxxxxx	*	xxxxxx	19.0	xxxxxx	*	xxxxxx	*	xxxxxx	*	xxxxxx
ApproachLOS:	*	*	*	*	*	C	*	*	*	*	*	*	*

Note: Queue reported is the number of cars per lane.

11-12pm Peak Hour - Project 2030 October Peak (Mitigated Lanes)
 Harris Quarry EIR
 County of Mendocino

Level of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

 Intersection #2 US 101/Quarry Access

Average Delay (sec/veh): 1.3 Worst Case Level of Service: C [18.4]

Street Name: US 101

Approach: North Bound South Bound East Bound West Bound

Movement: L T R L T R L T R L T R L T R

Control: Uncontrolled Uncontrolled Uncontrolled Uncontrolled Stop Sign Stop Sign

Rights: Include Include Include Include Include Include

Lanes: 1 0 2 0 0 0 2 0 1 0 0 0 1 1 0 0 0 0 0 0

Volume Module:

Base Vol:	56	863	0	0	839	44	44	0	56	0	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	56	863	0	0	839	44	44	0	56	0	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	56	863	0	0	839	44	44	0	56	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0	0
Final Volume:	56	863	0	0	839	44	44	0	56	0	0	0	0

Critical Gap Module:

Critical Gap:	4.1	xxxx	xxxx	xxxx	xxxx	xxxx	6.8	6.5	6.9	xxxx	xxxx	xxxx	xxxx
FollowUpTim:	2.2	xxxx	xxxx	xxxx	xxxx	xxxx	3.5	4.0	3.3	xxxx	xxxx	xxxx	xxxx

Capacity Module:

Conflict Vol:	883	xxxx	xxxx	xxxx	xxxx	1405	1836	442	xxxx	xxxx	xxxx	xxxx	xxxx
Potent Cap.:	775	xxxx	xxxx	xxxx	xxxx	133	77	569	xxxx	xxxx	xxxx	xxxx	xxxx
Move Cap.:	775	xxxx	xxxx	xxxx	xxxx	126	71	569	xxxx	xxxx	xxxx	xxxx	xxxx
Total Cap.:	xxxx	xxxx	xxxx	xxxx	xxxx	254	187	xxxx	186	173	xxxx	xxxx	xxxx
Volume/Cap.:	0.07	xxxx	xxxx	xxxx	xxxx	0.17	0.00	0.10	xxxx	xxxx	xxxx	xxxx	xxxx

Level of Service Module:

2Way95thQ:	0.2	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx
Control Del:	10.0	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx
LOS by Move:	B	*	*	*	*	*	*	*	*	*	*	*	*
Movement:	LT - LTR	RT	LT - LTR	RT	LT - LTR	RT	LT - LTR	RT	LT - LTR	RT	LT - LTR	RT	RT
Shared Cap.:	xxxx	xxxx	xxxx	xxxx	xxxx	368	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx
SharedQueue:	xxxx	xxxx	xxxx	xxxx	xxxx	1.1	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx
Shrd ConDel:	xxxx	xxxx	xxxx	xxxx	xxxx	18.4	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx
Shared LOS:	*	*	*	*	*	C	*	*	*	*	*	*	*
ApproachDel:	xxxxxx	*	xxxxxx	*	xxxxxx	18.4	xxxxxx	*	xxxxxx	*	xxxxxx	*	xxxxxx
ApproachLOS:	*	*	*	*	*	C	*	*	*	*	*	*	*

Note: Queue reported is the number of cars per lane.

1:15-2:15pm Peak Hour - Project 2030 October Peak (Mitigated Lanes)
Harris Quarry EIR
County of Mendocino

Level Of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart
Average Delay (sec/veh): 0.6 Worst Case Level Of Service: C [19.9]

Street Name: US 101 Black Bart
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Uncontrolled Uncontrolled Stop Sign Include Stop Sign Include
Rights: 1 0 2 0 0 0 0 2 0 1 0 0 1 0 0 0 0 0 0 0 0
Lanes: 1 0 2 0 0 0 0 2 0 1 0 0 1 0 0 0 0 0 0 0 0

Volume Module:
Base Vol: 24 1029 0 0 876 33 38 0 12 0 0 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Critical Gap Module:
Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx xxxxx
FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx xxxxx

Capacity Module:
Conflict Vol: 909 xxxxx xxxxx xxxxx xxxxx 1439 1953 438 xxxxx xxxxx xxxxx
Potential Cap.: 757 xxxxx xxxxx xxxxx xxxxx 126 65 572 xxxxx xxxxx xxxxx

Level Of Service Module:
2Way95thQ: 0.1 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Control Del: 9.9 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx

Note: Queue reported is the number of cars per lane.

1:15-2:15pm Peak Hour - Project 2030 October Peak (Mitigated Lanes)
Harris Quarry EIR
County of Mendocino

Level Of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 US 101/Quarry Access
Average Delay (sec/veh): 0.9 Worst Case Level Of Service: C [17.5]

Street Name: US 101 Quarry Access
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Uncontrolled Uncontrolled Stop Sign Include Stop Sign Include
Rights: 1 0 2 0 0 0 0 2 0 1 0 0 0 0 1 0 0 0 0 0 0 0
Lanes: 1 0 2 0 0 0 0 2 0 1 0 0 0 0 1 0 0 0 0 0 0 0

Volume Module:
Base Vol: 44 1021 0 0 856 32 32 0 44 0 0 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Critical Gap Module:
Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx xxxxx
FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx xxxxx

Capacity Module:
Conflict Vol: 888 xxxxx xxxxx xxxxx xxxxx 1471 1981 444 xxxxx xxxxx xxxxx
Potential Cap.: 771 xxxxx xxxxx xxxxx xxxxx 120 62 567 xxxxx xxxxx xxxxx

Level Of Service Module:
2Way95thQ: 0.2 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Control Del: 10.0 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx

Note: Queue reported is the number of cars per lane.

4:45-5:45pm Peak Hour - Project 2030 October Peak (Mitigated Lanes)
Harris Quarry EIR
County of Mendocino

Level Of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart
Average Delay (sec/veh): 0.4 Worst Case Level Of Service: C [15.2]
Street Name: US 101 Black Bart
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
Rights: Include Include Include Include
Lanes: 1 0 2 0 0 0 2 0 1 0 0 1 0 0 0 0 0 0 0
Volume Module:
Base Vol: 26 1069 0 0 838 27 15 0 24 0 0 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 26 1069 0 0 838 27 15 0 24 0 0 0
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 26 1069 0 0 838 27 15 0 24 0 0 0
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
FinalVolume: 26 1069 0 0 838 27 15 0 24 0 0 0

Critical Gap Module:
Critical Gap: 4.1 xxxxxx xxxxxx xxxxxx xxxxxx 6.8 6.5 6.9 xxxxxx xxxxxx xxxxxx
FollowUpTim: 2.2 xxxxxx xxxxxx xxxxxx xxxxxx 3.5 4.0 3.3 xxxxxx xxxxxx xxxxxx

Capacity Module:
Conflict Vol: 865 xxxxx xxxxxx xxxxx xxxxx 1425 1959 419 xxxxx xxxxx xxxxxx
Potential Cap.: 787 xxxxx xxxxxx xxxxx xxxxx 129 64 589 xxxxx xxxxx xxxxxx
Move Cap.: 787 xxxxx xxxxxx xxxxx xxxxx 126 62 589 xxxxx xxxxx xxxxxx
Total Cap.: xxxxx xxxxx xxxxxx xxxxx xxxxx 257 174 xxxxxx 167 167 xxxxxx
Volume/Cap: 0.03 xxxxx xxxxx xxxxx xxxxx 0.06 0.00 0.04 xxxxx xxxxx xxxxxx

Level Of Service Module:
2Way95thQ: 0.1 xxxxx xxxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Control Del: 9.7 xxxxx xxxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
LOS by Move: A *
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
SharedQueue: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Shrd ConDel: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 15.2 xxxxx xxxxx xxxxx xxxxx
Shared LOS: * * * * * * * * * * * C * * * * * * * * * *
ApproachDel: xxxxxx xxxxxx 15.2 xxxxxx *
ApproachLOS: * * * * * * * * * * * C * * * * * * * * * * *

Note: Queue reported is the number of cars per lane.

4:45-5:45pm Peak Hour - Project 2030 October Peak (Mitigated Lanes)
Harris Quarry EIR
County of Mendocino

Level Of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 US 101/Quarry Access
Average Delay (sec/veh): 0.1 Worst Case Level Of Service: C [15.3]
Street Name: US 101 Quarry Access
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
Rights: Include Include Include Include
Lanes: 1 0 2 0 0 0 0 1 1 0 0 0 1 0 0 0 0 0 0 0
Volume Module:
Base Vol: 0 1089 0 0 862 0 6 0 0 6 0 0 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 0 1089 0 0 862 0 6 0 0 6 0 0 0
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 0 1089 0 0 862 0 6 0 0 6 0 0 0
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0
FinalVolume: 0 1089 0 0 862 0 6 0 0 6 0 0 0

Critical Gap Module:
Critical Gap: xxxxxx xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxxx xxxxx xxxxx
FollowUpTim: xxxxxx xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxxx xxxxx xxxxx

Capacity Module:
Conflict Vol: xxxxx xxxxx xxxxx xxxxx xxxxx 1407 1951 431 xxxxx xxxxx xxxxxx
Potential Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx 132 65 578 xxxxx xxxxx xxxxxx
Move Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx 132 65 578 xxxxx xxxxx xxxxxx
Total Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx 262 180 xxxxxx 180 180 xxxxxx
Volume/Cap: xxxxx xxxxx xxxxx xxxxx xxxxx 0.02 0.00 0.01 xxxxx xxxxx xxxxx

Level Of Service Module:
2Way95thQ: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Control Del: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
LOS by Move: *
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 361 xxxxx xxxxx xxxxx xxxxx
SharedQueue: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 0.1 xxxxx xxxxx xxxxx xxxxx
Shrd ConDel: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 15.3 xxxxx xxxxx xxxxx xxxxx
Shared LOS: * * * * * * * * * * * C * * * * * * * * * *
ApproachDel: xxxxxx xxxxxx 15.3 xxxxxx *
ApproachLOS: * * * * * * * * * * * C * * * * * * * * * * *

Note: Queue reported is the number of cars per lane.

9-10am Peak Hour - Project 2040 July Peak (Mitigated Lanes)
Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart
Average Delay (sec/veh): 0.5 Worst Case Level of Service: C [18.3]

Table with columns: Street Name, Approach, Movement, Control, Rights, Lanes, Volume Module, Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap, Level of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Queue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

9-10am Peak Hour - Project 2040 July Peak (Mitigated Lanes)
Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 US 101/Quarry Access
Average Delay (sec/veh): 1.0 Worst Case Level of Service: C [18.4]

Table with columns: Street Name, Approach, Movement, Control, Rights, Lanes, Volume Module, Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap, Level of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Queue, Shrd Condel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

1:15-2:15pm Peak Hour - Project 2040 July Peak (Mitigated Lanes)
Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart
Average Delay (sec/veh): 0.5 Worst Case Level of Service: C [23.2]
Street Name: US 101
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Uncontrolled Uncontrolled Uncontrolled Uncontrolled Stop Sign
Rights: Include Include Include Include Include
Lanes: 1 0 2 0 0 0 2 0 1 0 0 1 0 0 0 0 0 0 0

Volume Module:
Base Vol: 27 1510 0 0 1266 36 19 0 27 0 0 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 27 1510 0 0 1266 36 19 0 27 0 0 0
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 27 1510 0 0 1266 36 19 0 27 0 0 0
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0
FinalVolume: 27 1510 0 0 1266 36 19 0 27 0 0 0

Critical Gap Module:
Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx
FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx
Capacity Module:
Conflict Vol: 1302 xxxxx xxxxx xxxxx xxxxx 2075 2830 633 xxxxx xxxxx
Potential Cap.: 539 xxxxx xxxxx xxxxx xxxxx 48 18 427 xxxxx xxxxx
Move Cap.: 539 xxxxx xxxxx xxxxx xxxxx 46 17 427 xxxxx xxxxx
Total Cap: xxxxx xxxxx xxxxx xxxxx xxxxx 152 97 xxxxx 87 91 xxxxx
Volume/Cap: 0.05 xxxxx xxxxx xxxxx xxxxx 0.13 0.00 0.06 xxxxx xxxxx

Level of Service Module:
2Way95thQ: 0.2 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Control Del: 12.0 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
LOS by Move: B *
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 244 xxxxx xxxxx xxxxx
SharedQueue: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 0.7 xxxxx xxxxx xxxxx
Shrd ConDel: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 23.2 xxxxx xxxxx xxxxx
Shared LOS: *
ApproachDel: xxxxxx *
ApproachLOS: *

Note: Queue reported is the number of cars per lane.

1:15-2:15pm Peak Hour - Project 2040 July Peak (Mitigated Lanes)
Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 US 101/Quarry Access
Average Delay (sec/veh): 0.7 Worst Case Level of Service: D [26.5]
Street Name: US 101
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Uncontrolled Uncontrolled Uncontrolled Uncontrolled Stop Sign
Rights: Include Include Include Include Include
Lanes: 1 0 2 0 0 0 0 1 1 0 0 0 1 1 0 0 0 0 0 0

Volume Module:
Base Vol: 32 1509 0 0 1265 28 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 32 1509 0 0 1265 28 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 32 1509 0 0 1265 28 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0 0
FinalVolume: 32 1509 0 0 1265 28 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Critical Gap Module:
Critical Gap: 4.1 xxxxx xxxxx xxxxx xxxxx 6.8 6.5 6.9 xxxxx xxxxx
FollowUpTim: 2.2 xxxxx xxxxx xxxxx xxxxx 3.5 4.0 3.3 xxxxx xxxxx
Capacity Module:
Conflict Vol: 1293 xxxxx xxxxx xxxxx xxxxx 2098 2852 647 xxxxx xxxxx
Potential Cap.: 543 xxxxx xxxxx xxxxx xxxxx 46 17 419 xxxxx xxxxx
Move Cap.: 543 xxxxx xxxxx xxxxx xxxxx 44 16 419 xxxxx xxxxx
Total Cap: xxxxx xxxxx xxxxx xxxxx xxxxx 148 95 xxxxx 85 89 xxxxx
Volume/Cap: 0.06 xxxxx xxxxx xxxxx xxxxx 0.19 0.00 0.08 xxxxx xxxxx

Level of Service Module:
2Way95thQ: 0.2 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Control Del: 12.0 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
LOS by Move: B *
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 226 xxxxx xxxxx xxxxx
SharedQueue: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 1.0 xxxxx xxxxx xxxxx
Shrd ConDel: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 26.5 xxxxx xxxxx xxxxx
Shared LOS: *
ApproachDel: xxxxxx *
ApproachLOS: *

Note: Queue reported is the number of cars per lane.

4:45-5:45pm Peak Hour - Project 2040 October Peak (Mitigated Lanes)

Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 US 101/Black Bart
Average Delay (sec/veh): 0.5 Worst Case Level of Service: C [16.9]

Table with columns: Street Name (US 101), Approach (North, South, East, West), Movement (L-T-R, L-T-R, L-T-R, L-T-R), Control (Uncontrolled, Uncontrolled, Uncontrolled), Rights (Include, Include, Include), Lanes (1 0 2 0 0, 0 0 2 0 1, 0 0 1 1 0 0, 0 0 0 0 0), Volume Module, Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap, Level of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

4:45-5:45pm Peak Hour - Project 2040 October Peak (Mitigated Lanes)

Harris Quarry EIR
County of Mendocino

Level of Service Computation Report
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 US 101/Quarry Access
Average Delay (sec/veh): 0.1 Worst Case Level of Service: C [16.9]

Table with columns: Street Name (US 101), Approach (North, South, East, West), Movement (L-T-R, L-T-R, L-T-R, L-T-R), Control (Uncontrolled, Uncontrolled, Uncontrolled), Rights (Include, Include, Include), Lanes (1 0 2 0 0, 0 0 1 1 0, 0 0 1 1 0 0, 0 0 0 0 0), Volume Module, Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, Critical Gap, FollowUpTim, Capacity Module, Conflict Vol, Potent Cap, Move Cap, Total Cap, Volume/Cap, Level of Service Module, 2Way95thQ, Control Del, LOS by Move, Movement, Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

Appendix I

Caltrans Letters Regarding Turn Lanes

DEPARTMENT OF TRANSPORTATION

DISTRICT 1, P. O. BOX 3700
EUREKA, CA 95502-3700
PHONE (707) 441-2009
FAX (707) 441-5869
TTY (Teletypewriter #707-445-6463)



*Flex your power!
Be energy efficient!*

August 8, 2006

1-MEN-101-40.88
Harris Quarry Access
APN: 147-180-07

Jason McConnell
Northern Aggregates, Inc
P.O. Box 1566
Willits, CA 95490

Dear Mr. McConnell,

Since our meeting on July 17, 2006, our office has had the chance to review the proposed access plan, traffic, and site distance data provided by Northern Aggregates for the Harris Quarry Expansion project. The project proposes to use the site's existing access for all Quarry traffic. Turn lanes would be constructed for left and right turn movements on US 101 to accommodate northbound and southbound traffic, respectively. The width of the existing access would remain the same, but would be relocated further to the north to decrease the slope of the driveway and better facilitate trucks turning on and off of US 101. We have the following comments:

- We agree with the need for a 200-foot (approximate) southbound right-turn lane at the Northern Aggregates access driveway.
- We agree with the provision of a left turn lane for northbound traffic at this location as it is likely to be an improvement over the existing situation, however, the 218-foot left turn lane does not provide adequate deceleration and storage lengths. A total of 591 feet will be required to accommodate both a deceleration lane length of 493 feet (per Table 405B of the Caltrans Highway Design Manual for 55 mile per hour traffic) and a storage length of 98 feet.
- The existing access opening serving the project is 30 feet wide. As discussed in our July 17th meeting, we will consider adjusting the location of the project access, however, the ultimate driveway width will be limited to 30 feet. The District will need to prepare a Director's Deed in order to modify the location of the access opening. The final location of the access opening will need to be determined and agreed upon before Northern Aggregates submits a formal encroachment permit application to the District.
- Any work within the Caltrans right of way will require an encroachment permit. As part of the permit review process, we will require an engineering report this project. Upon receipt of an encroachment permit application, our District Permit Engineer will determine which of the following engineering reports will need to be prepared: Permit Engineering Evaluation Report (PEER), Combined Project Study Report/Project Report (PSR/PR), or Project Report

Mr. Jason McConnell

08/08/06

Page 2

(PR). These reports address issues affecting operation, maintenance, and tort liability on the State highway, as well as document conformance of proposed work to current Caltrans policies, practices, and standards. The type of engineering report selected will depend on a number of factors, including project cost and project complexity. PEER documents are typically prepared by Caltrans for non-complex, non-controversial State highway projects that cost under \$1,000,000 for construction, while PSR/PR and PR documents are prepared by a registered engineer representing the applicant for complex projects that cost over \$1,000,000. Please contact John Carson, Chief of the District 1 Permits Office, at (707) 445-6385 for more information about the permit requirements.

We look forward to continuing cooperation with you on this project. If you have questions or need further assistance, please contact me at the number above, or contact Lezlie Kimura of District 1 Community Planning at (707) 441-4542.

Sincerely,

A handwritten signature in black ink, appearing to read 'Jesse Robertson', with a long horizontal flourish extending to the right.

Jesse Robertson
Associate Transportation Planner
District 1 Community Planning

c: Nash Gonzalez, County of Mendocino

DEPARTMENT OF TRANSPORTATION

DISTRICT 1, P. O. BOX 3700
EUREKA, CA 95502-3700
PHONE (707) 441-2009
FAX (707) 441-5869
TTY (Teletypewriter #707-445-6463)



*Flex your power!
Be energy efficient!*

September 18, 2006

1-MEN-101-40.88
Harris Quarry Access
APN: 147-180-07

Jason McConnell
Northern Aggregates, Inc
P.O. Box 1566
Willits, CA 95490

Dear Mr. McConnell,

At your request, we have revisited our previous written comments (August 8, 2006) requiring 591 feet for the proposed left turn lane at Northern Aggregates' access driveway, to be located on northbound US 101 near Black Bart Drive.

We will not support Northern Aggregates' proposal to construct a 218-foot left turn lane, based on the information we have now, as it does not meet State standards. We have, however, revised our requirement from August 8, and reduced the required left-turn lane length from 591 feet to 469 feet. The Highway Design Manual (HDM) allows for some deceleration (10-20 mph) to take place on the through lanes to reduce the length of the deceleration lane (HDM Index 405.2d). Using this criterion, we would support a reduced design speed from the posted 65 mph to minimum speed of 43.5 mph, resulting in a deceleration lane length of 371 feet. The total length of the 469-foot turn lane includes the revised 371 feet of deceleration lane length plus 98 feet of storage length.

If you wish to pursue an exception for a reduced deceleration lane length, you will have to demonstrate that the proposed length is either infeasible or appropriate, based on prevailing speeds, highway grade, widening constraints, etc. Any assertions made in support of reduced lane length must be based on substantive, independently verifiable information.

If you have questions or need further assistance, please contact me at the number above, or contact Lezlie Kimura of District 1 Community Planning at (707) 441-4542.

Sincerely,

A handwritten signature in black ink, appearing to read "Jesse Robertson".

Jesse Robertson
Associate Transportation Planner
District 1 Community Planning

c: Nash Gonzalez, County of Mendocino

Mr. Jason McConnell

09/18/06

Page 2

bc: 1. CSWillis
2. RAJackman
3. IGR
DSCampbell
JPCarson
TAArseneau
1. RMMartinelli
2. DAMorgan
Correspondence File
CHRON

JGR/lmk

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Appendix J

Ramp Level of Service Calculations

RAMPS AND RAMP JUNCTIONS WORKSHEET													
General Information						Site Information							
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101	Junction	Harris Quarry Driveway	Agency or Company	W-Trans	Jurisdiction	Memoronto County/Citizens	Date Performed	11/24/2009	Analysis Year	2010
Project Description: Harris Quarry - Project Conditions													
Terrain: Rolling													
Upstream Adj Ramp	<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off $L_{down} = ft$ $V_u = veh/h$												
Downstream Adj Ramp	<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off $L_{down} = ft$ $V_d = veh/h$												
Conversion to pch Under Base Conditions $S_{FF} = 65.0 \text{ mph}$ $S_{FR} = 35.0 \text{ mph}$ Sketch (show lanes, L_A , L_D , V_R)													
(pch)	V (veh/h)	PHF	Terrain	%Truck	%RV	f_p	f_{HV}	f_p	f_{HV}	f_p	f_{HV}	$V = VIPPHF \times f_p \times f_{HV}$	
Freeway	820	0.95	Rolling	8	2	0.877	1.00	984				984	
Ramp	44	0.95	Rolling	0	0	1.000	1.00	46				46	
UpStream													
DownStream													
Merge Areas													
Diverge Areas													
Estimation of V_{12} $V_{12} = V_R + (V_F - V_R) P_{FM}$ (Equation 25-8 or 25-9) 1,000 using Equation (Exhibit 25-5) $V_{12} = pch$ (Equation 25-12) $V_{12} = 984 \text{ pch}$ $V_{12} = 984 \text{ pch}$ $V_{12} = 984 \text{ pch}$ Is V_3 or $V_{a34} > 2,700 \text{ pch}$? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V_3 or $V_{a34} > 1.5 \cdot V_{12}$? <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, $V_{12a} = pch$ (Equation 25-18)													
Capacity Checks													
Actual Capacity LOS F? Actual Capacity LOS F? $V_{FO} = V_F$ Exhibit 25-7 984 4700 No V_R Exhibit 25-3 938 4700 No V_{R12} Exhibit 25-3 46 2000 No													
Flow Entering Merge Influence Area													
Actual Max Desirable Violation? Actual Max Desirable Violation? V_{R12} Exhibit 25-7 25-7 984 4400/All No V_{12} Exhibit 25-7 984 4400/All No													
Level of Service Determination (if not F)													
$D_R = 5.475 + 0.00734 \cdot V_R + 0.0078$ $D_R = 5.2$ (pcmi/mi) $LOS = A$ (Exhibit 25-4)													
Speed Determination													
$D_s = 0.432$ (Exhibit 25-19) $S_R = 55.1 \text{ mph}$ (Exhibit 25-19) $S_u = N/A \text{ mph}$ (Exhibit 25-19) $S_d = 55.1 \text{ mph}$ (Exhibit 25-19)													

RAMPS AND RAMP JUNCTIONS WORKSHEET													
General Information						Site Information							
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101	Junction	Harris Quarry Driveway	Agency or Company	W-Trans	Jurisdiction	Memoronto County/Citizens	Date Performed	11/24/2009	Analysis Year	2010
Project Description: Harris Quarry - Project Conditions													
Terrain: Rolling													
Upstream Adj Ramp	<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off $L_{down} = ft$ $V_u = veh/h$												
Downstream Adj Ramp	<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off $L_{down} = ft$ $V_d = veh/h$												
Conversion to pch Under Base Conditions $S_{FF} = 65.0 \text{ mph}$ $S_{FR} = 35.0 \text{ mph}$ Sketch (show lanes, L_A , L_D , V_R)													
(pch)	V (veh/h)	PHF	Terrain	%Truck	%RV	f_p	f_{HV}	f_p	f_{HV}	f_p	f_{HV}	$V = VIPPHF \times f_p \times f_{HV}$	
Freeway	531	0.95	Rolling	8	2	0.877	1.00	637				637	
Ramp	44	0.95	Rolling	0	0	1.000	1.00	46				46	
UpStream													
DownStream													
Merge Areas													
Diverge Areas													
Estimation of V_{12} $V_{12} = V_R + (V_F - V_R) P_{FM}$ (Equation 25-8 or 25-9) 1,000 using Equation (Exhibit 25-5) $V_{12} = pch$ (Equation 25-12) $V_{12} = 637 \text{ pch}$ $V_{12} = 637 \text{ pch}$ $V_{12} = 637 \text{ pch}$ Is V_3 or $V_{a34} > 2,700 \text{ pch}$? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V_3 or $V_{a34} > 1.5 \cdot V_{12}$? <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, $V_{12a} = pch$ (Equation 25-18)													
Capacity Checks													
Actual Capacity LOS F? Actual Capacity LOS F? $V_{FO} = V_F$ Exhibit 25-7 637 4700 No V_R Exhibit 25-3 591 4700 No V_{R12} Exhibit 25-3 46 2000 No													
Flow Entering Merge Influence Area													
Actual Max Desirable Violation? Actual Max Desirable Violation? V_{R12} Exhibit 25-7 25-7 637 4400/All No V_{12} Exhibit 25-7 637 4400/All No													
Level of Service Determination (if not F)													
$D_R = 5.475 + 0.00734 \cdot V_R + 0.0078$ $D_R = 5.2$ (pcmi/mi) $LOS = A$ (Exhibit 25-4)													
Speed Determination													
$D_s = 0.432$ (Exhibit 25-19) $S_R = 55.1 \text{ mph}$ (Exhibit 25-19) $S_u = N/A \text{ mph}$ (Exhibit 25-19) $S_d = 55.1 \text{ mph}$ (Exhibit 25-19)													

General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	W/T Trans	Junction	Harris Quarry Driveway-Offramp
Date Performed	9/4/2009	Jurisdiction	Menardocounty/Cattans
Analysis Time Period	July Peak 4:45pm-5:45pm	Analysis Year	2010
Project Description: Harris Quarry - Project Conditions			
Inputs			
Upstream Adj Ramp	Terrain: Rolling	Downstream Adj Ramp	Yes <input type="checkbox"/> On No <input type="checkbox"/> Off
S_{FF}	65.0 mph	S_{FR}	35.0 mph
S_u	veh/h	S_d	veh/h
Conversion to pch/h Under Base Conditions			
(pch)	PHF	Terrain	%Truck
Freeway	0.95	Rolling	8
Ramp	0.95	Rolling	0
UpStream			
DownStream			
Merge Areas			
Estimation of V_{12}			
$V_{12} = V_{12} (P_{FM})$	(Equation 25-2)	$V_{12} = V_R + (V_F - V_R) P_{FD}$	(Equation 25-8 or 25-9)
$V_{12} = 1034$	using Equation (Exhibit 25-5)	$V_{12} = 1241$	1000 using Equation (Exhibit 25-12)
$V_{12} = 1034$	pc/h	$V_{12} = 1241$	pc/h
V_3 or V_{a34}	pc/h (Equation 25-4 or 25-5)	V_3 or V_{a34}	0 pc/h (Equation 25-4 or 25-5)
V_3 or $V_{a34} > 2,700$ pc/h?	Yes <input type="checkbox"/> No <input type="checkbox"/>	V_3 or $V_{a34} > 2,700$ pc/h?	Yes <input type="checkbox"/> No <input type="checkbox"/>
V_3 or $V_{a34} > 1.5 \cdot V_{12}$?	Yes <input type="checkbox"/> No <input type="checkbox"/>	V_3 or $V_{a34} > 1.5 \cdot V_{12}$?	Yes <input type="checkbox"/> No <input type="checkbox"/>
If Yes, $V_{12a} =$	pc/h (Equation 25-8)	If Yes, $V_{12a} =$	pc/h (Equation 25-18)
Capacity Checks			
Actual	Capacity	Actual	Capacity
V_{FO}	Exhibit 25-7	V_F	Exhibit 25-14
V_{VR}	1241	V_R	Exhibit 25-14
V_R	0	V_R	Exhibit 25-3
Flow Entering Merge Influence Area			
Actual	Max Desirable	Actual	Max Desirable
V_{R12}	Exhibit 25-7	V_{12}	Exhibit 25-14
Level of Service Determination (if not F)			
$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$			
$D_R = 9.8$ (pc/mi/h)			
$LOS = A$ (Exhibit 25-4)			
Speed Determination			
$D_s = 0.431$ (Exhibit 25-19)			
$S_R = 55.1$ mph (Exhibit 25-19)			
$S_u = N/A$ mph (Exhibit 25-19)			
$S_d = 55.1$ mph (Exhibit 25-15)			

General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	W/T Trans	Junction	Harris Quarry Driveway-Offramp
Date Performed	9/4/2009	Jurisdiction	Menardocounty/Cattans
Analysis Time Period	July Peak 1:15pm-2:15pm	Analysis Year	2010
Project Description: Harris Quarry - Project Conditions			
Inputs			
Upstream Adj Ramp	Terrain: Rolling	Downstream Adj Ramp	Yes <input type="checkbox"/> On No <input type="checkbox"/> Off
S_{FF}	65.0 mph	S_{FR}	35.0 mph
S_u	veh/h	S_d	veh/h
Conversion to pch/h Under Base Conditions			
(pch)	PHF	Terrain	%Truck
Freeway	0.95	Rolling	8
Ramp	0.95	Rolling	0
UpStream			
DownStream			
Merge Areas			
Estimation of V_{12}			
$V_{12} = V_{12} (P_{FM})$	(Equation 25-2)	$V_{12} = V_R + (V_F - V_R) P_{FD}$	(Equation 25-8 or 25-9)
$V_{12} = 989$	using Equation (Exhibit 25-5)	$V_{12} = 1163$	1000 using Equation (Exhibit 25-12)
$V_{12} = 989$	pc/h	$V_{12} = 1163$	pc/h
V_3 or V_{a34}	pc/h (Equation 25-4 or 25-5)	V_3 or V_{a34}	0 pc/h (Equation 25-4 or 25-5)
V_3 or $V_{a34} > 2,700$ pc/h?	Yes <input type="checkbox"/> No <input type="checkbox"/>	V_3 or $V_{a34} > 2,700$ pc/h?	Yes <input type="checkbox"/> No <input type="checkbox"/>
V_3 or $V_{a34} > 1.5 \cdot V_{12}$?	Yes <input type="checkbox"/> No <input type="checkbox"/>	V_3 or $V_{a34} > 1.5 \cdot V_{12}$?	Yes <input type="checkbox"/> No <input type="checkbox"/>
If Yes, $V_{12a} =$	pc/h (Equation 25-8)	If Yes, $V_{12a} =$	pc/h (Equation 25-18)
Capacity Checks			
Actual	Capacity	Actual	Capacity
V_{FO}	Exhibit 25-7	V_F	Exhibit 25-14
V_{VR}	1129	V_R	Exhibit 25-14
V_R	34	V_R	Exhibit 25-3
Flow Entering Merge Influence Area			
Actual	Max Desirable	Actual	Max Desirable
V_{R12}	Exhibit 25-7	V_{12}	Exhibit 25-14
Level of Service Determination (if not F)			
$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$			
$D_R = 9.8$ (pc/mi/h)			
$LOS = A$ (Exhibit 25-4)			
Speed Determination			
$D_s = 0.431$ (Exhibit 25-19)			
$S_R = 55.1$ mph (Exhibit 25-19)			
$S_u = N/A$ mph (Exhibit 25-19)			
$S_d = 55.1$ mph (Exhibit 25-15)			

General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	W/T rans	Junction	Harris Quarry Driveway-Offramp
Date Performed	9/4/2009	Jurisdiction	Menardocounty/Citizens
Analysis Time Period	October Peak 11am-	Analysis Year	2010
Project Description: Harris Quarry - Project Conditions			
Inputs		Terrain: Rolling	
Upstream Adj Ramp	<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off	Downstream Adj Ramp	<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off
V_{up} = ft	S_{FF} = 65.0 mph	S_{FR} = 35.0 mph	L_{down} = ft
V_u = veh/h	Sketch (show lanes, L_{LD} , V_{LD})		
Conversion to pch/h Under Base Conditions			
(pch)	PHF	Terrain	%Truck
Freeway	0.95	Rolling	8
Ramp	0.95	Rolling	0
UpStream	56	Rolling	0
DownStream			
Merge Areas			
Estimation of V_{12}			
$V_{12} = V_{12} (P_{FM})$	(Equation 25-2)	$V_{12} = V_R + (V_F - V_R) P_{FD}$	(Equation 25-8 or 25-9)
$V_{12} = V_{FO}$	using Equation (Exhibit 25-5)	$V_{12} = V_{R12}$	1000, using Equation (Exhibit 25-12)
$V_{12} = V_{12}$	pc/h	$V_{12} = V_{12}$	755 pch
$V_{12} = V_{12}$	pc/h (Equation 25-4 or 25-5)	$V_{12} = V_{12}$	0 pch (Equation 25-15 or 25-16)
$V_{12} = V_{12}$	$V_{12} > 2,700$ pch? <input type="checkbox"/> Yes <input type="checkbox"/> No	$V_{12} = V_{12}$	$V_{12} > 2,700$ pch? <input type="checkbox"/> Yes <input type="checkbox"/> No
$V_{12} = V_{12}$	$V_{12} > 1.5 \cdot V_{12}$ <input type="checkbox"/> Yes <input type="checkbox"/> No	$V_{12} = V_{12}$	$V_{12} > 1.5 \cdot V_{12}$ <input type="checkbox"/> Yes <input type="checkbox"/> No
$V_{12} = V_{12}$	pc/h (Equation 25-8)	$V_{12} = V_{12}$	pc/h (Equation 25-18)
Capacity Checks			
Actual	Capacity	LOS F?	Capacity
V_{FO}	Exhibit 25-7		Exhibit 25-14
$V_{FO} = V_{FO}$			755
V_{R12}			696
V_{R12}			59
Flow Entering Merge Influence Area			
Actual	Max Desirable	Violation?	Actual
V_{R12}	Exhibit 25-7		Exhibit 25-14
V_{R12}			755
Level of Service Determination (if not F)			
$D_R = 5.475 + 0.00734 \cdot V_{R12} + 0.0078$			$D_R = 4.252 + 0.0086 \cdot V_{12} - 0.009 \cdot L_D$
$D_R = 6.2$ (pc/mi/h)			$P_e = 6.2$ (pc/mi/h)
$LOS = A$ (Exhibit 25-4)			$LOS = A$ (Exhibit 25-4)
Speed Determination			
$M_S =$ (Exhibit 25-19)			$D_e = 0.433$ (Exhibit 25-19)
$S_R =$ mph (Exhibit 25-19)			$S_R =$ mph (Exhibit 25-19)
$S_u =$ mph (Exhibit 25-19)			$S_u =$ mph (Exhibit 25-19)
$S_w =$ mph (Exhibit 25-14)			$S_w =$ mph (Exhibit 25-14)

General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	W/Traans	Junction	Harris Quarry Driveway-Offramp
Date Performed	9/4/2009	Jurisdiction	Menardocounty/Cattinas
Analysis Time Period	2:15pm	Analysis Year	2010
Project Description: Harris Quarry - Project Conditions			
Inputs			
Upstream Adj Ramp	Terrain: Rolling	Downstream Adj Ramp	Yes <input type="checkbox"/> On No <input type="checkbox"/> Off
S_{FF}	65.0 mph	S_{FR}	35.0 mph
V_{up}	ft	V_{down}	ft
V_u	veh/h	V_D	veh/h
Conversion to pch Under Base Conditions			
(pch)	PHF	Terrain	%Truck
Freeway	0.95	Rolling	8
Ramp	0.95	Rolling	0
UpStream			
DownStream			
Merge Areas			
Estimation of V_{12}			
$V_{12} = V_u + (P_{FM})$	(Equation 25-2)	$V_{12} = V_R + (V_F - V_R) P_{FD}$	(Equation 25-8 or 25-9)
$V_{12} = 744$	using Equation (Exhibit 25-5)	$V_{12} = 983$	1000 using Equation (Exhibit 25-12)
$V_{12} = 744$		$V_{12} = 983$	
V_3 or V_{a34}	0 pch (Equation 25-4 or 25-5)	V_3 or V_{a34}	0 pch (Equation 25-4 or 25-5)
V_3 or $V_{a34} > 2,700$ pch?	Yes <input type="checkbox"/> No <input type="checkbox"/>	V_3 or $V_{a34} > 2,700$ pch?	Yes <input type="checkbox"/> No <input type="checkbox"/>
V_3 or $V_{a34} > 1.5 \cdot V_{12}$	Yes <input type="checkbox"/> No <input type="checkbox"/>	V_3 or $V_{a34} > 1.5 \cdot V_{12}$	Yes <input type="checkbox"/> No <input type="checkbox"/>
If Yes, V_{12a}		If Yes, V_{12a}	
Capacity Checks			
Actual	Capacity	Actual	Capacity
V_F	Exhibit 25-7	V_F	Exhibit 25-14
$V_{FO} = V_F$		$V_{FO} = V_F$	
V_R	Exhibit 25-3	V_R	Exhibit 25-3
V_R	46	V_R	0
Flow Entering Merge Influence Area			
Actual	Max Desirable	Actual	Max Desirable
V_{R12}	Exhibit 25-7	V_{12}	Exhibit 25-14
V_{R12}	883	V_{12}	883
Level of Service Determination (if not F)			
$D_R = 5.475 + 0.00734 \cdot V_R + 0.0078$			
$D_R = 5.475 + 0.00734 \cdot 46 + 0.0078$			
$D_R = 5.51$ mph (Exhibit 25-19)			
$P_R = 7.4$ (pc/mi/ln)			
$LOS = A$ (Exhibit 25-4)			
Speed Determination			
$M_S = 0.432$ (Exhibit 25-19)			
$S_R = 55.1$ mph (Exhibit 25-19)			
$S_M = N/A$ mph (Exhibit 25-19)			
$S_L = 55.1$ mph (Exhibit 25-14)			
$S_S = 55.1$ mph (Exhibit 25-15)			

General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	W/Traans	Junction	Harris Quarry Driveway-Offramp
Date Performed	9/4/2009	Jurisdiction	Menardocounty/Cattinas
Analysis Time Period	2:15pm	Analysis Year	2010
Project Description: Harris Quarry - Project Conditions			
Inputs			
Upstream Adj Ramp	Terrain: Rolling	Downstream Adj Ramp	Yes <input type="checkbox"/> On No <input type="checkbox"/> Off
S_{FF}	65.0 mph	S_{FR}	35.0 mph
V_{up}	ft	V_{down}	ft
V_u	veh/h	V_D	veh/h
Conversion to pch Under Base Conditions			
(pch)	PHF	Terrain	%Truck
Freeway	0.95	Rolling	8
Ramp	0.95	Rolling	0
UpStream			
DownStream			
Merge Areas			
Estimation of V_{12}			
$V_{12} = V_u + (P_{FM})$	(Equation 25-2)	$V_{12} = V_R + (V_F - V_R) P_{FD}$	(Equation 25-8 or 25-9)
$V_{12} = 744$	using Equation (Exhibit 25-5)	$V_{12} = 983$	1000 using Equation (Exhibit 25-12)
$V_{12} = 744$		$V_{12} = 983$	
V_3 or V_{a34}	0 pch (Equation 25-4 or 25-5)	V_3 or V_{a34}	0 pch (Equation 25-4 or 25-5)
V_3 or $V_{a34} > 2,700$ pch?	Yes <input type="checkbox"/> No <input type="checkbox"/>	V_3 or $V_{a34} > 2,700$ pch?	Yes <input type="checkbox"/> No <input type="checkbox"/>
V_3 or $V_{a34} > 1.5 \cdot V_{12}$	Yes <input type="checkbox"/> No <input type="checkbox"/>	V_3 or $V_{a34} > 1.5 \cdot V_{12}$	Yes <input type="checkbox"/> No <input type="checkbox"/>
If Yes, V_{12a}		If Yes, V_{12a}	
Capacity Checks			
Actual	Capacity	Actual	Capacity
V_F	Exhibit 25-7	V_F	Exhibit 25-14
$V_{FO} = V_F$		$V_{FO} = V_F$	
V_R	Exhibit 25-3	V_R	Exhibit 25-3
V_R	87	V_R	0
Flow Entering Merge Influence Area			
Actual	Max Desirable	Actual	Max Desirable
V_{R12}	Exhibit 25-7	V_{12}	Exhibit 25-14
V_{R12}	883	V_{12}	883
Level of Service Determination (if not F)			
$D_R = 5.475 + 0.00734 \cdot V_R + 0.0078$			
$D_R = 5.475 + 0.00734 \cdot 87 + 0.0078$			
$D_R = 5.51$ mph (Exhibit 25-19)			
$P_R = 7.4$ (pc/mi/ln)			
$LOS = A$ (Exhibit 25-4)			
Speed Determination			
$M_S = 0.432$ (Exhibit 25-19)			
$S_R = 55.1$ mph (Exhibit 25-19)			
$S_M = N/A$ mph (Exhibit 25-19)			
$S_L = 55.1$ mph (Exhibit 25-14)			
$S_S = 55.1$ mph (Exhibit 25-15)			

General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	W+Trans	Junction	Harris Quarry Driveway-Offramp
Date Performed	11/24/2009	Jurisdiction	Blendocato County/Citizens 2014 without Willits Bypass
Analysis Time Period	July Peak 11am-Noon	Analysis Year	
Project Description	Harris Quarry - Project Conditions		
Inputs			
Upstream Adj Ramp	Terrain: Rolling	Downstream Adj Ramp	Yes <input type="checkbox"/> On No <input type="checkbox"/> Off
S_{FF} = 65.0 mph	S_{FR} = 35.0 mph	L_{down} = ft	L_{up} = ft
V_u = veh/h	V_d = veh/h	Sketch (show lanes, L_{LD} , V_{LR} , V_{RD})	
Conversion to pch/h Under Base Conditions			
(pch)	PHF	Terrain	%Truck
Freeway	0.95	Rolling	8
Ramp	0.95	Rolling	0
UpStream			
DownStream			
Merge Areas			
Estimation of V_{12}			
$V_{12} = V_{12} (P_{FM})$	(Equation 25-2 or 25-3)	$V_{12} = V_R + (V_F - V_R) P_{FD}$	(Equation 25-8 or 25-9)
$V_{12} = V_{FO}$	using Equation (Exhibit 25-5)	$V_{12} = V_{R12}$	1000, using Equation (Exhibit 25-12)
$V_{12} = V_{12}$	pc/h	$V_{12} = V_{12}$	1037 pc/h
$V_{12} = V_{12}$	pc/h (Equation 25-4 or 25-5)	$V_{12} = V_{12}$	0 pc/h (Equation 25-15 or 25-16)
$V_{12} = V_{12}$	Yes <input type="checkbox"/> No <input type="checkbox"/>	$V_{12} = V_{12}$	> 2,700 pc/h? Yes <input type="checkbox"/> No <input type="checkbox"/>
$V_{12} = V_{12}$	Yes <input type="checkbox"/> No <input type="checkbox"/>	$V_{12} = V_{12}$	> 1.5 * V_{12} ? Yes <input type="checkbox"/> No <input type="checkbox"/>
$V_{12} = V_{12}$	pc/h (Equation 25-8)	$V_{12} = V_{12}$	pc/h (Equation 25-18)
Capacity Checks			
Actual	Capacity	LOS F?	Capacity
V_{FO}	Exhibit 25-7		Exhibit 25-14
V_{R12}	685		4700
V_{R12}	639		4700
V_{R12}	46		2000
Flow Entering Merge Influence Area			
Actual	Max Desirable	Violation?	Actual
V_{R12}	Exhibit 25-7		Exhibit 25-14
V_{R12}	685		4400/Al
Level of Service Determination (if not F)			
$D_R = 5.475 + 0.00734 V_R + 0.0078$	$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$		
$D_R = 5.6$ (pc/mi/h)	$D_R = 5.6$ (pc/mi/h)		
$LOS = A$ (Exhibit 25-4)	$LOS = A$ (Exhibit 25-4)		
Speed Determination			
$M_S =$ (Exhibit 25-19)	$D_S = 0.432$ (Exhibit 25-19)		
$S_R =$ mph (Exhibit 25-19)	$S_R = 55.1$ mph (Exhibit 25-19)		
$S_U =$ mph (Exhibit 25-19)	$S_U = N/A$ mph (Exhibit 25-19)		
$S_D =$ mph (Exhibit 25-14)	$S_D = 55.1$ mph (Exhibit 25-15)		

General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	W/T Trans	Junction	Harris Quarry Driveway-Offramp
Date Performed	9/4/2009	Jurisdiction	Monteclaro County/Citizens
Analysis Time Period	July Peak 4:45pm-5:45pm	Analysis Year	2014 without Wifills Bypass
Project Description: Harris Quarry - Project Conditions			
Inputs			
Upstream Adj Ramp	Terrain: Rolling	Downstream Adj Ramp	Yes <input type="checkbox"/> On No <input type="checkbox"/> Off
V_{up} = ft	S_{FF} = 65.0 mph	S_{FR} = 35.0 mph	L_{down} = ft
V_u = veh/h	Sketch (show lanes, L_{LD} , V_{LR} , V_{L})		V_D = veh/h
Conversion to pch/h Under Base Conditions			
(pch)	PHF	Terrain	%Truck
Freeway	1111	0.95	Rolling
Ramp	0	0.95	Rolling
UpStream			
DownStream			
Merge Areas			
Estimation of V_{12}			
$V_{12} = V_{12} + (P_{FM})$	(Equation 25-2)	$V_{12} = V_R + (V_F - V_R) P_{FD}$	(Equation 25-8 or 25-9)
$V_{12} = V_{FO}$	using Equation (Exhibit 25-5)	$V_{12} = V_{FO}$	1000 using Equation (Exhibit 25-12)
V_{12}	1249 pch	V_{12}	1333 pch
V_3 or V_{a34}	0 pch (Equation 25-4 or 25-5)	V_3 or V_{a34}	0 pch (Equation 25-4 or 25-5)
$I_s V_3$ or $I_s V_{a34} > 2,700$ pch?	Yes <input type="checkbox"/> No <input type="checkbox"/>	$I_s V_3$ or $I_s V_{a34} > 2,700$ pch?	Yes <input type="checkbox"/> No <input type="checkbox"/>
$I_s V_3$ or $I_s V_{a34} > 1.5 \cdot V_{12}$	Yes <input type="checkbox"/> No <input type="checkbox"/>	$I_s V_3$ or $I_s V_{a34} > 1.5 \cdot V_{12}$	Yes <input type="checkbox"/> No <input type="checkbox"/>
If Yes, V_{12a}	0 pch (Equation 25-8)	If Yes, V_{12a}	0 pch (Equation 25-18)
Capacity Checks			
Actual	Capacity	LOS F?	Capacity
V_F	1249	LOS F?	1333
$V_{FO} = V_F$	Exhibit 25-7	$V_{FO} = V_F$	Exhibit 25-14
V_R	1215	V_R	1333
V_R	34	V_R	0
Flow Entering Merge Influence Area			
Actual	Max Desirable	Violation?	Actual
V_{R12}	Exhibit 25-7	Violation?	Exhibit 25-14
V_{12}	1249	V_{12}	1333
Level of Service Determination (if not F)			
$D_R = 5.475 + 0.00734 \cdot V_R + 0.0078$	$D_R = 4.252 + 0.0086 \cdot V_{12} - 0.009 \cdot L_D$	$D_R = 4.252 + 0.0086 \cdot V_{12} - 0.009 \cdot L_D$	
$D_R = 10.5$ (pcmi/mi)	$D_R = 10.5$ (pcmi/mi)	$D_R = 11.2$ (pcmi/mi)	
$LOS = B$ (Exhibit 25-4)	$LOS = B$ (Exhibit 25-4)	$LOS = B$ (Exhibit 25-4)	
Speed Determination			
$M_S = 0.431$ (Exhibit 25-19)	$M_S = 0.431$ (Exhibit 25-19)	$M_S = 0.428$ (Exhibit 25-19)	
$S_R = 55.1$ mph (Exhibit 25-19)	$S_R = 55.1$ mph (Exhibit 25-19)	$S_R = 55.2$ mph (Exhibit 25-19)	
$S_U = N/A$ mph (Exhibit 25-19)	$S_U = N/A$ mph (Exhibit 25-19)	$S_U = N/A$ mph (Exhibit 25-19)	
$S_L = 55.1$ mph (Exhibit 25-14)	$S_L = 55.1$ mph (Exhibit 25-14)	$S_L = 55.2$ mph (Exhibit 25-15)	
HCS 5.1M, Version 5.4			

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information			Site Information						
Analyst: TDH			Freeway/Dir of Travel: Northbound US 101						
Agency or Company: W+Trans			Junction: Harris Quarry Driveway-Offramp						
Date Performed: 9/15/2009			Jurisdiction: Hendon County/Collins						
Analysis Time Period: October Peak 11am-2014 without Wiffls			Analysis Year: Bypass						
Project Description: Harris Quarry - Project Conditions									
Inputs			Terrain: Rolling						
Upstream Adj Ramp: <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No <input type="checkbox"/> Off			Downstream Adj Ramp: <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No <input type="checkbox"/> Off						
S _{FF} = 65.0 mph			S _{FR} = 35.0 mph						
S _u = veh/h			S _d = veh/h						
Conversion to pch Under Base Conditions									
(pch)	V (veh/h)	PHF	Terrain	%Truck	%RV	f _{HV}	f _p	f _p	V = VIPPHF x f _p
Freeway	676	0.95	Rolling	8	2	0.877	1.00	811	
Ramp	56	0.95	Rolling	0	0	1.000	1.00	59	
Merge Areas									
Estimation of V ₁₂									
V ₁₂ = V ₁₂ (P _{FM}) (Equation 25-2)									
V ₁₂ = V _R + (V _F - V _R) P _{FD} (Equation 25-8 or 25-9)									
V ₁₂ = 526									
Capacity Checks									
LOS F? Actual Capacity									
V _F = 526									
V _{FO} = V _F = 526									
V _R = 59									
Flow Entering Merge Influence Area									
Actual Max Desirable Violation?									
V _{R12} = 59									
Level of Service Determination (if not F)									
D _R = 4.252 + 0.00627 V _R - 0.009 L _D									
D _R = 4.3 (pc/mi/h)									
LOS = A (Exhibit 25-4)									
Speed Determination									
M _S = 0.433 (Exhibit 25-19)									
S _R = 55.0 mph (Exhibit 25-19)									
S _u = N/A mph (Exhibit 25-19)									
S _d = 55.0 mph (Exhibit 25-15)									

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information			Site Information						
Analyst: TDH			Freeway/Dir of Travel: Northbound US 101						
Agency or Company: W+Trans			Junction: Harris Quarry Driveway-Offramp						
Date Performed: 9/15/2009			Jurisdiction: Hendon County/Collins						
Analysis Time Period: October Peak 9am-10am-2014 without Wiffls			Analysis Year: Bypass						
Project Description: Harris Quarry - Project Conditions									
Inputs			Terrain: Rolling						
Upstream Adj Ramp: <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No <input type="checkbox"/> Off			Downstream Adj Ramp: <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No <input type="checkbox"/> Off						
S _{FF} = 65.0 mph			S _{FR} = 35.0 mph						
S _u = veh/h			S _d = veh/h						
Conversion to pch Under Base Conditions									
(pch)	V (veh/h)	PHF	Terrain	%Truck	%RV	f _{HV}	f _p	f _p	V = VIPPHF x f _p
Freeway	438	0.95	Rolling	8	2	0.877	1.00	526	
Ramp	56	0.95	Rolling	0	0	1.000	1.00	59	
Merge Areas									
Estimation of V ₁₂									
V ₁₂ = V ₁₂ (P _{FM}) (Equation 25-2)									
V ₁₂ = V _R + (V _F - V _R) P _{FD} (Equation 25-8 or 25-9)									
V ₁₂ = 467									
Capacity Checks									
LOS F? Actual Capacity									
V _F = 467									
V _{FO} = V _F = 467									
V _R = 59									
Flow Entering Merge Influence Area									
Actual Max Desirable Violation?									
V _{R12} = 59									
Level of Service Determination (if not F)									
D _R = 4.252 + 0.00627 V _R - 0.009 L _D									
D _R = 4.3 (pc/mi/h)									
LOS = A (Exhibit 25-4)									
Speed Determination									
M _S = 0.433 (Exhibit 25-19)									
S _R = 55.0 mph (Exhibit 25-19)									
S _u = N/A mph (Exhibit 25-19)									
S _d = 55.0 mph (Exhibit 25-15)									

General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	WTrans	Junction	Harris Quarry Driveway-Offramp
Date Performed	9/15/2009	Jurisdiction	Montecito County/Citizens
Analysis Time Period	October Peak 4:45pm-5:45pm	Analysis Year	2014 without Wflits Bypass
Project Description: Harris Quarry - Project Conditions			
Inputs			
Upstream Adj Ramp	Terrain: Rolling	Downstream Adj Ramp	Yes <input type="checkbox"/> On No <input type="checkbox"/> Off
$S_{FF} = 65.0$ mph	$S_{FR} = 35.0$ mph	$L_{down} = ft$	$V_D = veh/h$
Conversion to pch Under Base Conditions			
(pch)	PHF	Terrain	%Truck
Freeway 883	0.95	Rolling	8
Ramp 0	0.95	Rolling	0
UpStream			
DownStream			
Merge Areas			
Estimation of V_{12}			
$V_{12} = V_{12} (P_{FM})$	(Equation 25-2)	$V_{12} = V_R + (V_F - V_R) P_{FD}$	(Equation 25-8 or 25-9)
$V_{12} = 1024$	using Equation (Exhibit 25-5)	$V_{12} = 1000$	using Equation (Exhibit 25-12)
$V_{12} = 1024$	pch	$V_{12} = 1024$	pch (Equation 25-4 or 25-5)
$V_{12} = 1024$	pch (Equation 25-4 or 25-5)	$V_{12} = 1024$	pch (Equation 25-15 or 25-16)
$V_{12} = 1024$	$V_{12} > 2,700$ pch? Yes <input type="checkbox"/> No	$V_{12} = 1024$	$V_{12} > 2,700$ pch? Yes <input type="checkbox"/> No
$V_{12} = 1024$	$V_{12} > 1.5 \cdot V_{12}^{1/2}$ Yes <input type="checkbox"/> No	$V_{12} = 1024$	$V_{12} > 1.5 \cdot V_{12}^{1/2}$ Yes <input type="checkbox"/> No
$V_{12} = 1024$	$V_{12} > 1.5 \cdot V_{12}^{1/2}$ pch (Equation 25-8)	$V_{12} = 1024$	$V_{12} > 1.5 \cdot V_{12}^{1/2}$ pch (Equation 25-18)
Capacity Checks			
Actual	Capacity	LOS F?	Capacity
$V_{FO} = 1024$	Exhibit 25-7		Exhibit 25-14
$V_{R} = 1024$	Exhibit 25-3		Exhibit 25-14
Flow Entering Merge Influence Area			
Actual	Max Desirable	Violation?	Actual
$V_{R12} = 1024$	Exhibit 25-7		Exhibit 25-14
Level of Service Determination (if not F)			
$D_R = 5.475 + 0.00734 \cdot V_R^{1.7} - 0.0078$		$D_R = 4.252 + 0.0086 \cdot V_{12} - 0.009 \cdot L_D$	
$D_R = 8.0$ (pcmi/mi)		$D_R = 8.6$ (pcmi/mi)	
$LOS = A$ (Exhibit 25-4)		$LOS = A$ (Exhibit 25-4)	
Speed Determination			
$M_S = 55.1$ mph (Exhibit 25-19)		$M_S = 55.2$ mph (Exhibit 25-19)	
$S_R = N/A$ mph (Exhibit 25-19)		$S_R = N/A$ mph (Exhibit 25-19)	
$S_S = 55.1$ mph (Exhibit 25-14)		$S_S = 55.2$ mph (Exhibit 25-15)	

General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	W+Trans	Junction	Harris Quarry Driveway-Offramp
Date Performed	11/24/2009	Jurisdiction	Menominee County/Citizens
Analysis Time Period	July Peak 11am-12noon	Analysis Year	2014 with Willis Bypass
Project Description: Harris Quarry - Project Conditions			
Inputs			
Upstream Adj Ramp	Terrain: Rolling	Downstream Adj Ramp	
<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off		<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off	
$L_{up} = ft$		$L_{down} = ft$	
$V_u = veh/h$		$V_d = veh/h$	
Conversion to pc/h Under Base Conditions			
(pc/h)	PHF	%Truck	%RV
Freeway	0.95	8	2
Ramp	0.95	0	0
UpStream			
DownStream			
Merge Areas			
Estimation of V_{12}			
$V_{12} = V_R + (V_F - V_R) P_{FM}$ (Equation 25-8 or 25-9)			
$L_{EQ} =$	1000, using Equation (Exhibit 25-12)		
$P_{FM} =$	0.95		
$V_{12} =$	685 pc/h		
V_3 or $V_{a34} =$	0 pc/h (Equation 25-4 or 25-5)		
$Is V_3$ or $V_{a34} > 2,700$ pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No			
$Is V_3$ or $V_{a34} > 1.5 \cdot V_{12}$? <input type="checkbox"/> Yes <input type="checkbox"/> No			
If Yes, $V_{12a} =$	685 pc/h (Equation 25-8)		
Capacity Checks			
Actual	Capacity	LOS F?	Capacity
$V_{FO} = V_F =$	Exhibit 25-7		Exhibit 25-14
$V_R =$	23		2000
Flow Entering Merge Influence Area			
Actual	Max Desirable	Violation?	
$V_{R12} =$	Exhibit 25-7		Exhibit 25-14
$V_{12} =$	685		4400/AI
Level of Service Determination (if not F)			
$D_R = 5.475 + 0.00734 \cdot V_R + 0.0078$			
$D_R = 5.6$ (pc/mi/h)			
$LOS = A$ (Exhibit 25-4)			
Speed Determination			
$D_s = 0.431$ (Exhibit 25-19)			
$S_R =$ mph (Exhibit 25-19)			
$S_u =$ mph (Exhibit 25-19)			
$S_s =$ mph (Exhibit 25-14)			

General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	W+Trans	Junction	Harris Quarry Driveway-Offramp
Date Performed	11/24/2009	Jurisdiction	Menominee County/Citizens
Analysis Time Period	July Peak 9am-10am	Analysis Year	2014 with Willis Bypass
Project Description: Harris Quarry - Project Conditions			
Inputs			
Upstream Adj Ramp	Terrain: Rolling	Downstream Adj Ramp	
<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off		<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off	
$L_{up} = ft$		$L_{down} = ft$	
$V_u = veh/h$		$V_d = veh/h$	
Conversion to pc/h Under Base Conditions			
(pc/h)	PHF	%Truck	%RV
Freeway	0.95	8	2
Ramp	0.95	0	0
UpStream			
DownStream			
Merge Areas			
Estimation of V_{12}			
$V_{12} = V_R + (V_F - V_R) P_{FM}$ (Equation 25-8 or 25-9)			
$L_{EQ} =$	1000, using Equation (Exhibit 25-12)		
$P_{FM} =$	0.95		
$V_{12} =$	685 pc/h		
V_3 or $V_{a34} =$	0 pc/h (Equation 25-4 or 25-5)		
$Is V_3$ or $V_{a34} > 2,700$ pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No			
$Is V_3$ or $V_{a34} > 1.5 \cdot V_{12}$? <input type="checkbox"/> Yes <input type="checkbox"/> No			
If Yes, $V_{12a} =$	685 pc/h (Equation 25-8)		
Capacity Checks			
Actual	Capacity	LOS F?	Capacity
$V_{FO} = V_F =$	Exhibit 25-7		Exhibit 25-14
$V_R =$	23		2000
Flow Entering Merge Influence Area			
Actual	Max Desirable	Violation?	
$V_{R12} =$	Exhibit 25-7		Exhibit 25-14
$V_{12} =$	685		4400/AI
Level of Service Determination (if not F)			
$D_R = 5.475 + 0.00734 \cdot V_R + 0.0078$			
$D_R = 5.6$ (pc/mi/h)			
$LOS = A$ (Exhibit 25-4)			
Speed Determination			
$D_s = 0.431$ (Exhibit 25-19)			
$S_R =$ mph (Exhibit 25-19)			
$S_u =$ mph (Exhibit 25-19)			
$S_s =$ mph (Exhibit 25-14)			

General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	W+Trans	Junction	Harris Quarry Driveway-Offramp
Date Performed	9/4/2009	Jurisdiction	Menardoto County/Citizens
Analysis Time Period	July Peak 4:45pm-5:45pm	Analysis Year	2014 with Willis Bypass
Project Description: Harris Quarry - Project Conditions			
Inputs			
Upstream Adj Ramp	Terrain: Rolling	Downstream Adj Ramp	
<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off		<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off	
$S_{FF} = 65.0$ mph		$S_{FR} = 35.0$ mph	
$S_u =$ veh/h		$S_d =$ veh/h	
Conversion to pch/h Under Base Conditions			
(pch)	PHF	Terrain	%Truck
Freeway	1111	Rolling	8
Ramp	0	Rolling	0
UpStream			
DownStream			
Merge Areas			
Estimation of V_{12}			
$V_{12} = V_{12} (P_{FM})$	(Equation 25-2)	$V_{12} = V_R + (V_F - V_R) P_{FD}$	(Equation 25-8 or 25-9)
$V_{12} = 1111$		$V_{12} = 1333$	
Capacity Checks			
$V_{FO} = V_F$	Exhibit 25-7	$V_{FO} = V_F$	Exhibit 25-7
$V_R = 0$		$V_R = 0$	
Flow Entering Merge Influence Area			
V_{R12}	Actual	Max Desirable	Violation?
	1333	4400/AI	No
Level of Service Determination (if not F)			
$D_R = 5.475 + 0.00734 V_R + 0.0078$		$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$	
$D_R = 5.475$		$D_R = 4.252$	
$D_R = 11.2$ (pc/ml/min)		$D_R = 11.2$ (pc/ml/min)	
$LOS = B$ (Exhibit 25-4)		$LOS = B$ (Exhibit 25-4)	
Speed Determination			
$M_S =$ (Exhibit 25-19)		$M_S = 0.428$ (Exhibit 25-19)	
$S_R =$ mph (Exhibit 25-19)		$S_R = 55.2$ mph (Exhibit 25-19)	
$S_u =$ mph (Exhibit 25-19)		$S_u = N/A$ mph (Exhibit 25-19)	
$S_d =$ mph (Exhibit 25-14)		$S_d = 55.2$ mph (Exhibit 25-15)	

General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	W+Trans	Junction	Harris Quarry Driveway-Offramp
Date Performed	9/4/2009	Jurisdiction	Menardoto County/Citizens
Analysis Time Period	July Peak 4:45pm-5:45pm	Analysis Year	2014 with Willis Bypass
Project Description: Harris Quarry - Project Conditions			
Inputs			
Upstream Adj Ramp	Terrain: Rolling	Downstream Adj Ramp	
<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off		<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off	
$S_{FF} = 65.0$ mph		$S_{FR} = 35.0$ mph	
$S_u =$ veh/h		$S_d =$ veh/h	
Conversion to pch/h Under Base Conditions			
(pch)	PHF	Terrain	%Truck
Freeway	1041	Rolling	8
Ramp	20	Rolling	0
UpStream			
DownStream			
Merge Areas			
Estimation of V_{12}			
$V_{12} = V_{12} (P_{FM})$	(Equation 25-2)	$V_{12} = V_R + (V_F - V_R) P_{FD}$	(Equation 25-8 or 25-9)
$V_{12} = 1041$		$V_{12} = 1249$	
Capacity Checks			
$V_{FO} = V_F$	Exhibit 25-7	$V_{FO} = V_F$	Exhibit 25-7
$V_R = 21$		$V_R = 21$	
Flow Entering Merge Influence Area			
V_{R12}	Actual	Max Desirable	Violation?
	1249	4400/AI	No
Level of Service Determination (if not F)			
$D_R = 5.475 + 0.00734 V_R + 0.0078$		$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$	
$D_R = 5.475$		$D_R = 4.252$	
$D_R = 10.5$ (pc/ml/min)		$D_R = 10.5$ (pc/ml/min)	
$LOS = B$ (Exhibit 25-4)		$LOS = B$ (Exhibit 25-4)	
Speed Determination			
$M_S =$ (Exhibit 25-19)		$M_S = 0.430$ (Exhibit 25-19)	
$S_R =$ mph (Exhibit 25-19)		$S_R = 55.1$ mph (Exhibit 25-19)	
$S_u =$ mph (Exhibit 25-19)		$S_u = N/A$ mph (Exhibit 25-19)	
$S_d =$ mph (Exhibit 25-14)		$S_d = 55.1$ mph (Exhibit 25-15)	

General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	W/Trans	Junction	Harris Quarry Driveway-Offramp
Date Performed	11/24/2009	Jurisdiction	Menardocounty/Cattans
Analysis Time Period	October Peak 11am	Analysis Year	2014 with Willis Bypass
Project Description: Harris Quarry - Project Conditions			
Inputs			
Upstream Adj Ramp	Terrain: Rolling	Downstream Adj Ramp	
<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off		<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off	
$L_{up} = ft$		$L_{down} = ft$	
$V_u = veh/h$		$V_d = veh/h$	
Conversion to pch Under Base Conditions			
(pch)	PHF	Terrain	%Truck
Freeway	0.95	Rolling	8
Ramp	0.95	Rolling	0
UpStream			
DownStream			
Merge Areas			
Estimation of V_{12}			
$V_{12} = V_u + (P_{FM})$	$V_{12} = V_R + (V_F - V_R) P_{FD}$		
(Equation 25-2)	(Equation 25-8 or 25-9)		
using Equation (Exhibit 25-5)	1,000 using Equation (Exhibit 25-12)		
$V_{12} = pch$	pch		
V_3 or V_{a34}	0 pch (Equation 25-4 or 25-5)		
$Is V_3$ or $V_{a34} > 2,700 pch?$	<input type="checkbox"/> Yes <input type="checkbox"/> No		
$Is V_3$ or $V_{a34} > 1.5 \cdot V_{12}?$	<input type="checkbox"/> Yes <input type="checkbox"/> No		
$Is V_3$ or $V_{a34} > 1.5 \cdot V_{12}?$	<input type="checkbox"/> Yes <input type="checkbox"/> No		
If Yes, $V_{12a} =$	pch (Equation 25-8)		
Capacity Checks			
Actual	Capacity	LOS F?	Capacity
V_F	811	Exhibit 25-14	4700
$V_{FO} = V_F$	811	Exhibit 25-14	4700
V_R	773	Exhibit 25-14	4700
V_R	38	Exhibit 25-3	2000
Flow Entering Merge Influence Area			
Actual	Max Desirable	Violation?	Actual
V_{R12}	Exhibit 25-7		V_{12}
			811
			Exhibit 25-14
			4400/Al
			No
Level of Service Determination (if not F)			
$D_R = 5.475 + 0.00734 \cdot V_R + 0.0078$			
$D_R = 4.252 + 0.0086 \cdot V_{12} - 0.009 \cdot L_D$			
$D_R = 6.7$ (pc/mi/h)			
$LOS = A$ (Exhibit 25-4)			
Speed Determination			
$M_S =$ (Exhibit 25-19)			
$S_R =$ mph (Exhibit 25-19)			
$S_U =$ mph (Exhibit 25-19)			
$S_L =$ mph (Exhibit 25-14)			

General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	W+Trans	Junction	Harris Quarry Driveway-Offramp
Date Performed	11/24/2009	Jurisdiction	Menardocounty/Cattans
Analysis Time Period	2:50pm	Analysis Year	2014 with Willis Bypass
Project Description: Harris Quarry - Project Conditions			
Inputs			
Upstream Adj Ramp	Terrain: Rolling	Downstream Adj Ramp	
<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off S _{FF} = 65.0 mph S _{FR} = 35.0 mph L _{down} = ft V _u = veh/h V _d = veh/h			
Conversion to pch Under Base Conditions			
(pch)	V (veh/h)	PHF	Terrain
Freeway	799	0.95	Rolling
Ramp	28	0.95	Rolling
UpStream			
DownStream			
Merge Areas			
Estimation of V₁₂			
V ₁₂ = V _r + (P _{FM})	V ₁₂ = V _r + (V _F - V _R) P _{FD}		
Eq (Equation 25-2)	Eq (Equation 25-8 or 25-9)		
1000 using Equation (Exhibit 25-5)	1000 using Equation (Exhibit 25-12)		
V ₁₂ = 989 pch	V ₁₂ = 989 pch		
V ₃ or V _{as34} > 2,700 pch? <input type="checkbox"/> Yes <input type="checkbox"/> No	0 pch (Equation 25-4 or 25-5)		
Is V ₃ or V _{as34} > 2,700 pch? <input type="checkbox"/> Yes <input type="checkbox"/> No	25-15 or 25-16		
Is V ₃ or V _{as34} > 1.5 * V ₁₂ ? <input type="checkbox"/> Yes <input type="checkbox"/> No	Yes <input type="checkbox"/> No		
Is V ₃ or V _{as34} > 1.5 * V ₁₂ ? <input type="checkbox"/> Yes <input type="checkbox"/> No	Yes <input type="checkbox"/> No		
If Yes, V _{12a} = pch (Equation 25-8)	pch (Equation 25-18)		
Capacity Checks			
Actual	Capacity	LOS F?	Capacity
V _F	989	25-14	4700
V _{FO} = V _F	989	Exhibit 25-14	4700
V _R	23	Exhibit 25-14	4700
V _R	23	Exhibit 25-3	2000
Flow Entering Merge Influence Area			
Actual	Max Desirable	Violation?	
V _{R12}	Exhibit 25-7		
V ₁₂	989	Exhibit 25-14	4400/Al
Level of Service Determination (if not F)			
D _R = 5.475 + 0.00734 V _R * 0.0078	D _R = 4.252 + 0.0086 V ₁₂ - 0.009 L _D		
V ₁₂ - 0.00627 L _A	P _e = 8.0 (pc/mi/h)		
D _R = (pc/mi/h)	LOS = A (Exhibit 25-4)		
Speed Determination			
M _S = (Exhibit 25-19)	D _S = 0.431 (Exhibit 25-19)		
S _R = mph (Exhibit 25-19)	S _R = 55.1 mph (Exhibit 25-19)		
S _F = mph (Exhibit 25-19)	S _F = N/A mph (Exhibit 25-19)		
S _S = mph (Exhibit 25-14)	S _S = 55.1 mph (Exhibit 25-15)		

General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	W+Trans	Junction	Harris Quarry Driveway-Offramp
Date Performed	9/4/2009	Jurisdiction	Menardocounty/Cattans
Analysis Time Period	5:45pm	Analysis Year	2014 with Willis Bypass
Project Description: Harris Quarry - Project Conditions			
Inputs			
Upstream Adj Ramp	Terrain: Rolling	Downstream Adj Ramp	
<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off S _{FF} = 65.0 mph S _{FR} = 35.0 mph L _{down} = ft V _u = veh/h V _d = veh/h			
Conversion to pch Under Base Conditions			
(pch)	V (veh/h)	PHF	Terrain
Freeway	883	0.95	Rolling
Ramp	0	0.95	Rolling
UpStream			
DownStream			
Merge Areas			
Estimation of V₁₂			
V ₁₂ = V _r + (P _{FM})	V ₁₂ = V _r + (V _F - V _R) P _{FD}		
Eq (Equation 25-2)	Eq (Equation 25-8 or 25-9)		
1000 using Equation (Exhibit 25-5)	1000 using Equation (Exhibit 25-12)		
V ₁₂ = 1024 pch	V ₁₂ = 1024 pch		
V ₃ or V _{as34} > 2,700 pch? <input type="checkbox"/> Yes <input type="checkbox"/> No	0 pch (Equation 25-4 or 25-5)		
Is V ₃ or V _{as34} > 2,700 pch? <input type="checkbox"/> Yes <input type="checkbox"/> No	25-15 or 25-16		
Is V ₃ or V _{as34} > 1.5 * V ₁₂ ? <input type="checkbox"/> Yes <input type="checkbox"/> No	Yes <input type="checkbox"/> No		
Is V ₃ or V _{as34} > 1.5 * V ₁₂ ? <input type="checkbox"/> Yes <input type="checkbox"/> No	Yes <input type="checkbox"/> No		
If Yes, V _{12a} = pch (Equation 25-8)	pch (Equation 25-18)		
Capacity Checks			
Actual	Capacity	LOS F?	Capacity
V _F	1024	25-14	4700
V _{FO} = V _F	1024	Exhibit 25-14	4700
V _R	0	Exhibit 25-3	2000
Flow Entering Merge Influence Area			
Actual	Max Desirable	Violation?	
V _{R12}	Exhibit 25-7		
V ₁₂	1024	Exhibit 25-14	4400/Al
Level of Service Determination (if not F)			
D _R = 5.475 + 0.00734 V _R * 0.0078	D _R = 4.252 + 0.0086 V ₁₂ - 0.009 L _D		
V ₁₂ - 0.00627 L _A	P _e = 8.6 (pc/mi/h)		
D _R = (pc/mi/h)	LOS = A (Exhibit 25-4)		
Speed Determination			
M _S = (Exhibit 25-19)	D _S = 0.428 (Exhibit 25-19)		
S _R = mph (Exhibit 25-19)	S _R = 55.2 mph (Exhibit 25-19)		
S _F = mph (Exhibit 25-19)	S _F = N/A mph (Exhibit 25-19)		
S _S = mph (Exhibit 25-14)	S _S = 55.2 mph (Exhibit 25-15)		

RAMPS AND RAMP JUNCTIONS WORKSHEET														
General Information					Site Information									
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101	Junction	Harris Quarry Driveway-Offramp	Agency or Company	W-Trans	Jurisdiction	Memoronto County/Citizens	Date Performed	11/24/2009	Analysis Year	2009	
Project Description: Harris Quarry - Project Conditions														
Analysis Time Period: July Peak 11am-12noon														
Analysis Year: 2009														
Project Description: Harris Quarry - Project Conditions														
Terrain: Rolling														
Upstream Adj Ramp	Downstream Adj Ramp													
<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off	<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off													
$L_{up} = ft$	$L_{down} = ft$													
$V_u = veh/h$	$V_d = veh/h$													
Conversion to pc/h Under Base Conditions														
(pc/h)	V (veh/h)	PHF	Terrain	%Truck	%RV	f_p	f_{HV}	f_p	f_{HV}	$V = VIPPHF \times f_p \times f_{HV}$				
Freeway	729	0.95	Rolling	8	2	0.877	1.00	1.00	1.00	1390				
Ramp	44	0.95	Rolling	0	0	1.000	1.00	1.00	1.00	46				
UpStream	DownStream													
Merge Areas														
Diverge Areas														
Estimation of V_{12}														
$V_{12} = V_R + (V_F - V_R) P_{FM}$ (Equation 25-8 or 25-9)														
$L_{EQ} =$	1,000 using Equation (Exhibit 25-5)													
$P_{FM} =$	0.875													
$V_{12} =$	875 pc/h													
V_3 or $V_{a34} =$	0 pc/h (Equation 25-4 or 25-5)													
$I_s V_3$ or $V_{a34} > 2,700$ pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No	<input type="checkbox"/> Yes <input type="checkbox"/> No													
$I_s V_3$ or $V_{a34} > 1.5 \cdot V_{12}^2$ <input type="checkbox"/> Yes <input type="checkbox"/> No	<input type="checkbox"/> Yes <input type="checkbox"/> No													
If Yes, $V_{12a} =$	875 pc/h (Equation 25-8)													
Capacity Checks														
Actual	Capacity	LOS F?	Actual	Capacity	LOS F?	Actual	Capacity	LOS F?	Actual	Capacity	LOS F?	Actual	Capacity	LOS F?
$V_{FO} =$	Exhibit 25-7		$V_F =$	Exhibit 25-14	4700	No	$V_{FO} = V_F - V_R =$	Exhibit 25-14	4700	No	$V_R =$	Exhibit 25-14	4700	No
$V_R =$	46		$V_R =$	46			$V_R =$	46			$V_R =$	46		
Flow Entering Merge Influence Area														
Actual	Max Desirable	Violation?	Actual	Max Desirable	Violation?	Actual	Max Desirable	Violation?	Actual	Max Desirable	Violation?	Actual	Max Desirable	Violation?
$V_{R12} =$	Exhibit 25-7		$V_{12} =$	Exhibit 25-7		$V_{12} =$	Exhibit 25-7		$V_{12} =$	Exhibit 25-7		$V_{12} =$	Exhibit 25-7	
Level of Service Determination (if not F)														
$D_R = 5.475 + 0.00734 \cdot V_R + 0.0078$														
$D_R = 7.3$ (pc/mi/h)														
$LOS = A$ (Exhibit 25-4)														
Speed Determination														
$D_s = 0.432$ (Exhibit 25-19)														
$S_R = 55.1$ mph (Exhibit 25-19)														
$S_u = N/A$ mph (Exhibit 25-19)														
$S_s = 55.1$ mph (Exhibit 25-15)														

General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	W+Trans	Junction	Harris Quarry Driveway-Offramp
Date Performed	9/4/2009	Jurisdiction	Menardocounty/Citizens
Analysis Time Period	2:15pm-2:30pm	Analysis Year	2030
Project Description: Harris Quarry - Project Conditions			
Inputs			
Upstream Adj Ramp	Terrain: Rolling	Downstream Adj Ramp	Yes <input type="checkbox"/> On No <input type="checkbox"/> Off
PHF	0.95	PHF	0.95
Terrain	Rolling	Terrain	Rolling
%Truck	0	%Truck	0
%RV	0	%RV	0
$S_{FF} = 65.0$ mph	$S_{FR} = 35.0$ mph	$S_{FF} = 65.0$ mph	$S_{FR} = 35.0$ mph
Sketch (show lanes, L_{LD} , $V_{R/V}$)			
Conversion to pch Under Base Conditions			
(pch)	V (veh/hr)	PHF	Terrain
Freeway	1329	0.95	Rolling
Ramp	32	0.95	Rolling
UpStream			
DownStream			
Estimation of V_{12}			
$V_{12} = V_R + (V_F - V_R) P_{FM}$			
(Equation 25-2)			
V_{EQ}	1995 pch		
V_{FM}	1995 pch		
V_{FD}	1995 pch		
V_{12}	1995 pch		
V_3 or V_{a34}	0 pch (Equation 25-4 or 25-5)		
V_3 or $V_{a34} > 2,700$ pch?	Yes <input type="checkbox"/> No <input type="checkbox"/>		
V_3 or $V_{a34} > 1.5 \cdot V_{12}$?	Yes <input type="checkbox"/> No <input type="checkbox"/>		
V_3 or $V_{a34} > 1.5 \cdot V_{12}$?	Yes <input type="checkbox"/> No <input type="checkbox"/>		
If Yes, V_{12a}	pch (Equation 25-8)		
Capacity Checks			
Actual	Capacity	LOS F?	Capacity
	1995		1995
$V_{FO} = V_F$	Exhibit 25-7		Exhibit 25-14
V_R	1995		Exhibit 25-14
V_R	34		Exhibit 25-3
Flow Entering Merge Influence Area			
Actual	Max Desirable	Violation?	Actual
V_{R12}	Exhibit 25-7		Exhibit 25-14
Level of Service Determination (if not F)			
$D_R = 5.475 + 0.00734 \cdot V_R + 0.0078$			
$D_R = 4.252 + 0.0086 \cdot V_{12} - 0.009 \cdot L_D$			
$D_R = 4.252 + 0.0086 \cdot V_{12} - 0.009 \cdot L_D$			
$D_R = 13.5$ (pcmi/mi)			
$L_{OS} = B$ (Exhibit 25-4)			
Speed Determination			
$M_S = 0.431$ (Exhibit 25-19)			
$S_R = 55.1$ mph (Exhibit 25-19)			
$S_M = N/A$ mph (Exhibit 25-19)			
$S_L = 55.1$ mph (Exhibit 25-15)			

General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	W+Trans	Junction	Harris Quarry Driveway-Offramp
Date Performed	9/4/2009	Jurisdiction	Menardocounty/Citizens
Analysis Time Period	5:45pm-6:00pm	Analysis Year	2030
Project Description: Harris Quarry - Project Conditions			
Inputs			
Upstream Adj Ramp	Terrain: Rolling	Downstream Adj Ramp	Yes <input type="checkbox"/> On No <input type="checkbox"/> Off
PHF	0.95	PHF	0.95
Terrain	Rolling	Terrain	Rolling
%Truck	0	%Truck	0
%RV	0	%RV	0
$S_{FF} = 65.0$ mph	$S_{FR} = 35.0$ mph	$S_{FF} = 65.0$ mph	$S_{FR} = 35.0$ mph
Sketch (show lanes, L_{LD} , $V_{R/V}$)			
Conversion to pch Under Base Conditions			
(pch)	V (veh/hr)	PHF	Terrain
Freeway	1418	0.95	Rolling
Ramp	0	0.95	Rolling
UpStream			
DownStream			
Estimation of V_{12}			
$V_{12} = V_R + (V_F - V_R) P_{FM}$			
(Equation 25-2)			
V_{EQ}	1702 pch		
V_{FM}	1702 pch		
V_{FD}	1702 pch		
V_{12}	1702 pch		
V_3 or V_{a34}	0 pch (Equation 25-4 or 25-5)		
V_3 or $V_{a34} > 2,700$ pch?	Yes <input type="checkbox"/> No <input type="checkbox"/>		
V_3 or $V_{a34} > 1.5 \cdot V_{12}$?	Yes <input type="checkbox"/> No <input type="checkbox"/>		
V_3 or $V_{a34} > 1.5 \cdot V_{12}$?	Yes <input type="checkbox"/> No <input type="checkbox"/>		
If Yes, V_{12a}	pch (Equation 25-8)		
Capacity Checks			
Actual	Capacity	LOS F?	Capacity
	1702		1702
$V_{FO} = V_F$	Exhibit 25-7		Exhibit 25-14
V_R	1702		Exhibit 25-14
V_R	0		Exhibit 25-3
Flow Entering Merge Influence Area			
Actual	Max Desirable	Violation?	Actual
V_{R12}	Exhibit 25-7		Exhibit 25-14
Level of Service Determination (if not F)			
$D_R = 5.475 + 0.00734 \cdot V_R + 0.0078$			
$D_R = 4.252 + 0.0086 \cdot V_{12} - 0.009 \cdot L_D$			
$D_R = 4.252 + 0.0086 \cdot V_{12} - 0.009 \cdot L_D$			
$D_R = 14.4$ (pcmi/mi)			
$L_{OS} = B$ (Exhibit 25-4)			
Speed Determination			
$M_S = 0.428$ (Exhibit 25-19)			
$S_R = 55.2$ mph (Exhibit 25-19)			
$S_M = N/A$ mph (Exhibit 25-19)			
$S_L = 55.2$ mph (Exhibit 25-15)			

General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	W/Tans	Junction	Harris Quarry Driveway-Offramp
Date Performed	9/4/2009	Jurisdiction	Menomonic County/Citizens
Analysis Time Period	October Peak 11am-	Analysis Year	2030
Project Description: Harris Quarry - Project Conditions			
Inputs			
Upstream Adj Ramp	Terrain: Rolling	Downstream Adj Ramp	
<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off		<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off	
$S_{FF} = 65.0$ mph		$S_{FR} = 35.0$ mph	
$S_u =$ veh/h		$S_d =$ veh/h	
Conversion to pch/h Under Base Conditions			
(pch)	PHF	Terrain	%Truck
Freeway	0.95	Rolling	8
Ramp	0.95	Rolling	0
UpStream			
DownStream			
Merge Areas			
Estimation of V_{12}			
$V_{12} = V_{12} (P_{FM})$	(Equation 25-2)	$V_{12} = V_R + (V_F - V_R) P_{FD}$	(Equation 25-8 or 25-9)
$V_{12} =$	using Equation (Exhibit 25-5)	$V_{12} =$	1000, using Equation (Exhibit 25-12)
$V_{12} =$	671 pch	$V_{12} =$	1038 pch
V_3 or V_{a34}		V_3 or V_{a34}	
V_3 or $V_{a34} > 2,700$ pch? <input type="checkbox"/> Yes <input type="checkbox"/> No		V_3 or $V_{a34} > 2,700$ pch? <input type="checkbox"/> Yes <input type="checkbox"/> No	
V_3 or $V_{a34} > 1.5 \cdot V_{12}$? <input type="checkbox"/> Yes <input type="checkbox"/> No		V_3 or $V_{a34} > 1.5 \cdot V_{12}$? <input type="checkbox"/> Yes <input type="checkbox"/> No	
If Yes, $V_{12a} =$		If Yes, $V_{12a} =$	
Capacity Checks			
Actual	Capacity	Actual	Capacity
V_F	671	V_F	1038
$V_{FO} = V_F$		$V_{FO} = V_F$	
V_R	612	V_R	977
V_R	59	V_R	59
Flow Entering Merge Influence Area			
Actual	Max Desirable	Actual	Max Desirable
V_{R12}	Exhibit 25-7	V_{R12}	Exhibit 25-14
Level of Service Determination (if not F)			
$D_R = 5.475 + 0.00734 \cdot V_R + 0.0078$		$D_R = 4.252 + 0.0086 \cdot V_{12} - 0.009 \cdot L_D$	
$D_R =$	5.5 (pc/mi/h)	$D_R =$	8.7 (pc/mi/h)
$LOS = A$ (Exhibit 25-4)		$LOS = A$ (Exhibit 25-4)	
Speed Determination			
$M_S =$ (Exhibit 25-19)		$M_S =$ (Exhibit 25-19)	
$S_R =$ mph (Exhibit 25-19)		$S_R =$ mph (Exhibit 25-19)	
$S_u =$ mph (Exhibit 25-19)		$S_u =$ mph (Exhibit 25-19)	
$S_d =$ mph (Exhibit 25-14)		$S_d =$ mph (Exhibit 25-14)	

General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	WTrans	Junction	Harris Quarry Driveway-Offramp
Date Performed	9/4/2009	Jurisdiction	Menardoto County/Citizens
Analysis Time Period	5:45pm-2:00	Analysis Year	2009
Project Description: Harris Quarry - Project Conditions			
Inputs			
Upstream Adj Ramp	Terrain: Rolling	Downstream Adj Ramp	Yes <input type="checkbox"/> On No <input type="checkbox"/> Off
S_{FF}	65.0 mph	S_{FR}	35.0 mph
S_u	veh/h	S_d	veh/h
Conversion to pch Under Base Conditions			
(pch)	PHF	Terrain	%Truck
Freeway	1089	0.95	Rolling
Ramp	0	0.95	Rolling
UpStream			
DownStream			
Merge Areas			
Estimation of V_{12}			
$V_{12} = V_{12} (P_{FM})$	(Equation 25-2)	$V_{12} = V_R + (V_F - V_R) P_{FD}$	(Equation 25-8 or 25-9)
$V_{12} = 1089$		$V_{12} = 1307$	
Capacity Checks			
Actual	Capacity	LOS F?	Capacity
V_{FO}	Exhibit 25-7		Exhibit 25-14
V_{FR}	1179		Exhibit 25-14
V_R	46		Exhibit 25-3
Flow Entering Merge Influence Area			
Actual	Max Desirable	Violation?	Actual
V_{R12}	Exhibit 25-7		Exhibit 25-14
V_{12}	1225		4400/AI
Level of Service Determination (if not F)			
$D_R = 5.475 + 0.00734 V_R + 0.0078$		$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$	
$D_R = 10.3$ (pc/ml/min)		$D_R = 11.0$ (pc/ml/min)	
LOS = B (Exhibit 25-4)		LOS = B (Exhibit 25-4)	
Speed Determination			
$M_S = 55.1$ mph (Exhibit 25-19)		$M_S = 55.2$ mph (Exhibit 25-19)	
$S_R = N/A$ mph (Exhibit 25-19)		$S_R = N/A$ mph (Exhibit 25-19)	
$S_u = 55.1$ mph (Exhibit 25-14)		$S_u = 55.2$ mph (Exhibit 25-15)	

General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	WTrans	Junction	Harris Quarry Driveway-Offramp
Date Performed	9/4/2009	Jurisdiction	Menardoto County/Citizens
Analysis Time Period	5:45pm-2:00	Analysis Year	2009
Project Description: Harris Quarry - Project Conditions			
Inputs			
Upstream Adj Ramp	Terrain: Rolling	Downstream Adj Ramp	Yes <input type="checkbox"/> On No <input type="checkbox"/> Off
S_{FF}	65.0 mph	S_{FR}	35.0 mph
S_u	veh/h	S_d	veh/h
Conversion to pch Under Base Conditions			
(pch)	PHF	Terrain	%Truck
Freeway	1021	0.95	Rolling
Ramp	44	0.95	Rolling
UpStream			
DownStream			
Merge Areas			
Estimation of V_{12}			
$V_{12} = V_{12} (P_{FM})$	(Equation 25-2)	$V_{12} = V_R + (V_F - V_R) P_{FD}$	(Equation 25-8 or 25-9)
$V_{12} = 1021$		$V_{12} = 1225$	
Capacity Checks			
Actual	Capacity	LOS F?	Capacity
V_{FO}	Exhibit 25-7		Exhibit 25-14
V_{FR}	1179		Exhibit 25-14
V_R	46		Exhibit 25-3
Flow Entering Merge Influence Area			
Actual	Max Desirable	Violation?	Actual
V_{R12}	Exhibit 25-7		Exhibit 25-14
V_{12}	1225		4400/AI
Level of Service Determination (if not F)			
$D_R = 5.475 + 0.00734 V_R + 0.0078$		$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$	
$D_R = 10.3$ (pc/ml/min)		$D_R = 11.0$ (pc/ml/min)	
LOS = B (Exhibit 25-4)		LOS = B (Exhibit 25-4)	
Speed Determination			
$M_S = 55.1$ mph (Exhibit 25-19)		$M_S = 55.2$ mph (Exhibit 25-19)	
$S_R = N/A$ mph (Exhibit 25-19)		$S_R = N/A$ mph (Exhibit 25-19)	
$S_u = 55.1$ mph (Exhibit 25-14)		$S_u = 55.2$ mph (Exhibit 25-15)	

RAMPS AND RAMP JUNCTIONS WORKSHEET												
General Information			Site Information									
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101									
Agency or Company	W+Trans	Junction	Harris Quarry Driveway-Offramp									
Date Performed	11/24/2009	Jurisdiction	Menardoto County/Citizens									
Analysis Time Period	July Peak 11am-12noon	Analysis Year	2040									
Project Description: Harris Quarry - Project Conditions												
Inputs												
Upstream Adj	Terrain: Rolling											
Ramp	<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off											
L_{up}	= ft											
V_u	= veh/h	S_{FF}	= 65.0 mph	S_{FR}	= 35.0 mph							
Sketch (show lanes, L_A , L_D , V_R)												
Conversion to pc/h Under Base Conditions												
(pc/h)	V	PHF	Terrain	%Truck	%RV	f_p	f_{HV}	f_p	f_{HV}	V	$V/PHF \times f_p \times f_{HV}$	
Freeway	828	0.95	Rolling	8	2	0.877	1.00	1.00	1.00	1532	1532	
Ramp	44	0.95	Rolling	0	0	1.000	1.00	1.00	1.00	46	46	
UpStream												
DownStream												
Merge Areas												
Estimation of V_{12}												
$V_{12} = V_R + (V_F - V_R) P_{FM}$ (Equation 25-8 or 25-9)												
L_{EQ}												
P_{FM}	using Equation (Exhibit 25-5)											
V_{12}	994 pc/h											
V_3 or V_{a34}	0 pc/h (Equation 25-4 or 25-5)											
V_3 or $V_{a34} > 2,700$ pc/h?	<input type="checkbox"/> Yes <input type="checkbox"/> No											
V_3 or $V_{a34} > 1.5 \cdot V_{12}$?	<input type="checkbox"/> Yes <input type="checkbox"/> No											
V_{12a}	994 pc/h (Equation 25-18)											
Capacity Checks												
Actual	Capacity	LOS F?	Actual	Capacity	LOS F?	Actual	Capacity	LOS F?	Actual	Capacity	LOS F?	
V_{FO}	Exhibit 25-7		V_F	Exhibit 25-14	4700	No	V_{FO}	Exhibit 25-14	4700	No	V_R	Exhibit 25-14
			V_R	Exhibit 25-14	4700	No						
			V_{12}	Exhibit 25-3	2000	No						
Flow Entering Merge Influence Area												
Actual	Max Desirable	Violation?	Actual	Max Desirable	Violation?	Actual	Max Desirable	Violation?	Actual	Max Desirable	Violation?	
V_{R12}	Exhibit 25-7		V_{12}	Exhibit 25-14	4400/All	No						
Level of Service Determination (if not F)												
$D_R = 5.475 + 0.00734 \cdot V_R + 0.0078$												
$D_R = 4.252 + 0.0086 \cdot V_{12} - 0.009 \cdot L_D$												
$D_R = 8.3$ (pc/mi/h)												
LOS = A (Exhibit 25-4)												
Speed Determination												
$D_s = 0.432$ (Exhibit 25-19)												
$S_R = 55.1$ mph (Exhibit 25-19)												
$S_u = N/A$ mph (Exhibit 25-19)												
$S_s = 55.1$ mph (Exhibit 25-15)												

General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	W/T Trans	Junction	Harris Quarry Driveway-Offramp
Date Performed	9/4/2009	Jurisdiction	Menardocounty/Citizens
Analysis Time Period	4:45pm-5:45pm	Analysis Year	2040
Project Description: Harris Quarry - Project Conditions			
Inputs		Terrain: Rolling	
Upstream Adj Ramp	<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off $L_{down} = ft$ $V_u = veh/h$	Downstream Adj Ramp	<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off $L_{down} = ft$ $V_d = veh/h$
Conversion to pch/h Under Base Conditions		Sketch (show lanes, L_{LD} , $V_{R/V}$)	
(pch)	V (veh/h)	PHF	Terrain
Freeway	1610	0.95	Rolling
Ramp	0	0.95	Rolling
DownStream			
Merge Areas			
Estimation of V_{12}			
$V_{12} = V_R + (V_F - V_R) P_{FM}$ (Equation 25-2)			
$V_{12} = V_R + (V_F - V_R) P_{FD}$ (Equation 25-9) 1000 using Equation (Exhibit 25-12)			
$V_{12} = 1811$ pch $V_{12} = 1811$ pch (Equation 25-4 or 25-5) $Is V_3 or V_{a34} > 2,700$ pch? <input type="checkbox"/> Yes <input type="checkbox"/> No $Is V_3 or V_{a34} > 1.5 * V_{12}$ <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, $V_{12a} = 1811$ pch (Equation 25-8)			
Capacity Checks			
Actual	Capacity	LOS F?	Capacity
			Exhibit 25-14
$V_{FO} = V_F$	1932		4700
V_R	1932		4700
V_R	0		2000
Flow Entering Merge Influence Area			
Actual	Max Desirable	Violation?	Actual
V_{R12}	Exhibit 25-7		Exhibit 25-14
Level of Service Determination (if not F)			
$D_R = 5.475 + 0.00734 V_R - 0.0078$			
$D_R = 5.475 + 0.00734 * 1932 - 0.0078 = 16.4$ (pc/ml/min)			
$LOS = B$ (Exhibit 25-4)			
Speed Determination			
$M_S = (Exhibit 25-19)$			
$S_R = 55.2$ mph (Exhibit 25-19)			
$S_U = N/A$ mph (Exhibit 25-19)			
$S_D = 55.2$ mph (Exhibit 25-15)			

General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	W/T Trans	Junction	Harris Quarry Driveway-Offramp
Date Performed	9/4/2009	Jurisdiction	Menardocounty/Citizens
Analysis Time Period	2:15pm-3:15pm	Analysis Year	2040
Project Description: Harris Quarry - Project Conditions			
Inputs		Terrain: Rolling	
Upstream Adj Ramp	<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off $L_{down} = ft$ $V_u = veh/h$	Downstream Adj Ramp	<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off $L_{down} = ft$ $V_d = veh/h$
Conversion to pch/h Under Base Conditions		Sketch (show lanes, L_{LD} , $V_{R/V}$)	
(pch)	V (veh/h)	PHF	Terrain
Freeway	1909	0.95	Rolling
Ramp	32	0.95	Rolling
DownStream			
Merge Areas			
Estimation of V_{12}			
$V_{12} = V_R + (V_F - V_R) P_{FM}$ (Equation 25-2)			
$V_{12} = V_R + (V_F - V_R) P_{FD}$ (Equation 25-9) 1000 using Equation (Exhibit 25-12)			
$V_{12} = 1811$ pch $V_{12} = 1811$ pch (Equation 25-4 or 25-5) $Is V_3 or V_{a34} > 2,700$ pch? <input type="checkbox"/> Yes <input type="checkbox"/> No $Is V_3 or V_{a34} > 1.5 * V_{12}$ <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, $V_{12a} = 1811$ pch (Equation 25-8)			
Capacity Checks			
Actual	Capacity	LOS F?	Capacity
			Exhibit 25-14
$V_{FO} = V_F$	1811		4700
V_R	1777		4700
V_R	34		2000
Flow Entering Merge Influence Area			
Actual	Max Desirable	Violation?	Actual
V_{R12}	Exhibit 25-7		Exhibit 25-14
Level of Service Determination (if not F)			
$D_R = 5.475 + 0.00734 V_R - 0.0078$			
$D_R = 5.475 + 0.00734 * 1811 - 0.0078 = 15.3$ (pc/ml/min)			
$LOS = B$ (Exhibit 25-4)			
Speed Determination			
$M_S = (Exhibit 25-19)$			
$S_R = 55.1$ mph (Exhibit 25-19)			
$S_U = N/A$ mph (Exhibit 25-19)			
$S_D = 55.1$ mph (Exhibit 25-15)			

General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	W+Trans	Junction	Harris Quarry Driveway-Offramp
Date Performed	9/4/2009	Jurisdiction	Menominee County/Citizens
Analysis Time Period	October Peak 11am-12pm	Analysis Year	2040
Project Description: Harris Quarry - Project Conditions			
Inputs		Terrain: Rolling	
Upstream Adj Ramp	<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off	Downstream Adj Ramp	<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off
V_{up} = ft	$S_{FF} = 65.0$ mph	$S_{FR} = 35.0$ mph	$L_{down} = ft$
V_u = veh/h	Sketch (show lanes, L_{LD} , V_{LD})		
Conversion to pch/h Under Base Conditions			
(pch)	PHF	Terrain	%Truck
Freeway	0.95	Rolling	8
Ramp	0.95	Rolling	0
UpStream			
DownStream			
Merge Areas			
Estimation of V_{12}			
$V_{12} = V_u + (P_{FM})$	(Equation 25-2)		
$V_{12} = 55 + (0.95 \times 8)$	using Equation (Exhibit 25-5)		
$V_{12} = 58.8$	pch		
V_3 or V_{a34}	pch (Equation 25-4 or 25-5)		
V_3 or $V_{a34} = 0$	0 pch (Equation 25-4 or 25-5)		
V_3 or $V_{a34} > 2,700$ pch?	<input type="checkbox"/> Yes <input type="checkbox"/> No		
V_3 or $V_{a34} > 1.5 \cdot V_{12}$?	<input type="checkbox"/> Yes <input type="checkbox"/> No		
V_{12a}	pch (Equation 25-8)		
$V_{12a} = 58.8$	pch		
Capacity Checks			
Capacity	LOS F?	Capacity	LOS F?
Actual	Exhibit 25-7	Actual	Exhibit 25-7
$V_{FO} = V_F$	Exhibit 25-7	$V_{FO} = V_F$	Exhibit 25-7
V_R	59	V_R	59
Flow Entering Merge Influence Area			
Actual	Max Desirable	Violation?	Violation?
V_{R12}	Exhibit 25-7	V_{12}	Exhibit 25-7
$V_{R12} = 58.8$	59	$V_{12} = 58.8$	59
Level of Service Determination (if not F)			
$D_R = 5.475 + 0.00734 \cdot V_R + 0.0078$	$D_R = 5.475 + 0.00734 \cdot 59 + 0.0078$		
$D_R = 5.475$	$D_R = 5.475$		
$D_R = 5.475$	$D_R = 5.475$		
$D_R = 5.475$	$D_R = 5.475$		
Speed Determination			
$M_S = (Exhibit 25-19)$	$M_S = 55.0$ mph (Exhibit 25-19)		
$S_R = mph$ (Exhibit 25-19)	$S_R = 55.0$ mph (Exhibit 25-19)		
$S_U = mph$ (Exhibit 25-19)	$S_U = N/A$ mph (Exhibit 25-19)		
$S_L = mph$ (Exhibit 25-14)	$S_L = 55.0$ mph (Exhibit 25-14)		

General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	W+Trans	Junction	Harris Quarry Driveway-Offramp
Date Performed	9/4/2009	Jurisdiction	Menardocounty/Citizens
Analysis Time Period	2:15pm	Analysis Year	2040
Project Description: Harris Quarry - Project Conditions			
Inputs			
Upstream Adj Ramp	Terrain: Rolling	Downstream Adj Ramp	
<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off		<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off	
$S_{FF} = 65.0$ mph		$S_{FR} = 35.0$ mph	
$S_u =$ veh/h		$S_d =$ veh/h	
Conversion to pch Under Base Conditions			
(pch)	PHF	Terrain	%Truck
Freeway	1238	0.95	Rolling
Ramp	0	0.95	Rolling
DownStream			
Merge Areas			
Estimation of V_{12}			
$V_{12} = V_{12} (P_{FM})$	(Equation 25-2)	$V_{12} = V_R + (V_F - V_R) P_{FD}$	(Equation 25-8 or 25-9)
$V_{12} = 1483$		$V_{12} = 1483$	
Capacity Checks			
Actual	Capacity	LOS F?	Capacity
$V_{FO} = 1483$	1483	Exhibit 25-14	4700
$V_R = 1483$	1483	Exhibit 25-14	4700
$V_{12} = 1483$	1483	Exhibit 25-14	4700
Flow Entering Merge Influence Area			
Actual	Max Desirable	Violation?	Actual
$V_{12} = 1483$	1483	Exhibit 25-14	4700
Level of Service Determination (if not F)			
$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$			
$D_R = 11.7$ (pc/ml/min)			
$LOS = B$ (Exhibit 25-4)			
Speed Determination			
$M_S =$ (Exhibit 25-19)			
$S_R =$ mph (Exhibit 25-19)			
$S_u =$ mph (Exhibit 25-19)			
$S_d =$ mph (Exhibit 25-19)			

General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	W+Trans	Junction	Harris Quarry Driveway-Offramp
Date Performed	11/24/2009	Jurisdiction	Menardocounty/Citizens
Analysis Time Period	2:15pm	Analysis Year	2040
Project Description: Harris Quarry - Project Conditions			
Inputs			
Upstream Adj Ramp	Terrain: Rolling	Downstream Adj Ramp	
<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off		<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off	
$S_{FF} = 65.0$ mph		$S_{FR} = 35.0$ mph	
$S_u =$ veh/h		$S_d =$ veh/h	
Conversion to pch Under Base Conditions			
(pch)	PHF	Terrain	%Truck
Freeway	1159	0.95	Rolling
Ramp	44	0.95	Rolling
DownStream			
Merge Areas			
Estimation of V_{12}			
$V_{12} = V_{12} (P_{FM})$	(Equation 25-2)	$V_{12} = V_R + (V_F - V_R) P_{FD}$	(Equation 25-8 or 25-9)
$V_{12} = 1391$		$V_{12} = 1391$	
Capacity Checks			
Actual	Capacity	LOS F?	Capacity
$V_{FO} = 1391$	1391	Exhibit 25-14	4700
$V_R = 1391$	1345	Exhibit 25-14	4700
$V_{12} = 1391$	46	Exhibit 25-3	2000
Flow Entering Merge Influence Area			
Actual	Max Desirable	Violation?	Actual
$V_{12} = 1391$	1391	Exhibit 25-14	4400/Al
Level of Service Determination (if not F)			
$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$			
$D_R = 11.7$ (pc/ml/min)			
$LOS = B$ (Exhibit 25-4)			
Speed Determination			
$M_S =$ (Exhibit 25-19)			
$S_R =$ mph (Exhibit 25-19)			
$S_u =$ mph (Exhibit 25-19)			
$S_d =$ mph (Exhibit 25-19)			

RAMPS AND RAMP JUNCTIONS WORKSHEET																																																						
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Date Performed	11/24/09	Analysis Time Period	July Peak 11am-12noon	Analysis Year	2010																																																	
Inputs Terrain: Rolling Upstream Adj Ramp: <input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off Downstream Adj Ramp: <input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off L _{up} = 1000 ft V _u = 0 veh/h S _{FR} = 35.0 mph S _{FF} = 65.0 mph Sketch (show lanes, L _A , L _D , V _R) V _D = veh/h V = V/PHF x f _{HV} f _{HV} = f _p x f _p																																																						
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General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	WTrans	Junction	Harris Quarry Driveway-Chramp
Date Performed	9/4/2009	Jurisdiction	Menard County/Cattans
Analysis Time Period	4.45pm-5.45pm	Analysis Year	2010
Project Description: Harris Quarry - Project Conditions			
Inputs			
Upstream Adj Ramp	Terrain: Rolling	Downstream Adj Ramp	
<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off		<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off	
$S_{FF} = 65.0$ mph	$S_{FR} = 35.0$ mph	$S_{FR} = 35.0$ mph	
$V_{up} = 1000$ ft		$S_{FR} = 35.0$ mph	
$V_u = 0$ veh/h		$S_{FR} = 35.0$ mph	
Conversion to pc/h Under Base Conditions			
(pc/h)	PHF	Terrain	%Truck
Freeway	0.95	Rolling	8
Ramp	0.95	Rolling	0
UpStream	0	Level	0
DownStream	0	Level	0
Estimation of V_{12}			
$V_{12} = V_R + (V_F - V_R)^{0.75} \cdot P_{FD}^{0.25}$ (Equation 25-8 or 25-9)			
$V_{12} = 1241$ pc/h			
Capacity Checks			
Actual	Capacity	LOS F?	Capacity
1247	1247	No	1247
Flow Entering Merge Influence Area			
Actual	Max Desirable	Violation?	Actual
1247	4600-All	No	1247
Level of Service Determination (if not F)			
$D_R = 5.475 + 0.00734 \cdot V_{12} + 0.0078 \cdot V_{12}^{-1}$			
$D_R = 12.1$ (pc/mi/h)			
LOS = B (Exhibit 25-4)			
Speed Determination			
$M_S = 0.300$ (Exhibit 25-19)			
$S_R = 58.1$ mph (Exhibit 25-19)			
$S_U = N/A$ mph (Exhibit 25-19)			
$S_D = 58.1$ mph (Exhibit 25-15)			

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General Information		Site Information																																																			
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101																																																		
Agency or Company	WTrans	Junction	Harris Quarry Driveway-Chramp																																																		
Date Performed	11/24/09	Jurisdiction	Menard County/Citizens																																																		
Analysis Time Period	October Peak 9am-10am	Analysis Year	2010																																																		
Project Description: Harris Quarry - Project Conditions																																																					
Terrain: Rolling																																																					
Upstream Adj Ramp	<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off $S_{FF} = 65.0$ mph $S_{FR} = 35.0$ mph $S_{up} = 1000$ ft $V_u = 0$ veh/h	Downstream Adj Ramp	<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off $S_{down} =$ ft $V_D =$ veh/h																																																		
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RAMPS AND RAMP JUNCTIONS WORKSHEET										
General Information					Site Information					
Analyst: TDH Agency or Company: W+Tans Date Performed: 9/4/2009 Analysis Time Period: 5:45pm-6:15pm Project Description: Harris County - Project Conditions					Freeway/Dir of Travel: Northbound US 101 Junction: Harris County Driveway - Orange Jurisdiction: Mendocino County/Caltans Analysis Year: 2010					
Inputs Terrain: Rolling Upstream Adj Ramp: <input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off Downstream Adj Ramp: <input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off $S_{FF} = 65.0$ mph $S_{FR} = 35.0$ mph $S_{up} = 1000$ ft $S_{down} =$ ft $V_u = 0$ veh/h $V_d =$ veh/h										
Conversion to pc/h Under Base Conditions										
(pc/h)	V (veh/h)	PHF	Terrain	%Truck	%Rv	f_p	f_p	f_p	f_p	$V = V/PHE \times f_p$
Freeway	784	0.95	Rolling	8	2	0.877	1.00	1.00	983	
Ramp	6	0.95	Rolling	0	0	1.000	1.00	1.00	6	
UpStream	0	0.90	Level	0	0	1.000	1.00	1.00	0	
DownStream										
Merge Areas										
Estimation of V_{12} $V_{12} = V_F + (V_F - V_R) \cdot P_{FD}$ (Equation 25-8 or 25-9) $V_{12} = 983$ pc/h										
Capacity Checks $V_{FO} = 989$ pc/h $V_{12} = 983$ pc/h $V_{12} < V_{FO}$										
Flow Entering Merge Influence Area Actual: 989 Max Desirable: 4600-All Violation? No										
Flow Entering Diverge Influence Area Actual: 989 Max Desirable: 4600-All Violation? No										
Level of Service Determination (if not F) $D_R = 5.475 + 0.00734 \cdot V_{12} - 0.00827 \cdot A$ $D_R = 9.8$ (pc/mi/h) $LOS = A$ (Exhibit 25-4)										
Speed Determination $M_S = 0.296$ (Exhibit 25-19) $S_R = 58.2$ mph (Exhibit 25-19) $S_U = N/A$ mph (Exhibit 25-19) $S_D = 58.2$ mph (Exhibit 25-15)										

RAMPS AND RAMP JUNCTIONS WORKSHEET										
General Information					Site Information					
Analyst: TDH Agency or Company: W+Tans Date Performed: 9/4/2009 Analysis Time Period: 2:15pm-2:45pm Project Description: Harris County - Project Conditions					Freeway/Dir of Travel: Northbound US 101 Junction: Harris County Driveway - Orange Jurisdiction: Mendocino County/Caltans Analysis Year: 2010					
Inputs Terrain: Rolling Upstream Adj Ramp: <input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off Downstream Adj Ramp: <input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off $S_{FF} = 65.0$ mph $S_{FR} = 35.0$ mph $S_{up} = 1000$ ft $S_{down} =$ ft $V_u = 0$ veh/h $V_d =$ veh/h										
Conversion to pc/h Under Base Conditions										
(pc/h)	V (veh/h)	PHF	Terrain	%Truck	%Rv	f_p	f_p	f_p	f_p	$V = V/PHE \times f_p$
Freeway	744	0.95	Rolling	8	2	0.877	1.00	1.00	893	
Ramp	32	0.95	Rolling	0	0	1.000	1.00	1.00	34	
UpStream	0	0.90	Level	0	0	1.000	1.00	1.00	0	
DownStream										
Merge Areas										
Estimation of V_{12} $V_{12} = V_F + (V_F - V_R) \cdot P_{FD}$ (Equation 25-8 or 25-9) $V_{12} = 893$ pc/h										
Capacity Checks $V_{FO} = 927$ pc/h $V_{12} = 893$ pc/h $V_{12} < V_{FO}$										
Flow Entering Merge Influence Area Actual: 927 Max Desirable: 4600-All Violation? No										
Flow Entering Diverge Influence Area Actual: 927 Max Desirable: 4600-All Violation? No										
Level of Service Determination (if not F) $D_R = 5.475 + 0.00734 \cdot V_{12} - 0.00827 \cdot A$ $D_R = 9.6$ (pc/mi/h) $LOS = A$ (Exhibit 25-4)										
Speed Determination $M_S = 0.296$ (Exhibit 25-19) $S_R = 58.2$ mph (Exhibit 25-19) $S_U = N/A$ mph (Exhibit 25-19) $S_D = 58.2$ mph (Exhibit 25-15)										

RAMPS AND RAMP JUNCTIONS WORKSHEET										
General Information					Site Information					
Analyst: TDH Agency or Company: W+Tans Date Performed: 9/4/2009 Analysis Time Period: 5:45pm-6:15pm Project Description: Harris County - Project Conditions					Freeway/Dir of Travel: Northbound US 101 Junction: Harris County Driveway - Orange Jurisdiction: Mendocino County/Caltans Analysis Year: 2010					
Inputs Terrain: Rolling Upstream Adj Ramp: <input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off Downstream Adj Ramp: <input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off $S_{FF} = 65.0$ mph $S_{FR} = 35.0$ mph $S_{up} = 1000$ ft $S_{down} =$ ft $V_u = 0$ veh/h $V_d =$ veh/h										
Conversion to pc/h Under Base Conditions										
(pc/h)	V (veh/h)	PHF	Terrain	%Truck	%Rv	f_p	f_p	f_p	f_p	$V = V/PHE \times f_p$
Freeway	784	0.95	Rolling	8	2	0.877	1.00	1.00	983	
Ramp	6	0.95	Rolling	0	0	1.000	1.00	1.00	6	
UpStream	0	0.90	Level	0	0	1.000	1.00	1.00	0	
DownStream										
Merge Areas										
Estimation of V_{12} $V_{12} = V_F + (V_F - V_R) \cdot P_{FD}$ (Equation 25-8 or 25-9) $V_{12} = 983$ pc/h										
Capacity Checks $V_{FO} = 989$ pc/h $V_{12} = 983$ pc/h $V_{12} < V_{FO}$										
Flow Entering Merge Influence Area Actual: 989 Max Desirable: 4600-All Violation? No										
Flow Entering Diverge Influence Area Actual: 989 Max Desirable: 4600-All Violation? No										
Level of Service Determination (if not F) $D_R = 5.475 + 0.00734 \cdot V_{12} - 0.00827 \cdot A$ $D_R = 9.8$ (pc/mi/h) $LOS = A$ (Exhibit 25-4)										
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General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	WTrans	Junction	Harris Quarry Driveway-Chramp
Date Performed	9/4/2009	Jurisdiction	Blendocato County/Citizens
Analysis Time Period	July Peak 11am-12noon	Analysis Year	2014 without Mills Bypass
Project Description: Harris Quarry - Project Conditions			

Inputs	
Upstream Adj Ramp	Terrain: Rolling
Downstream Adj Ramp	
$V_{up} = 1000$ ft	
$V_u = 0$ veh/h	
$S_{FF} = 65.0$ mph	$S_{FR} = 35.0$ mph
Sketch (show lanes, L_A , L_D , V_R)	

Conversion to pc/h Under Base Conditions				
(pc/h)	V (veh/h)	PHF	Terrain	%Truck
Freeway	881	0.95	Rolling	8
Ramp	32	0.95	Rolling	0
UpStream	0	0.90	Level	0
DownStream	0	0.90	Level	0

Merge Areas					Diverge Areas				
Estimation of V_{12}					Estimation of V_{12}				
$V_{12} = V_F + (V_F - V_R) P_{FD}$ (Equation 25-2)					$V_{12} = V_R + (V_F - V_R) P_{FD}$ (Equation 25-8 or 25-9)				
1,000 using Equation (Exhibit 25-5)					using Equation (Exhibit 25-12)				
$V_{12} = 685$ pc/h					$V_{12} = 1057$ pc/h				

Capacity Checks		LOS F?	
Actual	Capacity	Actual	Capacity
719	Exhibit 25-7	No	Exhibit 25-14
$V_{FO} = 719$	Exhibit 25-7	No	Exhibit 25-14
V_R	Exhibit 25-3		Exhibit 25-3

Flow Entering Merge Influence Area		Flow Entering Diverge Influence Area	
Actual	Max Desirable	Actual	Max Desirable
719	4600/All		4600/All
V_{R12}	Exhibit 25-7	V_{12}	Exhibit 25-14

Level of Service Determination (if not F)	
$D_R = 5.475 + 0.00734 V_{R12} + 0.0078 V_{12}$	
$D_R = 7.9$ (pc/mi/h)	
$LOS = A$ (Exhibit 25-4)	

Speed Determination	
$M_S = 0.294$ (Exhibit 25-19)	
$S_R = 58.2$ mph (Exhibit 25-19)	
$S_U = N/A$ mph (Exhibit 25-19)	
$S_D = 58.2$ mph (Exhibit 25-14)	

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General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	WTrans	Junction	Harris Quarry Driveway-Chramp
Date Performed	9/4/2009	Jurisdiction	Blendocato County/Citizens
Analysis Time Period	July Peak 11am-12noon	Analysis Year	2014 without Mills Bypass
Project Description: Harris Quarry - Project Conditions			

Inputs	
Upstream Adj Ramp	Terrain: Rolling
Downstream Adj Ramp	
$V_{up} = 1000$ ft	
$V_u = 0$ veh/h	
$S_{FF} = 65.0$ mph	$S_{FR} = 35.0$ mph
Sketch (show lanes, L_A , L_D , V_R)	

Conversion to pc/h Under Base Conditions				
(pc/h)	V (veh/h)	PHF	Terrain	%Truck
Freeway	571	0.95	Rolling	8
Ramp	32	0.95	Rolling	0
UpStream	0	0.90	Level	0
DownStream	0	0.90	Level	0

Merge Areas					Diverge Areas				
Estimation of V_{12}					Estimation of V_{12}				
$V_{12} = V_F + (V_F - V_R) P_{FD}$ (Equation 25-2)					$V_{12} = V_R + (V_F - V_R) P_{FD}$ (Equation 25-8 or 25-9)				
1,000 using Equation (Exhibit 25-5)					using Equation (Exhibit 25-12)				
$V_{12} = 685$ pc/h					$V_{12} = 1057$ pc/h				

Capacity Checks		LOS F?	
Actual	Capacity	Actual	Capacity
719	Exhibit 25-7	No	Exhibit 25-14
$V_{FO} = 719$	Exhibit 25-7	No	Exhibit 25-14
V_R	Exhibit 25-3		Exhibit 25-3

Flow Entering Merge Influence Area		Flow Entering Diverge Influence Area	
Actual	Max Desirable	Actual	Max Desirable
719	4600/All		4600/All
V_{R12}	Exhibit 25-7	V_{12}	Exhibit 25-14

Level of Service Determination (if not F)	
$D_R = 5.475 + 0.00734 V_{R12} + 0.0078 V_{12}$	
$D_R = 7.9$ (pc/mi/h)	
$LOS = A$ (Exhibit 25-4)	

Speed Determination	
$M_S = 0.294$ (Exhibit 25-19)	
$S_R = 58.2$ mph (Exhibit 25-19)	
$S_U = N/A$ mph (Exhibit 25-19)	
$S_D = 58.2$ mph (Exhibit 25-14)	

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General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	WTrans	Junction	Harris Quarry Driveway-Chramp
Date Performed	9/4/2009	Jurisdiction	Blendocato County/Collins
Analysis Time Period	4.45pm-5.45pm	Analysis Year	2014 without Willits Bypass
Project Description: Harris Quarry - Project Conditions			
Inputs			
Upstream Adj Ramp	Terrain: Rolling		Downstream Adj Ramp
<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off			<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off
$L_{up} = 1000$ ft			$L_{down} =$ ft
$V_u = 0$ veh/h			$V_d =$ veh/h
Conversion to pc/h Under Base Conditions			
(pc/h)	V (veh/h)	PHF	Terrain
		%Truck	%Rv
Freeway	1111	0.95	Rolling
Ramp	6	0.95	Rolling
UpStream	0	0.90	Level
DownStream	0	0	1.000
Merge Areas			
Estimation of V_{12}			
$V_{12} = V_R + (V_F - V_R)^{FD}$ (Equation 25-8 or 25-9)			
1,000 using Equation (Exhibit 25-5)			
$V_{12} = 1333$ pc/h			
Capacity Checks			
Actual Capacity LOS F? Actual Exhibit 25-14			
LOS F? Actual Exhibit 25-14			
Flow Entering Merge Influence Area			
Actual Max Desirable Violation? Actual Max Desirable Violation?			
Exhibit 25-7 Exhibit 25-14			
Level of Service Determination (if not F)			
$D_R = 5.475 + 0.00734 \cdot V_R + 0.0078 \cdot V_{12}$			
$D_R = 4.252 + 0.00866 \cdot V_{12} - 0.009 \cdot L_D$			
$D_R = 2.3$ (pc/mi/h)			
LOS = B (Exhibit 25-4)			
Speed Determination			
$M_S = 0.301$ (Exhibit 25-19)			
$S_R = 58.1$ mph (Exhibit 25-19)			
$S_U = N/A$ mph (Exhibit 25-19)			
$S_D = 58.1$ mph (Exhibit 25-15)			

General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	WTrans	Junction	Harris Quarry Driveway-Chramp
Date Performed	11/24/2009	Jurisdiction	Blendocato County/Collins
Analysis Time Period	2:15pm-2:55pm	Analysis Year	2014 without Willits Bypass
Project Description: Harris Quarry - Project Conditions			
Inputs			
Upstream Adj Ramp	Terrain: Rolling		Downstream Adj Ramp
<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off			<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off
$L_{up} = 1000$ ft			$L_{down} =$ ft
$V_u = 0$ veh/h			$V_d =$ veh/h
Conversion to pc/h Under Base Conditions			
(pc/h)	V (veh/h)	PHF	Terrain
		%Truck	%Rv
Freeway	1041	0.95	Rolling
Ramp	28	0.95	Rolling
UpStream	0	0.90	Level
DownStream	0	0	1.000
Merge Areas			
Estimation of V_{12}			
$V_{12} = V_R + (V_F - V_R)^{FD}$ (Equation 25-8 or 25-9)			
1,000 using Equation (Exhibit 25-5)			
$V_{12} = 1249$ pc/h			
Capacity Checks			
Actual Capacity LOS F? Actual Exhibit 25-14			
LOS F? Actual Exhibit 25-14			
Flow Entering Merge Influence Area			
Actual Max Desirable Violation? Actual Max Desirable Violation?			
Exhibit 25-7 Exhibit 25-14			
Level of Service Determination (if not F)			
$D_R = 5.475 + 0.00734 \cdot V_R + 0.0078 \cdot V_{12}$			
$D_R = 4.252 + 0.00866 \cdot V_{12} - 0.009 \cdot L_D$			
$D_R = 2.3$ (pc/mi/h)			
LOS = B (Exhibit 25-4)			
Speed Determination			
$M_S = 0.300$ (Exhibit 25-19)			
$S_R = 58.1$ mph (Exhibit 25-19)			
$S_U = N/A$ mph (Exhibit 25-19)			
$S_D = 58.1$ mph (Exhibit 25-15)			

General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	WTrans	Junction	Harris Quarry Driveway-Chramp
Date Performed	11/24/2009	Jurisdiction	Blendocato County/Citizens
Analysis Time Period	October Peak 9am-10am	Analysis Year	2014 without Mills Bypass
Project Description: Harris Quarry-Project Conditions			
Terrain: Rolling			
Upstream Adj Ramp	<input type="checkbox"/> Yes <input checked="" type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off	Downstream Adj Ramp	<input type="checkbox"/> Yes <input checked="" type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off
L_{up} = 1000 ft		S_{FR} = 35.0 mph	S_{down} = ft
V_u = 0 veh/h		Sketch (show lanes, L_{up} , L_{down} , V_{FR})	V_D = veh/h
Conversion to pc/h Under Base Conditions			
(pc/h)	PHF	Terrain	%Truck
Freeway	0.95	Rolling	8
Ramp	0.95	Rolling	0
UpStream	0	Level	0
DownStream	0	Level	0
Merge Areas			
Estimation of V_{12}			
$V_{12} = V_F + (V_F - V_R) P_{FD}$ (Equation 25-2)			
L_{EQ}	1,000 using Equation (Exhibit 25-3)		
P_{FD}	0.95		
V_{12}	572 pc/h		
V_3 or V_{a04}	0		
V_3 or $V_{a04} > 2,700$ pc/h?	<input type="checkbox"/> Yes <input checked="" type="checkbox"/> No		
V_3 or $V_{a04} > 1.5 \cdot V_{12}$?	<input type="checkbox"/> Yes <input checked="" type="checkbox"/> No		
V_3 or $V_{a04} > 1.5 \cdot V_{12}$?	<input type="checkbox"/> Yes <input checked="" type="checkbox"/> No		
V_{12a}	572 pc/h (Equation 25-3)		
Capacity Checks			
Actual	Capacity	LOS F?	Capacity
572	Exhibit 25-7	No	Exhibit 25-14
V_{FO}	572		Exhibit 25-14
Flow Entering Merge Influence Area			
Actual	Max Desirable	Violation?	Actual
572	4600/AI	No	Exhibit 25-14
Level of Service Determination (if not F)			
$D_R = 5.475 + 0.00734 \cdot V_{12} - 0.00827 \cdot L_A$			
$D_R = 6.8$ (pc/mi/h)			
LOS = A (Exhibit 25-4)			
Speed Determination			
$M_S = 0.293$ (Exhibit 25-19)			
$S_R = 58.3$ mph (Exhibit 25-19)			
$S_U = N/A$ mph (Exhibit 25-19)			
$S_D = 58.3$ mph (Exhibit 25-15)			

General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	WTrans	Junction	Harris Quarry Driveway-Chramp
Date Performed	11/24/2009	Jurisdiction	Blendocato County/Citizens
Analysis Time Period	October Peak 11am	Analysis Year	2014 without Mills Bypass
Project Description: Harris Quarry-Project Conditions			
Terrain: Rolling			
Upstream Adj Ramp	<input type="checkbox"/> Yes <input checked="" type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off	Downstream Adj Ramp	<input type="checkbox"/> Yes <input checked="" type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off
L_{up} = 1000 ft		S_{FR} = 35.0 mph	S_{down} = ft
V_u = 0 veh/h		Sketch (show lanes, L_{up} , L_{down} , V_{FR})	V_D = veh/h
Conversion to pc/h Under Base Conditions			
(pc/h)	PHF	Terrain	%Truck
Freeway	0.95	Rolling	8
Ramp	0.95	Rolling	0
UpStream	0	Level	0
DownStream	0	Level	0
Merge Areas			
Estimation of V_{12}			
$V_{12} = V_F + (V_F - V_R) P_{FD}$ (Equation 25-2)			
L_{EQ}	1,000 using Equation (Exhibit 25-3)		
P_{FD}	0.95		
V_{12}	811 pc/h		
V_3 or V_{a04}	0		
V_3 or $V_{a04} > 2,700$ pc/h?	<input type="checkbox"/> Yes <input checked="" type="checkbox"/> No		
V_3 or $V_{a04} > 1.5 \cdot V_{12}$?	<input type="checkbox"/> Yes <input checked="" type="checkbox"/> No		
V_3 or $V_{a04} > 1.5 \cdot V_{12}$?	<input type="checkbox"/> Yes <input checked="" type="checkbox"/> No		
V_{12a}	811 pc/h (Equation 25-3)		
Capacity Checks			
Actual	Capacity	LOS F?	Capacity
811	Exhibit 25-7	No	Exhibit 25-14
V_{FO}	811		Exhibit 25-14
Flow Entering Merge Influence Area			
Actual	Max Desirable	Violation?	Actual
811	4600/AI	No	Exhibit 25-14
Level of Service Determination (if not F)			
$D_R = 5.475 + 0.00734 \cdot V_{12} - 0.00827 \cdot L_A$			
$D_R = 4.252 + 0.00866 \cdot V_{12} - 0.009 \cdot L_D$			
LOS = A (Exhibit 25-4)			
Speed Determination			
$M_S = 0.293$ (Exhibit 25-19)			
$S_R = 58.2$ mph (Exhibit 25-19)			
$S_U = N/A$ mph (Exhibit 25-19)			
$S_D = 58.2$ mph (Exhibit 25-15)			

General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	WTrans	Junction	Harris Quarry Driveway-Chramp
Date Performed	9/4/2009	Jurisdiction	Blendocato County/Citizens
Analysis Time Period	2:15pm-5:45pm	Analysis Year	2014 without Willits Bypass
Project Description: Harris Quarry - Project Conditions			
Terrain: Rolling			
Upstream Adj Ramp	Yes <input type="checkbox"/> No <input type="checkbox"/> On <input type="checkbox"/> Off <input type="checkbox"/>	Downstream Adj Ramp	Yes <input type="checkbox"/> No <input type="checkbox"/> On <input type="checkbox"/> Off <input type="checkbox"/>
$L_{up} = 1000$ ft		$S_{FR} = 35.0$ mph	$S_{down} =$ ft
$V_u = 0$ veh/h		Sketch (show lanes, L_{up} , L_{down} , V_{FR})	$V_D =$ veh/h
Conversion to pc/h Under Base Conditions			
(pc/h)	V (veh/h)	PHF	Terrain
Freeway	883	0.95	Rolling
Ramp	6	0.95	Rolling
UpStream	0	0.90	Level
DownStream	0	0	0
Estimation of V_{12}			
$V_{12} = V_F + (P_{FM})$ (Equation 25-2)			
$V_{12} = V_R + (V_F - V_R) \cdot P_{FD}$ (Equation 25-8 or 25-9)			
1,000 using Equation (Exhibit 25-5)			
$V_{12} = 1024$ pc/h			
Capacity Checks			
V_3 or V_{a04} pc/h (Equation 25-4 or 25-5)			
V_3 or $V_{a04} > 2,700$ pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No			
V_3 or $V_{a04} > 1.5 \cdot V_{12}$? <input type="checkbox"/> Yes <input type="checkbox"/> No			
V_3 or $V_{a04} > 1.5 \cdot V_{12}$? <input type="checkbox"/> Yes <input type="checkbox"/> No			
$V_{12a} = 1024$ pc/h (Equation 25-8)			
Capacity Checks			
Actual	Capacity	LOS F?	Capacity
$V_{FO} = 1030$	Exhibit 25-7	No	Exhibit 25-14
$V_R = 1030$	Exhibit 25-7	No	Exhibit 25-14
Flow Entering Merge Influence Area			
Actual	Max Desirable	Violation?	Actual
$V_{R12} = 1030$	4600-Alt	No	Exhibit 25-14
Level of Service Determination (if not F)			
$D_R = 5.475 + 0.00734 \cdot V_{12} - 0.0078 \cdot V_{12}$			
$D_R = 4.252 + 0.0086 \cdot V_{12} - 0.009 \cdot L_D$			
$D_R = 10.1$ (pc/mi/h)			
$LOS = B$ (Exhibit 25-4)			
Speed Determination			
$M_S = 0.297$ (Exhibit 25-19)			
$S_R = 58.2$ mph (Exhibit 25-19)			
$S_U = N/A$ mph (Exhibit 25-19)			
$S_D = 58.2$ mph (Exhibit 25-15)			

General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	WTrans	Junction	Harris Quarry Driveway-Chramp
Date Performed	9/4/2009	Jurisdiction	Blendocato County/Citizens
Analysis Time Period	2:15pm-5:45pm	Analysis Year	2014 without Willits Bypass
Project Description: Harris Quarry - Project Conditions			
Terrain: Rolling			
Upstream Adj Ramp	Yes <input type="checkbox"/> No <input type="checkbox"/> On <input type="checkbox"/> Off <input type="checkbox"/>	Downstream Adj Ramp	Yes <input type="checkbox"/> No <input type="checkbox"/> On <input type="checkbox"/> Off <input type="checkbox"/>
$L_{up} = 1000$ ft		$S_{FR} = 35.0$ mph	$S_{down} =$ ft
$V_u = 0$ veh/h		Sketch (show lanes, L_{up} , L_{down} , V_{FR})	$V_D =$ veh/h
Conversion to pc/h Under Base Conditions			
(pc/h)	V (veh/h)	PHF	Terrain
Freeway	799	0.95	Rolling
Ramp	32	0.95	Rolling
UpStream	0	0.90	Level
DownStream	0	0	0
Estimation of V_{12}			
$V_{12} = V_F + (P_{FM})$ (Equation 25-2)			
$V_{12} = V_R + (V_F - V_R) \cdot P_{FD}$ (Equation 25-8 or 25-9)			
1,000 using Equation (Exhibit 25-5)			
$V_{12} = 993$ pc/h			
Capacity Checks			
V_3 or V_{a04} pc/h (Equation 25-4 or 25-5)			
V_3 or $V_{a04} > 2,700$ pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No			
V_3 or $V_{a04} > 1.5 \cdot V_{12}$? <input type="checkbox"/> Yes <input type="checkbox"/> No			
V_3 or $V_{a04} > 1.5 \cdot V_{12}$? <input type="checkbox"/> Yes <input type="checkbox"/> No			
$V_{12a} = 993$ pc/h (Equation 25-8)			
Capacity Checks			
Actual	Capacity	LOS F?	Capacity
$V_{FO} = 993$	Exhibit 25-7	No	Exhibit 25-14
$V_R = 993$	Exhibit 25-7	No	Exhibit 25-14
Flow Entering Merge Influence Area			
Actual	Max Desirable	Violation?	Actual
$V_{R12} = 993$	4600-Alt	No	Exhibit 25-14
Level of Service Determination (if not F)			
$D_R = 5.475 + 0.00734 \cdot V_{12} - 0.0078 \cdot V_{12}$			
$D_R = 4.252 + 0.0086 \cdot V_{12} - 0.009 \cdot L_D$			
$D_R = 10.1$ (pc/mi/h)			
$LOS = B$ (Exhibit 25-4)			
Speed Determination			
$M_S = 0.297$ (Exhibit 25-19)			
$S_R = 58.2$ mph (Exhibit 25-19)			
$S_U = N/A$ mph (Exhibit 25-19)			
$S_D = 58.2$ mph (Exhibit 25-15)			

General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	WTrans	Junction	Harris Quarry Driveway-Chramp
Date Performed	9/4/2009	Jurisdiction	Menardoto County/Citizens
Analysis Time Period	July Peak 11am-12noon	Analysis Year	2014 with Williams Bypass
Project Description: Harris Quarry - Project Conditions			
Inputs			
Upstream Adj Ramp	Terrain: Rolling	Downstream Adj Ramp	
<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off		<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off	
$L_{up} = 1000$ ft		$L_{down} =$ ft	
$V_u = 0$ veh/h		$V_d =$ veh/h	
Conversion to pc/h Under Base Conditions			
(pc/h)	V (veh/h)	PHF	Terrain
Freeway	571	0.95	Rolling
Ramp	48	0.95	Rolling
UpStream	0	0.90	Level
DownStream	0	0.90	Level
Merge Areas			
Estimation of V_{12}			
$V_{12} = V_R + (V_F - V_R) \cdot F_D$ (Equation 25-2)			
EQ = 1,000 using Equation (Exhibit 25-5)			
$V_{12} = 685$ pc/h			
Capacity Checks			
Actual Capacity LOS F? Exhibit 25-14			
LOS = A (Exhibit 25-4)			
Flow Entering Merge Influence Area			
Actual Max Desirable Violation? Exhibit 25-7 Exhibit 25-14			
Level of Service Determination (if not F) $D_R = 5.475 + 0.00734 \cdot V_R + 0.0078 \cdot V_{12} = 5.475 + 0.00734 \cdot 0 + 0.0078 \cdot 685 = 5.547$			
Level of Service Determination (if not F) $D_R = 4.252 + 0.00866 \cdot V_{12} - 0.009 \cdot L_D = 4.252 + 0.00866 \cdot 685 - 0.009 \cdot 1000 = 4.252 + 5.921 - 9.0 = 1.173$			
Speed Determination			
$M_S = 0.294$ (Exhibit 25-19)			
$S_R = 58.2$ mph (Exhibit 25-19)			
$S_U = N/A$ mph (Exhibit 25-19)			
$S_D = 58.2$ mph (Exhibit 25-14)			

General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	WTrans	Junction	Harris Quarry Driveway-Chramp
Date Performed	9/4/2009	Jurisdiction	Menardoto County/Citizens
Analysis Time Period	July Peak 9am-10am	Analysis Year	2014 with Williams Bypass
Project Description: Harris Quarry - Project Conditions			
Inputs			
Upstream Adj Ramp	Terrain: Rolling	Downstream Adj Ramp	
<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off		<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off	
$L_{up} = 1000$ ft		$L_{down} =$ ft	
$V_u = 0$ veh/h		$V_d =$ veh/h	
Conversion to pc/h Under Base Conditions			
(pc/h)	V (veh/h)	PHF	Terrain
Freeway	571	0.95	Rolling
Ramp	48	0.95	Rolling
UpStream	0	0.90	Level
DownStream	0	0.90	Level
Merge Areas			
Estimation of V_{12}			
$V_{12} = V_R + (V_F - V_R) \cdot F_D$ (Equation 25-2)			
EQ = 1,000 using Equation (Exhibit 25-5)			
$V_{12} = 738$ pc/h			
Capacity Checks			
Actual Capacity LOS F? Exhibit 25-14			
LOS = A (Exhibit 25-4)			
Flow Entering Merge Influence Area			
Actual Max Desirable Violation? Exhibit 25-7 Exhibit 25-14			
Level of Service Determination (if not F) $D_R = 5.475 + 0.00734 \cdot V_R + 0.0078 \cdot V_{12} = 5.475 + 0.00734 \cdot 0 + 0.0078 \cdot 738 = 5.575$			
Level of Service Determination (if not F) $D_R = 4.252 + 0.00866 \cdot V_{12} - 0.009 \cdot L_D = 4.252 + 0.00866 \cdot 738 - 0.009 \cdot 1000 = 4.252 + 6.371 - 9.0 = 1.623$			
Speed Determination			
$M_S = 0.294$ (Exhibit 25-19)			
$S_R = 58.2$ mph (Exhibit 25-19)			
$S_U = N/A$ mph (Exhibit 25-19)			
$S_D = 58.2$ mph (Exhibit 25-14)			

General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	W-Trans	Junction	Harris Quarry Driveway-Chramp
Date Performed	9/4/2009	Jurisdiction	Menardocounty/Cattans
Analysis Time Period	July Peak 4:45pm-5:45pm	Analysis Year	2014 with Willis Bypass
Project Description: Harris Quarry - Project Conditions			
Inputs			
Upstream Adj Ramp	Terrain: Rolling	Downstream Adj Ramp	
<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off		<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off	
$L_{up} = 1000$ ft		$L_{down} =$ ft	
$V_u = 0$ veh/h		$V_d =$ veh/h	
Conversion to pc/h Under Base Conditions			
(pc/h)	V (veh/h)	PHF	Terrain
		%Truck	%Rv
Freeway	1111	0.95	Rolling
Ramp	6	0.95	Rolling
UpStream	0	0.90	Level
DownStream	0	0	0
Merge Areas			
Estimation of V_{12}			
$V_{12} = V_R + (V_F - V_R)^{0.75} \cdot V_F^{0.25}$ (Equation 25-2)			
= 1000 using Equation (Exhibit 25-5)			
$V_{12} = 1333$ pc/h			
Capacity Checks			
Actual	Capacity	LOS F?	Capacity
$V_{FO} = 1339$	Exhibit 25-7	No	Exhibit 25-14
$V_R = 0$	Exhibit 25-3		Exhibit 25-3
Flow Entering Merge Influence Area			
Actual	Max Desirable	Violation?	
$V_{R12} = 1339$	Exhibit 25-7	No	Exhibit 25-14
Level of Service Determination (if not F)			
$D_R = 5.475 + 0.00734 \cdot V_{12} - 0.0078 \cdot V_{12}^2 = 0.00627 \cdot V_{12}$			
$D_R = 12.4$ (pc/mi/h)			
LOS = B (Exhibit 25-4)			
Speed Determination			
$M_S = 0.301$ (Exhibit 25-19)			
$S_R = 58.1$ mph (Exhibit 25-19)			
$S_U = N/A$ mph (Exhibit 25-19)			
$S_D = 58.1$ mph (Exhibit 25-15)			

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General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	W-Trans	Junction	Harris Quarry Driveway-Chramp
Date Performed	11/24/2009	Jurisdiction	Menardocounty/Cattans
Analysis Time Period	2:15pm-3:15pm	Analysis Year	2014 with Willis Bypass
Project Description: Harris Quarry - Project Conditions			
Inputs			
Upstream Adj Ramp	Terrain: Rolling	Downstream Adj Ramp	
<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off		<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off	
$L_{up} = 1000$ ft		$L_{down} =$ ft	
$V_u = 0$ veh/h		$V_d =$ veh/h	
Conversion to pc/h Under Base Conditions			
(pc/h)	V (veh/h)	PHF	Terrain
		%Truck	%Rv
Freeway	1041	0.95	Rolling
Ramp	40	0.95	Rolling
UpStream	0	0.90	Level
DownStream	0	0	0
Merge Areas			
Estimation of V_{12}			
$V_{12} = V_R + (V_F - V_R)^{0.75} \cdot V_F^{0.25}$ (Equation 25-2)			
= 1249 using Equation (Exhibit 25-5)			
$V_{12} = 1249$ pc/h			
Capacity Checks			
Actual	Capacity	LOS F?	Capacity
$V_{FO} = 1291$	Exhibit 25-7	No	Exhibit 25-14
$V_R = 0$	Exhibit 25-3		Exhibit 25-3
Flow Entering Merge Influence Area			
Actual	Max Desirable	Violation?	
$V_{R12} = 1291$	Exhibit 25-7	No	Exhibit 25-14
Level of Service Determination (if not F)			
$D_R = 5.475 + 0.00734 \cdot V_{12} - 0.0078 \cdot V_{12}^2 = 0.00627 \cdot V_{12}$			
$D_R = 12.4$ (pc/mi/h)			
LOS = B (Exhibit 25-4)			
Speed Determination			
$M_S = 0.300$ (Exhibit 25-19)			
$S_R = 58.1$ mph (Exhibit 25-19)			
$S_U = N/A$ mph (Exhibit 25-19)			
$S_D = 58.1$ mph (Exhibit 25-15)			

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RAMPS AND RAMP JUNCTIONS WORKSHEET												
General Information						Site Information						
Analyst	TDH					Freeway/Dir of Travel					Northbound US 101	
Agency or Company	WTrans					Junction					Harris Quarry Driveway-Chramp	
Date Performed	9/4/2009					Jurisdiction					Menardoto County/Citizens	
Analysis Time Period	12mo					Analysis Year					2014 with Willis Bypass	
Project Description: Harris Quarry - Project Conditions												
Terrain: Rolling												
Upstream Adj Ramp	<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off					Downstream Adj Ramp	<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off					$v_{down} = ft$
$v_{up} = 1000 ft$						$v_{down} = 1000 ft$						$v_D = veh/h$
$V_u = 0 veh/h$						$V_D = 0 veh/h$						$V_{FR} = 35.0 mph$
Sketch (show lanes, L_A , L_D , V_R)												
Conversion to pc/h Under Base Conditions												
(pc/h)	V (veh/h)	PHF	Terrain	%Truck	%Rv	f_p	f_{HV}	f_p	f_p	f_p	$v = V/PHF \times f_p \times f_p$	
Freeway	676	0.95	Rolling	8	2	0.877	1.00	811				
Ramp	64	0.95	Rolling	0	0	1.000	1.00	67				
UpStream	0	0.90	Level	0	0	1.000	1.00	0				
DownStream												
Merge Areas												
Estimation of V_{12}												
$V_{12} = V_F + V_{FM}$ (Equation 25-2)												
$V_{12} = V_R + (V_F - V_R) \cdot P_{FD}$ (Equation 25-8 or 25-9) using Equation (Exhibit 25-12)												
$V_{12} = V_{12}$												
V_3 or V_{a04} (Equation 25-4 or 25-5) pc/h												
V_3 or $V_{a04} > 2,700 pc/h$? <input type="checkbox"/> Yes <input type="checkbox"/> No pc/h (Equation 25-4 or 25-5)												
$Is V_3$ or $V_{a04} > 1.5 \cdot V_{12}$? <input type="checkbox"/> Yes <input type="checkbox"/> No pc/h (Equation 25-18)												
$Is V_3$ or $V_{a04} > 1.5 \cdot V_{12}$? <input type="checkbox"/> Yes <input type="checkbox"/> No pc/h (Equation 25-18)												
$Is V_3$ or $V_{a04} > 1.5 \cdot V_{12}$? <input type="checkbox"/> Yes <input type="checkbox"/> No pc/h (Equation 25-18)												
$Is V_3$ or $V_{a04} > 1.5 \cdot V_{12}$? <input type="checkbox"/> Yes <input type="checkbox"/> No pc/h (Equation 25-18)												
Capacity Checks												
Actual Capacity LOS F? Actual Capacity LOS F?												
$V_{FO} = 593$ Exhibit 25-7												
$V_{FO} = V_F$ Exhibit 25-14												
$V_{R} = 0$ Exhibit 25-14												
Flow Entering Merge Influence Area												
Actual Max Desirable Violation? Actual Max Desirable Violation?												
$V_{R12} = 593$ Exhibit 25-7 4600-AI No V_{12} Exhibit 25-14												
Level of Service Determination (if not F)												
$D_R = 5.475 + 0.00734 \cdot V_{R12} + 0.0078 \cdot V_{12} - 0.00627 \cdot L_A$												
$D_R = 6.9$ (pc/mi/h)												
$LOS = A$ (Exhibit 25-4)												
Speed Determination												
$M_S = 0.293$ (Exhibit 25-19)												
$S_R = 58.3$ mph (Exhibit 25-19)												
$S_U = N/A$ mph (Exhibit 25-19)												
$S_D = 58.3$ mph (Exhibit 25-15)												
$S = 58.3$ mph (Exhibit 25-14)												
HCS™ Version 5.4												

RAMPS AND RAMP JUNCTIONS WORKSHEET												
General Information						Site Information						
Analyst	TDH					Freeway/Dir of Travel					Northbound US 101	
Agency or Company	WTrans					Junction					Harris Quarry Driveway-Chramp	
Date Performed	9/4/2009					Jurisdiction					Menardoto County/Citizens	
Analysis Time Period	12mo					Analysis Year					2014 with Willis Bypass	
Project Description: Harris Quarry - Project Conditions												
Terrain: Rolling												
Upstream Adj Ramp	<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off					Downstream Adj Ramp	<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off					$v_{down} = ft$
$v_{up} = 1000 ft$						$v_{down} = 1000 ft$						$v_D = veh/h$
$V_u = 0 veh/h$						$V_D = 0 veh/h$						$V_{FR} = 35.0 mph$
Sketch (show lanes, L_A , L_D , V_R)												
Conversion to pc/h Under Base Conditions												
(pc/h)	V (veh/h)	PHF	Terrain	%Truck	%Rv	f_p	f_{HV}	f_p	f_p	f_p	$v = V/PHF \times f_p \times f_p$	
Freeway	438	0.95	Rolling	8	2	0.877	1.00	526				
Ramp	64	0.95	Rolling	0	0	1.000	1.00	67				
UpStream	0	0.90	Level	0	0	1.000	1.00	0				
DownStream												
Merge Areas												
Estimation of V_{12}												
$V_{12} = V_F + V_{FM}$ (Equation 25-2)												
$V_{12} = V_R + (V_F - V_R) \cdot P_{FD}$ (Equation 25-8 or 25-9) using Equation (Exhibit 25-12)												
$V_{12} = V_{12}$												
V_3 or V_{a04} (Equation 25-4 or 25-5) pc/h												
V_3 or $V_{a04} > 2,700 pc/h$? <input type="checkbox"/> Yes <input type="checkbox"/> No pc/h (Equation 25-4 or 25-5)												
$Is V_3$ or $V_{a04} > 1.5 \cdot V_{12}$? <input type="checkbox"/> Yes <input type="checkbox"/> No pc/h (Equation 25-18)												
$Is V_3$ or $V_{a04} > 1.5 \cdot V_{12}$? <input type="checkbox"/> Yes <input type="checkbox"/> No pc/h (Equation 25-18)												
$Is V_3$ or $V_{a04} > 1.5 \cdot V_{12}$? <input type="checkbox"/> Yes <input type="checkbox"/> No pc/h (Equation 25-18)												
$Is V_3$ or $V_{a04} > 1.5 \cdot V_{12}$? <input type="checkbox"/> Yes <input type="checkbox"/> No pc/h (Equation 25-18)												
Capacity Checks												
Actual Capacity LOS F? Actual Capacity LOS F?												
$V_{FO} = 593$ Exhibit 25-7												
$V_{FO} = V_F$ Exhibit 25-14												
$V_{R} = 0$ Exhibit 25-14												
Flow Entering Merge Influence Area												
Actual Max Desirable Violation? Actual Max Desirable Violation?												
$V_{R12} = 593$ Exhibit 25-7 4600-AI No V_{12} Exhibit 25-14												
Level of Service Determination (if not F)												
$D_R = 5.475 + 0.00734 \cdot V_{R12} + 0.0078 \cdot V_{12} - 0.00627 \cdot L_A$												
$D_R = 6.9$ (pc/mi/h)												
$LOS = A$ (Exhibit 25-4)												
Speed Determination												
$M_S = 0.293$ (Exhibit 25-19)												
$S_R = 58.3$ mph (Exhibit 25-19)												
$S_U = N/A$ mph (Exhibit 25-19)												
$S_D = 58.3$ mph (Exhibit 25-15)												
$S = 58.3$ mph (Exhibit 25-14)												
HCS™ Version 5.4												

General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	WTrans	Junction	Harris Quarry Driveway-Chramp
Date Performed	9/4/2009	Jurisdiction	Menard County/Cattans
Analysis Time Period	2:50pm-4:45pm	Analysis Year	2014 with Willis Bypass
Project Description: Harris Quarry - Project Conditions			
Inputs			
Upstream Adj Ramp	Terrain: Rolling	Downstream Adj Ramp	
<input type="checkbox"/> Yes <input checked="" type="checkbox"/> No		<input type="checkbox"/> Yes <input checked="" type="checkbox"/> On <input type="checkbox"/> No <input checked="" type="checkbox"/> Off	
$S_{FF} = 65.0$ mph		$S_{FR} = 35.0$ mph	
$S_{up} = 1000$ ft		$S_{down} =$ ft	
$V_u = 0$ veh/h		$V_D =$ veh/h	
Conversion to pch Under Base Conditions			
(pch)	PHF	Terrain	%Truck
Freeway	0.95	Rolling	8
Ramp	0.95	Rolling	0
UpStream	0	Level	0
DownStream	0	Level	0
Merge Areas			
Estimation of V_{12}			
$V_{12} = V_R + (V_F - V_R)^{PF}$ (Equation 25-2)			
$V_{12} = 1000$ using Equation (Exhibit 25-5)			
Capacity Checks			
Actual	Capacity	LOS F?	Capacity
1030	1030	No	1030
Flow Entering Merge Influence Area			
Actual	Max Desirable	Violation?	
1030	4600-All	No	
Level of Service Determination (if not F)			
$D_R = 5.475 + 0.00734 \cdot V_R + 0.0078 \cdot V_{12} - 0.00627 \cdot L_A$			
$D_R = 10.4$ (pc/mi/h)			
LOS = B (Exhibit 25-4)			
Speed Determination			
$M_S = 0.297$ (Exhibit 25-19)			
$S_R = 58.2$ mph (Exhibit 25-19)			
$S_U = N/A$ mph (Exhibit 25-19)			
$S_D = 58.2$ mph (Exhibit 25-14)			

RAMPS AND RAMP JUNCTIONS WORKSHEET											
General Information					Site Information						
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101	Harris County	Driveway	Junction	Chicamp	Memoranda	County/Citizens		
Agency or Company	WTrans	Jurisdiction	Analysis Year	2030							
Date Performed	9/4/2009										
Analysis's Time Period	July Peak 11am-12noon	Analysis Year	2030								
Project Description	Harris County - Project Conditions										
Inputs											
Upstream Adj	Terrain: Rolling										
Ramp	Yes <input type="checkbox"/> No <input type="checkbox"/>	On <input type="checkbox"/> Off <input type="checkbox"/>								Downstream Adj	Yes <input type="checkbox"/> No <input type="checkbox"/>
Up	1000 ft	Down	350 mph	Sketch (show lanes, L _A , L _D , V _R)						Downstream Adj	Yes <input type="checkbox"/> No <input type="checkbox"/>
V _u	0 veh/h	V _d	0 veh/h							V _d	veh/h
Conversion to pc/h Under Base Conditions											
(pc/h)	V	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	f _p	V = V/PHF x f _{HV}		
Freeway	729	0.95	Rolling	8	2	0.877	1.00	0.875			
Ramp	32	0.95	Rolling	0	0	1.000	1.00	34			
UpStream	0	0.90	Level	0	0	1.000	1.00	0			
DownStream	0	0.90	Level	0	0	1.000	1.00	0			
Merge Areas											
Estimation of V₁₂											
V ₁₂ = V _R + (V _F - V _R) ² / FD (Equation 25-8 or 25-9)											
1,000 using Equation (Exhibit 25-5)											
V ₁₂ = 875 pc/h											
V ₃ or V _{av34} = 0 pc/h (Equation 25-4 or 25-5)											
Is V ₃ or V _{av34} > 2,700 pc/h? Yes <input type="checkbox"/> No <input type="checkbox"/>											
Is V ₃ or V _{av34} > 1.5 * V ₁₂ ? Yes <input type="checkbox"/> No <input type="checkbox"/>											
If Yes, V _{12a} = 875 pc/h (Equation 25-18)											
Capacity Checks											
Actual	Capacity	LOS F?	Actual	Capacity	LOS F?	Actual	Capacity	LOS F?	Actual		
V _{FO}	908	Exhibit 25-7	V _F	Exhibit 25-14	Exhibit 25-14	V _R	Exhibit 25-14	Exhibit 25-14	Exhibit 25-14		
Flow Entering Merge Influence Area											
Actual	Max Desirable	Violation?	Actual	Max Desirable	Violation?	Actual	Max Desirable	Violation?	Actual		
V _{R12}	908	Exhibit 25-7	V ₁₂	4600-Alt	No	V ₁₂	4600-Alt	No	V ₁₂		
Level of Service Determination (if not F)											
D _R = 5.475 + 0.00734 * V _R + 0.0078 * V ₁₂ = 4.252 + 0.0086 * V ₁₂ - 0.009 * L _D											
D _R = 9.4 (pc/mi/h)											
LOS = A (Exhibit 25-4)											
Speed Determination											
M _S = 0.206 (Exhibit 25-19)											
S _R = 58.2 mph (Exhibit 25-19)											
S _F = N/A mph (Exhibit 25-19)											
S ₁ = 58.2 mph (Exhibit 25-19)											
S ₂ = 58.2 mph (Exhibit 25-19)											

General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	W+Trans	Junction	Harris Quarry Driveway-Chramp
Date Performed	9/4/2009	Jurisdiction	Menardocounty/Citizens
Analysis Time Period	4.45pm-5.45pm	Analysis Year	2000
Project Description: Harris Quarry - Project Conditions			
Inputs			
Upstream Adj Ramp	Terrain: Rolling	Downstream Adj Ramp	
<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off		<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off	
$L_{up} = 1000$ ft		$L_{down} =$ ft	
$V_u = 0$ veh/h		$V_d =$ veh/h	
Conversion to pc/h Under Base Conditions			
(pc/h)	V (veh/h)	PHF	Terrain
Freeway	1418	0.95	Rolling
Ramp	6	0.95	Rolling
DownStream	0	0.90	Level
Capacity Checks			
Estimation of V_{12}			
$V_{12} = V_R + (V_F - V_R)^{FD}$ (Equation 25-8 or 25-9)			
1,000 using Equation (Exhibit 25-5)			
$V_{12} = 1702$ pc/h			
Flow Entering Merge Influence Area			
Flow Entering Diverge Influence Area			
Level of Service Determination (if not F)			
$D_R = 5.475 + 0.00734 \cdot V_R + 0.0078 \cdot V_{12} - 0.00627 \cdot L_A$			
$D_R = 15.7$ (pc/mi/h)			
LOS = B (Exhibit 25-4)			
Speed Determination			
$M_S = 0.308$ (Exhibit 25-19)			
$S_R = 57.9$ mph (Exhibit 25-19)			
$S_U =$ N/A mph (Exhibit 25-19)			
$S_D = 57.9$ mph (Exhibit 25-15)			

General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	W+Trans	Junction	Harris Quarry Driveway-Chramp
Date Performed	11/24/2009	Jurisdiction	Menardocounty/Citizens
Analysis Time Period	2:15pm-3:15pm	Analysis Year	2030
Project Description: Harris Quarry - Project Conditions			
Inputs			
Upstream Adj Ramp	Terrain: Rolling	Downstream Adj Ramp	
<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off		<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off	
$L_{up} = 1000$ ft		$L_{down} =$ ft	
$V_u = 0$ veh/h		$V_d =$ veh/h	
Conversion to pc/h Under Base Conditions			
(pc/h)	V (veh/h)	PHF	Terrain
Freeway	1329	0.95	Rolling
Ramp	28	0.95	Rolling
DownStream	0	0.90	Level
Capacity Checks			
Estimation of V_{12}			
$V_{12} = V_R + (V_F - V_R)^{FD}$ (Equation 25-8 or 25-9)			
1,000 using Equation (Exhibit 25-5)			
$V_{12} = 1595$ pc/h			
Flow Entering Merge Influence Area			
Flow Entering Diverge Influence Area			
Level of Service Determination (if not F)			
$D_R = 5.475 + 0.00734 \cdot V_R + 0.0078 \cdot V_{12} - 0.00627 \cdot L_A$			
$D_R = 15.0$ (pc/mi/h)			
LOS = B (Exhibit 25-4)			
Speed Determination			
$M_S = 0.306$ (Exhibit 25-19)			
$S_R = 58.0$ mph (Exhibit 25-19)			
$S_U =$ N/A mph (Exhibit 25-19)			
$S_D = 58.0$ mph (Exhibit 25-15)			

General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	W+Trans	Junction	Harris Quarry Driveway-Chramp
Date Performed	9/4/2009	Jurisdiction	Menardocounty/Citizens
Analysis Time Period	4.45pm-5.45pm	Analysis Year	2000
Project Description: Harris Quarry - Project Conditions			
Inputs			
Upstream Adj Ramp	Terrain: Rolling	Downstream Adj Ramp	
<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off		<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off	
$L_{up} = 1000$ ft		$L_{down} =$ ft	
$V_u = 0$ veh/h		$V_d =$ veh/h	
Conversion to pc/h Under Base Conditions			
(pc/h)	V (veh/h)	PHF	Terrain
Freeway	1418	0.95	Rolling
Ramp	6	0.95	Rolling
DownStream	0	0.90	Level
Capacity Checks			
Estimation of V_{12}			
$V_{12} = V_R + (V_F - V_R)^{FD}$ (Equation 25-8 or 25-9)			
1,000 using Equation (Exhibit 25-5)			
$V_{12} = 1702$ pc/h			
Flow Entering Merge Influence Area			
Flow Entering Diverge Influence Area			
Level of Service Determination (if not F)			
$D_R = 5.475 + 0.00734 \cdot V_R + 0.0078 \cdot V_{12} - 0.00627 \cdot L_A$			
$D_R = 15.7$ (pc/mi/h)			
LOS = B (Exhibit 25-4)			
Speed Determination			
$M_S = 0.308$ (Exhibit 25-19)			
$S_R = 57.9$ mph (Exhibit 25-19)			
$S_U =$ N/A mph (Exhibit 25-19)			
$S_D = 57.9$ mph (Exhibit 25-15)			

General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	WTrans	Junction	Harris Quarry Driveway-Chramp
Date Performed	11/24/2009	Jurisdiction	Menomonic County/Citizens
Analysis Time Period	October Peak 11am-	Analysis Year	2030
Project Description: Harris Quarry - Project Conditions			
Terrain: Rolling			
Upstream Adj Ramp	Yes <input type="checkbox"/> No <input type="checkbox"/> On <input type="checkbox"/> Off <input type="checkbox"/>	Downstream Adj Ramp	Yes <input type="checkbox"/> No <input type="checkbox"/> On <input type="checkbox"/> Off <input type="checkbox"/>
$S_{FF} = 65.0$ mph	$S_{FR} = 35.0$ mph	$S_{FR} = 35.0$ mph	$S_{FR} = 35.0$ mph
$S_{up} = 1000$ ft	$S_{down} = 1000$ ft	$S_{up} = 1000$ ft	$S_{down} = 1000$ ft
$V_u = 0$ veh/h	$V_d = 0$ veh/h	$V_u = 0$ veh/h	$V_d = 0$ veh/h
Conversion to pc/h Under Base Conditions			
(pc/h)	PHF	Terrain	%Truck
Freeway	0.95	Rolling	8
Ramp	0.95	Rolling	0
UpStream	0	Level	0
DownStream	0	Level	0
Merge Areas			
Diverge Areas			
Estimation of V_{12}			
$V_{12} = V_R + (V_F - V_R) \cdot P_{FD}$ (Equation 25-8 or 25-9)			
EQ = 1,000 using Equation (Exhibit 25-5)			
P = FD = 0			
F = 0			
V ₁₂ = 1036 pc/h			
V ₃ or V _{as04} = 0			
Is V ₃ or V _{as04} > 2,700 pc/h? Yes <input type="checkbox"/> No <input type="checkbox"/>			
Is V ₃ or V _{as04} > 1.5 * V ₁₂ ? Yes <input type="checkbox"/> No <input type="checkbox"/>			
If Yes, V _{12a} = 1036 pc/h (Equation 25-18)			
Capacity Checks			
Actual	Capacity	LOS F?	Capacity
V _{FO} = 717	Exhibit 25-7	No	Exhibit 25-14
V _R = 0	Exhibit 25-5		Exhibit 25-14
V _R = 0	Exhibit 25-3		Exhibit 25-3
Flow Entering Merge Influence Area			
Actual	Max Desirable	Violation?	Actual
V _{R12} = 717	4600/AI	No	V ₁₂ = 1036
Level of Service Determination (if not F)			
$D_R = 5.475 + 0.00734 \cdot V_R + 0.0078 \cdot V_{12}$			
$D_R = 7.9$ (pc/mi/h)			
LOS = A (Exhibit 25-4)			
Speed Determination			
$M_S = 0.294$ (Exhibit 25-19)			
$S_R = 58.2$ mph (Exhibit 25-19)			
$S_S = N/A$ mph (Exhibit 25-19)			
$S_S = 58.2$ mph (Exhibit 25-15)			

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst: TDH Agency or Company: W+Tans Date Performed: 9/4/2009 Analysis Time Period: 2:15pm-2:30pm Project Description: Harris Quarry - Project Conditions					Freeway/Dir of Travel: Northbound US 101 Junction: Harris Quarry Driveway - Chirrup, Mendocino County/Caltans Jurisdiction: Harris Quarry - Project Conditions Analysis Year: 2030				
Inputs Terrain: Rolling Upstream Adj Ramp: <input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off Downstream Adj Ramp: <input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off $L_{up} = 1000$ ft $V_u = 0$ veh/h $S_{FF} = 65.0$ mph $S_{FR} = 35.0$ mph Sketch (show lanes, L_{LD} , V_{LD})									
Conversion to pc/h Under Base Conditions									
(pc/h)	V (veh/h)	PHF	Terrain	%Truck	%Rv	f_{HV}	f_p	f_p	$V = V/PHE \times f_p$
Freeway	1089	0.95	Rolling	8	2	0.877	1.00	1307	
Ramp	6	0.95	Rolling	0	0	1.000	1.00	6	
UpStream	0	0.90	Level	0	0	1.000	1.00	0	
DownStream									
Merge Areas									
Estimation of V_{12} $V_{12} = V_R + (V_F - V_R)^{FD}$ (Equation 25-8 or 25-9) $V_{12} = 1307$ pc/h V_3 or $V_{a04} = 0$ pc/h (Equation 25-15 or 25-16) V_3 or $V_{a04} > 2,700$ pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No V_3 or $V_{a04} > 1.5 \cdot V_{12}$? <input type="checkbox"/> Yes <input type="checkbox"/> No $V_{12} = 1307$ pc/h (Equation 25-18)									
Capacity Checks									
Actual	Capacity	LOS F?	Capacity		Actual	Capacity	LOS F?		
$V_{FO} = 1313$	Exhibit 25-7	No	Exhibit 25-7	Exhibit 25-7	$V_F = 1313$	Exhibit 25-14	Exhibit 25-14	Exhibit 25-14	
Flow Entering Merge Influence Area									
Actual	Max Desirable	Violation?	Actual	Max Desirable	Violation?	Actual	Max Desirable	Violation?	
$V_{R12} = 1313$	Exhibit 25-7	No	Exhibit 25-7	Exhibit 25-7	No	$V_{12} = 1313$	Exhibit 25-14	Exhibit 25-14	
Level of Service Determination (if not F) $D_R = 5.475 + 0.00734 \cdot V_R + 0.0078 \cdot V_{12} - 0.00627 \cdot L_A$ $D_R = 12.1$ (pc/mi/h) $LOS = B$ (Exhibit 25-4)									
Speed Determination $M_S = 0.300$ (Exhibit 25-19) $S_R = 58.1$ mph (Exhibit 25-19) $S_U = N/A$ mph (Exhibit 25-19) $S_D = 58.1$ mph (Exhibit 25-15)									

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst: TDH Agency or Company: W+Tans Date Performed: 9/4/2009 Analysis Time Period: 2:15pm-2:30pm Project Description: Harris Quarry - Project Conditions					Freeway/Dir of Travel: Northbound US 101 Junction: Harris Quarry Driveway - Chirrup, Mendocino County/Caltans Jurisdiction: Harris Quarry - Project Conditions Analysis Year: 2030				
Inputs Terrain: Rolling Upstream Adj Ramp: <input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off Downstream Adj Ramp: <input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off $L_{up} = 1000$ ft $V_u = 0$ veh/h $S_{FF} = 65.0$ mph $S_{FR} = 35.0$ mph Sketch (show lanes, L_{LD} , V_{LD})									
Conversion to pc/h Under Base Conditions									
(pc/h)	V (veh/h)	PHF	Terrain	%Truck	%Rv	f_{HV}	f_p	f_p	$V = V/PHE \times f_p$
Freeway	1021	0.95	Rolling	8	2	0.877	1.00	1225	
Ramp	32	0.95	Rolling	0	0	1.000	1.00	34	
UpStream	0	0.90	Level	0	0	1.000	1.00	0	
DownStream									
Merge Areas									
Estimation of V_{12} $V_{12} = V_R + (V_F - V_R)^{FD}$ (Equation 25-8 or 25-9) $V_{12} = 1225$ pc/h V_3 or $V_{a04} = 0$ pc/h (Equation 25-15 or 25-16) V_3 or $V_{a04} > 2,700$ pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No V_3 or $V_{a04} > 1.5 \cdot V_{12}$? <input type="checkbox"/> Yes <input type="checkbox"/> No $V_{12} = 1225$ pc/h (Equation 25-18)									
Capacity Checks									
Actual	Capacity	LOS F?	Capacity		Actual	Capacity	LOS F?		
$V_{FO} = 1259$	Exhibit 25-7	No	Exhibit 25-7	Exhibit 25-7	$V_F = 1259$	Exhibit 25-14	Exhibit 25-14	Exhibit 25-14	
Flow Entering Merge Influence Area									
Actual	Max Desirable	Violation?	Actual	Max Desirable	Violation?	Actual	Max Desirable	Violation?	
$V_{R12} = 1259$	Exhibit 25-7	No	Exhibit 25-7	Exhibit 25-7	No	$V_{12} = 1259$	Exhibit 25-14	Exhibit 25-14	
Level of Service Determination (if not F) $D_R = 5.475 + 0.00734 \cdot V_R + 0.0078 \cdot V_{12} - 0.00627 \cdot L_A$ $D_R = 12.1$ (pc/mi/h) $LOS = B$ (Exhibit 25-4)									
Speed Determination $M_S = 0.300$ (Exhibit 25-19) $S_R = 58.1$ mph (Exhibit 25-19) $S_U = N/A$ mph (Exhibit 25-19) $S_D = 58.1$ mph (Exhibit 25-15)									

RAMPS AND RAMP JUNCTIONS WORKSHEET																																																											
General Information					Site Information																																																						
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<p>Inputs</p> <p>Terrain: Rolling</p> <p>Upstream Adj: <input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off <input type="checkbox"/> No <input type="checkbox"/> Off</p> <p>Adj Ramp: <input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off <input type="checkbox"/> No <input type="checkbox"/> Off</p> <p>$L_{up} = 1000$ ft $S_{FR} = 35.0$ mph $S_{down} =$ ft</p> <p>$V_u = 0$ veh/h $V_d =$ veh/h</p>																																																											
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(pc/h)	V (veh/h)	PHF	Terrain	%Truck	%Rv	f_{HV}	f_p	f_{pHV}	$V = V/PHF \times f_p \times f_{pHV}$																																																		
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<p>Level of Service Determination (if not F)</p> <p>$D_R = 5.475 + 0.00734 \cdot V_R + 0.0078 \cdot V_{12}$</p> <p>$D_R = 4.252 + 0.0086 \cdot V_{12} - 0.009 \cdot L_D$</p> <p>$D_R = 14.5$ (pc/mi/h)</p> <p>LOS = B (Exhibit 25-4)</p>																																																											
<p>Speed Determination</p> <p>$M_S = 0.305$ (Exhibit 25-19)</p> <p>$S_R = 58.2$ mph (Exhibit 25-19)</p> <p>$S_S = N/A$ mph (Exhibit 25-19)</p> <p>$S = 58.0$ mph (Exhibit 25-14)</p>																																																											

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V_{R12}	1028	Exhibit 25-7	V_{12}	4600-Alt	14																																																						
<p>Level of Service Determination (if not F)</p> <p>$D_R = 5.475 + 0.00734 \cdot V_R + 0.0078 \cdot V_{12}$</p> <p>$D_R = 4.252 + 0.0086 \cdot V_{12} - 0.009 \cdot L_D$</p> <p>$D_R = 10.3$ (pc/mi/h)</p> <p>LOS = B (Exhibit 25-4)</p>																																																											
<p>Speed Determination</p> <p>$M_S = 0.297$ (Exhibit 25-19)</p> <p>$S_R = 58.2$ mph (Exhibit 25-19)</p> <p>$S_S = N/A$ mph (Exhibit 25-19)</p> <p>$S = 58.0$ mph (Exhibit 25-14)</p>																																																											

General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	W-Trans	Junction	Harris Quarry Driveway-Chramp
Date Performed	9/4/2009	Jurisdiction	Menardoto County/Citizens
Analysis Time Period	4:45pm-5:45pm	Analysis Year	2040
Project Description: Harris Quarry - Project Conditions			
Inputs			
Upstream Adj Ramp	Terrain: Rolling	Downstream Adj Ramp	
<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off		<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off	
$S_{FF} = 65.0$ mph		$S_{FR} = 35.0$ mph	
$S_{up} = 1000$ ft		$S_{down} =$ ft	
$V_u = 0$ veh/h		$V_D =$ veh/h	
Conversion to pc/h Under Base Conditions			
(pc/h)	PHF	Terrain	%Truck
Freeway	0.95	Rolling	8
Ramp	0.95	Rolling	0
UpStream	0	Level	0
DownStream	0	Level	0
Merge Areas			
Estimation of V_{12}			
$V_{12} = V_R + (V_F - V_R)^{0.75} \cdot F_{FD}$ (Equation 25-2)			
= 1,000 using Equation (Exhibit 25-5)			
Capacity Checks			
Actual	Capacity	LOS F?	Capacity
1938	1938	No	1938
Flow Entering Merge Influence Area			
Actual	Max Desirable	Violation?	Actual
1938	4600-All	No	1938
Level of Service Determination (if not F)			
$D_R = 5.475 + 0.00734 \cdot V_R + 0.0078 \cdot V_{12} - 0.00627 \cdot L_A$			
$D_R = 7.5$ (pc/mi/h)			
LOS = B (Exhibit 25-4)			
Speed Determination			
$M_S = 0.313$ (Exhibit 25-19)			
$S_R = 57.8$ mph (Exhibit 25-19)			
$S_S = N/A$ mph (Exhibit 25-19)			
$S_{12} = 57.8$ mph (Exhibit 25-14)			

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General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	W-Trans	Junction	Harris Quarry Driveway-Chramp
Date Performed	11/24/2009	Jurisdiction	Menardoto County/Citizens
Analysis Time Period	2:15pm-3:15pm	Analysis Year	2040
Project Description: Harris Quarry - Project Conditions			
Inputs			
Upstream Adj Ramp	Terrain: Rolling	Downstream Adj Ramp	
<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off		<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off	
$S_{FF} = 65.0$ mph		$S_{FR} = 35.0$ mph	
$S_{up} = 1000$ ft		$S_{down} =$ ft	
$V_u = 0$ veh/h		$V_D =$ veh/h	
Conversion to pc/h Under Base Conditions			
(pc/h)	PHF	Terrain	%Truck
Freeway	0.95	Rolling	8
Ramp	0.95	Rolling	0
UpStream	0	Level	0
DownStream	0	Level	0
Merge Areas			
Estimation of V_{12}			
$V_{12} = V_R + (V_F - V_R)^{0.75} \cdot F_{FD}$ (Equation 25-2)			
= 1,811 using Equation (Exhibit 25-5)			
Capacity Checks			
Actual	Capacity	LOS F?	Capacity
1840	1840	No	1840
Flow Entering Merge Influence Area			
Actual	Max Desirable	Violation?	Actual
1840	4600-All	No	1840
Level of Service Determination (if not F)			
$D_R = 5.475 + 0.00734 \cdot V_R + 0.0078 \cdot V_{12} - 0.00627 \cdot L_A$			
$D_R = 16.7$ (pc/mi/h)			
LOS = B (Exhibit 25-4)			
Speed Determination			
$M_S = 0.311$ (Exhibit 25-19)			
$S_R = 57.9$ mph (Exhibit 25-19)			
$S_S = N/A$ mph (Exhibit 25-19)			
$S_{12} = 57.9$ mph (Exhibit 25-14)			

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General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	WTrans	Junction	Harris Quarry Driveway-Chramp
Date Performed	11/24/2009	Jurisdiction	Menard County/Cattans
Analysis Time Period	October Peak 11am-12noon	Analysis Year	2040
Project Description: Harris Quarry - Project Conditions			
Inputs			
Upstream Adj Ramp	Terrain: Rolling		Downstream Adj Ramp
<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off			<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off
$L_{up} = 1000$ ft	$S_{FF} = 65.0$ mph		$S_{FR} = 35.0$ mph
$V_u = 0$ veh/h	Sketch (show lanes, L_{up} , L_{down} , V_{12})		$V_d =$ veh/h
Conversion to pc/h Under Base Conditions			
(pc/h)	V (veh/h)	PHF	Terrain
		%Truck	%Rv
Freeway	980	0.95	Rolling
Ramp	44	0.95	Rolling
UpStream	0	0.90	Level
DownStream	0	0.90	Level
Merge Areas			
Estimation of V_{12}			
$V_{12} = V_R + (V_F - V_R)^{0.75} \cdot F_{FD}$ (Equation 25-2)			
$V_{12} = 1000$ using Equation (Exhibit 25-3)			
$V_{12} = 1176$ pc/h			
Capacity Checks			
V_3 or $V_{a34} > 2,700$ pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No			
V_3 or $V_{a34} > 1.5 \cdot V_{12}$? <input type="checkbox"/> Yes <input type="checkbox"/> No			
$V_{12a} = 1176$ pc/h (Equation 25-8)			
Capacity Checks			
Actual	Capacity	LOS F?	Capacity
$V_{FO} = 1222$	Exhibit 25-7	No	Exhibit 25-14
Flow Entering Merge Influence Area			
Actual	Max Desirable	Violation?	Actual
$V_{R12} = 1222$	Exhibit 25-7	No	Exhibit 25-14
Level of Service Determination (if not F)			
$D_R = 5.475 + 0.00734 \cdot V_{12} - 0.0078 \cdot V_{12}^2$			
$D_R = 11.9$ (pc/mi/h)			
$LOS = B$ (Exhibit 25-4)			
Speed Determination			
$M_S = 0.299$ (Exhibit 25-19)			
$S_R = 58.1$ mph (Exhibit 25-19)			
$S_U = N/A$ mph (Exhibit 25-19)			
$S_D = 58.1$ mph (Exhibit 25-15)			

General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	WTrans	Junction	Harris Quarry Driveway-Chramp
Date Performed	11/24/2009	Jurisdiction	Menard County/Cattans
Analysis Time Period	October Peak 9am-10am	Analysis Year	2040
Project Description: Harris Quarry - Project Conditions			
Inputs			
Upstream Adj Ramp	Terrain: Rolling		Downstream Adj Ramp
<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off			<input type="checkbox"/> Yes <input type="checkbox"/> On <input type="checkbox"/> No <input type="checkbox"/> Off
$L_{up} = 1000$ ft	$S_{FF} = 65.0$ mph		$S_{FR} = 35.0$ mph
$V_u = 0$ veh/h	Sketch (show lanes, L_{up} , L_{down} , V_{12})		$V_d =$ veh/h
Conversion to pc/h Under Base Conditions			
(pc/h)	V (veh/h)	PHF	Terrain
		%Truck	%Rv
Freeway	635	0.95	Rolling
Ramp	44	0.95	Rolling
UpStream	0	0.90	Level
DownStream	0	0.90	Level
Merge Areas			
Estimation of V_{12}			
$V_{12} = V_R + (V_F - V_R)^{0.75} \cdot F_{FD}$ (Equation 25-2)			
$V_{12} = 782$ pc/h			
Capacity Checks			
V_3 or $V_{a34} > 2,700$ pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No			
V_3 or $V_{a34} > 1.5 \cdot V_{12}$? <input type="checkbox"/> Yes <input type="checkbox"/> No			
$V_{12a} = 782$ pc/h (Equation 25-8)			
Capacity Checks			
Actual	Capacity	LOS F?	Capacity
$V_{FO} = 808$	Exhibit 25-7	No	Exhibit 25-14
Flow Entering Merge Influence Area			
Actual	Max Desirable	Violation?	Actual
$V_{R12} = 808$	Exhibit 25-7	No	Exhibit 25-14
Level of Service Determination (if not F)			
$D_R = 5.475 + 0.00734 \cdot V_{12} - 0.0078 \cdot V_{12}^2$			
$D_R = 8.6$ (pc/mi/h)			
$LOS = A$ (Exhibit 25-4)			
Speed Determination			
$M_S = 0.295$ (Exhibit 25-19)			
$S_R = 58.2$ mph (Exhibit 25-19)			
$S_U = N/A$ mph (Exhibit 25-19)			
$S_D = 58.2$ mph (Exhibit 25-15)			

General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	WTrans	Junction	Harris Quarry Driveway-Chramp
Date Performed	9/4/2009	Jurisdiction	Menardocounty/Citizens
Analysis Time Period	2:15pm	Analysis Year	2040
Project Description: Harris Quarry - Project Conditions			

Inputs	
Upstream Adj Ramp	Terrain: Rolling
Downstream Adj Ramp	
S_{FF}	65.0 mph
S_{FR}	35.0 mph
S_{up}	1000 ft
S_{down}	ft
V_u	veh/h
V_d	veh/h

Conversion to pc/h Under Base Conditions	
Freeway	1238
Ramp	6
UpStream	0
DownStream	0

Merge Areas		Diverge Areas	
V_{12}	V_F (FPM)	Equation 25-2	Equation 25-2
EO		1,000 using Equation (Exhibit 25-3)	1,000 using Equation (Exhibit 25-3)
FM			
V_{12}		1483 pc/h	1483 pc/h

Capacity Checks	
Actual	Capacity
1489	1489
Violation?	No

Flow Entering Merge Influence Area	
Actual	Max Desirable
1489	4600-Alt
Violation?	No

Level of Service Determination (if not F)	
D_R	4.252 + 0.0086 V_{12} - 0.009 L_D
D_R	14.0 (pc/mi/h)
LOS	B (Exhibit 25-4)

Speed Determination	
M_S	0.302 (Exhibit 25-19)
S_R	58.0 mph (Exhibit 25-19)
S_u	N/A mph (Exhibit 25-19)
S_d	58.0 mph (Exhibit 25-15)

General Information		Site Information	
Analyst	TDH	Freeway/Dir of Travel	Northbound US 101
Agency or Company	WTrans	Junction	Harris Quarry Driveway-Chramp
Date Performed	9/4/2009	Jurisdiction	Menardocounty/Citizens
Analysis Time Period	2:15pm	Analysis Year	2040
Project Description: Harris Quarry - Project Conditions			

Inputs	
Upstream Adj Ramp	Terrain: Rolling
Downstream Adj Ramp	
S_{FF}	65.0 mph
S_{FR}	35.0 mph
S_{up}	1000 ft
S_{down}	ft
V_u	veh/h
V_d	veh/h

Conversion to pc/h Under Base Conditions	
Freeway	1159
Ramp	32
UpStream	0
DownStream	0

Merge Areas		Diverge Areas	
V_{12}	V_F (FPM)	Equation 25-2	Equation 25-2
EO		1,000 using Equation (Exhibit 25-3)	1,000 using Equation (Exhibit 25-3)
FM			
V_{12}		1391 pc/h	1391 pc/h

Capacity Checks	
Actual	Capacity
1425	1425
Violation?	No

Flow Entering Merge Influence Area	
Actual	Max Desirable
1425	4600-Alt
Violation?	No

Level of Service Determination (if not F)	
D_R	4.252 + 0.0086 V_{12} - 0.009 L_D
D_R	13.4 (pc/mi/h)
LOS	B (Exhibit 25-4)

Speed Determination	
M_S	0.302 (Exhibit 25-19)
S_R	58.0 mph (Exhibit 25-19)
S_u	N/A mph (Exhibit 25-19)
S_d	58.0 mph (Exhibit 25-15)